

Determining Spatial Distribution and Physical Properties of the
Vashon Advance Outwash near Mountlake Terrace, WA

Kendra Pivaroff-Ward

A report prepared in partial fulfillment of
the requirements for the degree of:

Master of Science
Earth and Space Sciences: Applied Geosciences

University of Washington

June 2015

Project Mentors:

Galan McInelly, GeoEngineers, Inc.
Bridget August, GeoEngineers, Inc.

Internship Coordinator:

Kathy Troost

Reading Committee:

Juliet Crider
Kathy Troost
Steven Walters

© Copyright 2015

Kendra Pivaroff-Ward

Executive Summary

This study is aimed at determining the spatial distribution, physical properties, and groundwater conditions of the Vashon advance outwash (Qva) in the Mountlake Terrace, WA area. The Qva is correlative with the Esperance Sand, as defined at its type section; however, local variations in the Qva are not well-characterized (Mullineaux, 1965). While the Qva is a dense glacial unit with low compressibility and high frictional shear strength (Gurtowski and Boirum, 1989), the strength of this unit can be reduced when it becomes saturated (Tubbs, 1974). This can lead to caving or flowing in excavations, and on a larger scale, can lead to slope failures and mass-wasting when intersected by steep slopes. By studying the Qva, we can better predict how it will behave under certain conditions, which will be beneficial to geologists, hydrogeologists, engineers, and environmental scientists during site assessments and early phases of project planning.

In this study, I use data from 27 geotechnical borings from previous field investigations and C-Tech Corporation's *EnterVol* software to create three-dimensional models of the subsurface geology in the study area. These models made it possible to visualize the spatial distribution of the Qva in relation to other geologic units. I also conducted a comparative study between data from the borings and generalized published data on the spatial distribution, relative density, soil classification, grain-size distribution, moisture content, groundwater conditions, and aquifer properties of the Qva.

I found that the elevation of the top of the Qva ranges from 247 to 477 ft. I found that the Qva is thickest where the modern topography is high, and is thinnest where the topography is low. The thickness of the Qva ranges from absent to 242 ft. Along the northern, east-west trending transect, the Qva thins to the east as it rises above a ridge composed of Pre-Vashon glacial deposits. Along the southern, east-west trending transect, the Qva pinches out against a ridge composed of pre-Vashon interglacial deposits. Two plausible explanations for this ridge are paleotopography and active faulting associated with the Southern Whidbey Fault Zone. Further investigations should be done using geophysical methods and the modeling methods described in this study to determine the nature of this ridge.

The relative density of the Qva in the study area ranges from loose to very dense, with the loose end of the spectrum probably relating to heave in saturated sands. I found subtle correlations between density and depth. Volumetric analysis of the soil groups listed in the boring logs indicate that the Qva in the study area is composed of approximately 9.5% gravel, 89.3% sand, and 1.2% silt and clay. The natural moisture content ranges from 3.0 to 35.4% in select samples from the Qva. The moisture content appears to increase with depth and fines content.

The water table in the study area ranges in elevation from 231.9 to 458 ft, based on observations and measurements recorded in the boring logs. The results from rising-head

and falling-head slug tests done at a single well in the study area indicate that the geometric mean of hydraulic conductivity is 15.93 ft/d (5.62×10^{-03} cm/s), the storativity is 3.28×10^{-03} , and the estimated transmissivity is 738.58 ft²/d in the vicinity of this observation well. At this location, there was 1.73 ft of seasonal variation in groundwater elevation between August 2014 and March 2015.

Table of Contents

1.0	Introduction	1
1.1	Study Area	1
2.0	Scope of Work	2
3.0	Background	2
3.1	Previous Investigations	2
3.2	Vashon Advance Outwash	4
3.2.1	Hydrogeology	5
4.0	Geologic Setting	6
4.1	Regional Geology	6
4.2	Local Geology	7
4.3	Structural Geology	7
5.0	Methods	9
5.1	Data Acquisition	9
5.1.1	Unit Thickness	9
5.1.2	Relative Density	10
5.1.3	Laboratory Test Results	10
5.1.4	Groundwater Conditions	11
5.2	Data Analysis	11
5.3	Modeling	11
5.3.1	Cross-Sections	12
5.3.2	Three-Dimensional Models	12
6.0	Observations	13
6.1	Spatial Distribution	13
6.2	Physical Properties	13
6.2.1	Relative Density	13
6.2.2	Grain Size	14
6.2.3	Moisture Content	14
6.3	Groundwater Conditions	14
7.0	Analysis & Discussion	15
7.1	Spatial Distribution	15

7.1.1	Variability in Unit Thickness	16
7.2	Physical Properties	17
7.3	Groundwater Conditions	17
8.0	Conclusions	18
9.0	Limitations and Assumptions	20
9.1	Software Limitations	20
10.0	Recommended Future Studies.....	21
10.1	Local Characterization of the Vashon Advance Outwash	21
10.2	Locating Southern Whidbey Island Fault Traces	21
11.0	References Cited	22
12.0	Figures	28
13.0	Tables	52
14.0	Appendices	57

List of Figures

Figure 1	Vicinity Map
Figure 2	Regional Map
Figure 3	Generalized Quaternary Geologic Section
Figure 4	Transect 1 Cross-Section
Figure 5	Transect 2 Cross-Section
Figure 6	Transect 3 Cross-Section
Figure 7	Transect 4 Cross-Section
Figure 8	Transect 5 Cross-Section
Figure 9A	<i>EnterVol</i> Block Diagram
Figure 9B	<i>EnterVol</i> Block Diagram with Vicinity Map Overlay
Figure 10	<i>EnterVol</i> Fence Diagram
Figure 11A	Qva with <i>EnterVol</i> Fence Diagram (From Above)
Figure 11B	Qva with <i>EnterVol</i> Fence Diagram (From Below)
Figure 12	Isopach Map of Qva Thickness
Figure 13	Density vs Fines Content
Figure 14	Elevation vs Density
Figure 15	Depth vs Density
Figure 16A	Summary of Soil Groups within the Qva
Figure 16B	Volumetric Analysis of Soil Groups within the Qva
Figure 17	Depth and Elevation vs Fines Content
Figure 18	Depth and Elevation vs Moisture Content
Figure 19	Fines Content vs Moisture Content
Figure 20	Groundwater Observations at Boring LLE-B11P

List of Tables

Table 1	Summary of Geotechnical Boreholes
Table 2	Geologic Contacts Database
Table 3	Volumetric Analysis of Soil Groups within the Qva
Table 4	Comparative Results

List of Appendices

Appendix A	Summary of Hydraulic Conductivity Data for Glacial Sediments and Conceptual Hydrogeologic Model
Appendix B	Boring Logs
Appendix C	Borehole Data Summary Sheet
Appendix D	Unified Soil Classification Guide

Acknowledgments

I would like to give thanks to GeoEngineers, for hosting me as an intern, and to Galan McNelly and Bridget August for their mentorship. I would also like to acknowledge Dan Campbell and Tim Bailey for allowing me to participate in the Sound Transit – Lynnwood Link Extension Project, and for their general support of this project. Gene Lohrmeyer deserves special recognition for his contribution to the modeling efforts, for which I am very appreciative. Additionally, I would like to thank Eliya Hogan and Johanna Neff for their support throughout the project.

I would also like to thank Dave McCormack at Aspect Consulting for providing me with boring logs and geotechnical reports for both phases of the Brightwater Treatment Plant Conveyance System Project.

Finally, I would like to acknowledge my reading committee, Juliet Crider, Kathy Troost, and Steven Walters, for their support of this project and dedication to the MESSAGE Program. I would like to give a special thanks to Kathy for helping formulate this project, providing references, taking me in the field, and sharing her wealth of knowledge on the geology and stratigraphy of the Puget Lowland.

Acronyms and Abbreviations

ASTM	American Society for Testing and Materials
BGS	Below ground surface
Brightwater Project	Brightwater Treatment Plant Conveyance System
CL	Lean clay
D&M	Dames & Moore
DEM	Digital Elevation Model
GeoEngineers	GeoEngineers, Incorporated
GP	Poorly-graded gravel
GP-GM	Poorly-graded gravel with silt
GM	Silty gravel
GW	Well-graded gravel
ID	Inner Diameter
ML	Lean silt
M _w	Moment magnitude
Mya	Million years ago
N-Value	Standard Penetration Resistance
NAVD	North American Vertical Datum
OD	Outer Diameter
Qaf	Quaternary artificial fill
Qpfnmw	Interglacial mass wasting deposits (pre-Vashon)
Qpg	Glacial, grouped (pre-Vashon)
Qpgf	Glacial, fluvial (pre-Vashon)
Qpgl	Glacial, lacustrine (pre-Vashon)
Qpn	Interglacial, grouped (pre- Vashon)
Qpnf	Interglacial fluvial (pre- Vashon)
Qpnl	Interglacial lacustrine (pre- Vashon)

Qva	Vashon advance outwash
Qvlc	Vashon Lawton Clay
Qvr	Vashon recessional outwash
Qvt	Vashon till
SFZ	Seattle Fault Zone
SM	Silty sand
SP	Poorly-graded sand
SP-SM	Poorly-graded sand with silt
SPT	Standard Penetration Test
ST-LLE	Sound Transit Lynnwood Link Extension
SW	Well-graded sand
SWIF	Southern Whidbey Island Fault Zone
SW-SM	Well-graded sand with silt
USGS	United States Geological Survey
UW	University of Washington

1.0 Introduction

The purpose of this research project is to characterize the spatial distribution and physical properties of the Vashon advance outwash (Qva) near Mountlake Terrace, WA, and to compare these characteristics to more extensive and generalized properties of the Qva, as defined in previous works. The information and methods provided in this report will be beneficial to geologists, hydrogeologists, engineers, and environmental scientists during site assessments and early phases of project planning.

In general, Qva is a granular, well-sorted, and permeable glaciofluvial unit (Mullineaux *et al.*, 1965). According to Troost *et al.* (2005), the Qva is equivalent of the Esperance Sand of Mullineaux *et al.* (1965). Although the Qva has high frictional shear strength and low compressibility as the result of glacial consolidation (Gurtowski and Boirum, 1989), some of the unit's properties can facilitate increased geologic or environmental risks. For instance, the development of water pressure in the pore space between granular particles may significantly decrease the shear strength of the unit; this is a contributing factor to landslides in western Washington (Tubbs, 1974). The permeable properties of the Qva also pose difficulty to excavations where dewatering, shoring, or other procedures are required to prevent water-bearing sands from caving, flowing, or collapsing (Laprade and Robinson, 1989). Additionally, the Qva forms an extensive, unconfined aquifer that may act as a conduit for chemical pollutants (Bjerg and Christensen, 1993; Golder Associates, 2008). The properties described above make this hydrostratigraphic unit susceptible to contamination, erosion, seepage, and landslides. Understanding of both the spatial distribution and physical properties of Qva in a specific context can aid in developing improved mitigation practices and prediction of slope failures, more informed decisions in building design and practices, improved groundwater modeling, and more accurate mapping of areas susceptible to both geologic hazards and environmental contamination.

1.1 Study Area

The study area is in a mixed residential and commercial area in proximity to the community of Mountlake Terrace. Geographically, it is 12 mi north of Seattle's metropolitan center, 2.5 mi northwest of Lake Washington, and 1.5 mi east of the Puget Sound shoreline. Lake Ballinger is located towards the center of the study site. The topography of this area consists of gently rolling drumlin hills from the last glaciation. The relief at the study site ranges from approximately 289 to 492 ft in elevation. Geologic hazards at this site include landslide-prone areas and erosion/sedimentation hazards as defined by the City of Mountlake Terrace ordinance codes (City of Mountlake Terrace, 2015; GeoEngineers, 2015). The project site is about 2.95 sq mi, and includes 5 transect lines, totaling 8.84 lineal miles (Figure 1). The southern edge of the study area is approximately 75 ft south of the King/Snohomish County

border. The eastern border of the study area is Interstate-5. The location and boundaries of the study site were determined based on the availability of quality data.

2.0 Scope of Work

This report characterizes and refines the spatial distribution, physical properties, and groundwater conditions of the Qva in the Mountlake Terrace area by:

- 1) documenting the spatial extent and thickness variations of Qva in cross-sections and three-dimensional models,
- 2) describing the spatial variation in density using standard penetration test (SPT) data from existing geotechnical boring logs,
- 3) describing variations in grain size and moisture content, using lab analyses of available soil samples, and
- 4) describing aquifer properties and groundwater conditions where data is available.

The work described above consisted of the following tasks:

- 1) obtaining and reviewing existing technical reports, geologic maps, topographic maps, soil survey data, laboratory test results, groundwater data, geospatial imagery, and other publications that aided in characterizing the subsurface conditions of the project site,
- 2) creating a database that includes the locations of the boreholes and the elevation of every geologic contact within each borehole,
- 3) generating cross-sections and three-dimensional models, using information in the above described database and C-Tech Corporation's *EnterVol* software, to illustrate the lateral and vertical distributions of the advance outwash, and
- 4) evaluating variations in the unit thickness, elevation, physical properties, and groundwater conditions.

3.0 Background

In this section, I discuss the previous investigations that have been done within the study area and give a general overview of the Qva.

3.1 Previous Investigations

This study incorporates data from 27 existing geotechnical borings, summarized in Table 1, and their corresponding geotechnical reports. These data were collected by local consulting firms and government agencies for various geotechnical investigations. The boring logs and geotechnical reports are publicly available through the Freedom of Information Act, and I

obtained the data by directly contacting the firms and agencies that house the documents. The following paragraphs describe the investigations that were previously conducted in the study area, and the data from them that I used to supplement my research.

A large investigation was conducted in King and Snohomish Counties for the Brightwater Treatment Plant Conveyance System (Brightwater Project), which was completed in 2012. This project consisted of two geotechnical studies, which are discussed in the next two paragraphs.

In 2002, Shannon & Wilson and HWA Geoscience began the initial investigation for the Brightwater Project, which consisted of 27 geotechnical borings and laboratory testing of select soil samples (King County, 2002). The purpose of this investigation was to determine the subsurface conditions for conceptual engineering and environmental impact assessment. Shannon & Wilson used the mud rotary method to drill their borings, while HWA Geosciences used a combination of mud rotary and Becker Hammer methods. Both companies used a 2.42-inch inner diameter (ID), 3.25-inch outer diameter (OD), ring-lined Dames and Moore (D&M) split-barrel sampler with a 300 lb. hammer dropped 30-inches to obtain soil samples. The borings that I used from this study ranged from 352 to 446.5 ft in depth, and include MW-3, MW-4, MW-5, and MW-6 from HWA Geosciences, and BW-4, BW-5, and BW-6 from Shannon & Wilson (Table 1).

The second investigation for the Brightwater Project was completed by CDM Smith and various subconsultants in 2003. The purpose of this investigation was to provide geotechnical services for the design of the wastewater treatment facility. These services included land-based drilling, soil sampling, in-situ testing, hydrogeologic testing, gas monitoring, and geophysical explorations, as well as geologic, index, strength, and deformation testing in the laboratory (King County, 2004). This study included a total of 157 borings. The drilling methods used to complete the borings included hollow-stem auger, mud rotary, rotosonic coring, and wireline coring. Soil samples were collected using a D&M sampler with a 300 lb. or 140 lb. hammer. The borings that I used from this study include E-105, E-106, E-107, E-108, E-109, E110, and E211, which range from 260 to 566 ft in depth (Table 1). Index tests were not completed on any of the soil samples within the Qva from these six borings. However, geologic testing was done on select samples to determine the stratigraphic relationships between the soil units. This testing included radiocarbon dating, optically-stimulated luminescence dating, tephrochronology, x-ray diffraction mineral analysis, bulk geochemistry, and micro- and macro-paleontological analyses (of shells, diatoms, and pollen).

In 2008, Golder Associates conducted a study to provide an overview of the geology and hydrogeology near Mountlake Terrace for OTAK, Inc., and published their findings in a technical memorandum. They used a compilation of existing data, including the Brightwater Environmental Impact Statement, nearby monitoring wells, and city and county data, to

determine the groundwater conditions in their study area. This study was particularly important for the groundwater analysis of my research because not a lot of groundwater data were readily available in my study area; however the Golder Associates (2008) study overlapped with a portion of my study area.

Another large investigation was completed in the Mountlake Terrace area. During the summer of 2014, GeoEngineers, Inc. (GeoEngineers) provided geotechnical consulting services for the Sound Transit Lynnwood Link Extension¹ (ST-LLE), which will extend the Link light rail from Seattle to Lynnwood. This investigation used hollow-stem auger and mud rotary methods to drill 84 boreholes ranging from 40 to 101.5 ft in depth (GeoEngineers, 2015). Samples were collected using a 2-inch OD split-barrel standard penetration test (SPT) sampler in accordance with the American Society for Testing and Materials (ASTM) standard D 1586, or with a 3-inch diameter Shelby tube sampler in accordance with ASTM D 1587. The SPT samples were obtained by driving the sampler 18 inches into the soil with a 140 lb. hammer free-falling 30-inches. The data that I used from this investigation includes: index test results, groundwater measurements, and boring logs from 13 geotechnical borings (Table 1).

3.2 Vashon Advance Outwash

In general, glacial advance outwash is a thick unit of fluvial sediments that are deposited by high-energy meltwaters ahead of an advancing glacier (Koloski *et al.*, 1989). These glaciofluvial sediments are then overridden by the glacier, resulting in post-depositional compaction (Easterbrook, 1969). Advance outwash is mostly composed of clean sand, although it often contains a wide range of grain sizes (Tubbs, 1974). As with other fluvial systems, coarser materials are deposited close to the source, and finer materials are sorted and carried away from the source; this process results in a coarsening-up sequence, by which silty sands are deposited farther away from the glacier, and gravel and coarser grained materials are deposited closer to the glacier (Moses, 2008). However, this coarsening up sequence is only a simplified facies model, and in actuality braided streams, point bars, and other fluvial features complicate the stratigraphy of these deposits (Troost and Booth, 2008).

The Qva is described as well-sorted, dense to very dense, fine to medium sand with lenses of gravel, silt, and clay (Mullineaux *et al.*, 1965). The Qva type section is an outcrop in the cliffs at Fort Lawton, in Discovery Park, Seattle, WA (Troost and Booth, 2008). Here, the unit is defined as outwash related to the advance of the Vashon Glacier and includes the transitional zone from the underlying Lawton Clay (Qvlc) (Mullineaux *et al.*, 1965). The transitional zone between the Qva and the Qvlc is typically tens of feet thick, and contains interbedded sand and silt/clay representing the transition from a proglacial lake to a stream environment

¹ I contributed to various aspects of the project while interning at GeoEngineers. Tasks that I helped with included logging borings, taking piezometer and barometer measurements, observing slug tests, surveying, and conducting laboratory analysis of field samples.

(Kathy Troost, University of Washington (UW), personal communication, 2015). In areas where the Qvlc is absent, the onset of the Vashon Stade is marked by the Qva, which may be in contact with pre-Vashon glacial (Qpg) or interglacial deposits (Qpn) (King County, 2002; King County, 2004; Troost and Booth, 2008; GeoEngineers, 2015). About 50% of Qva is capped by Vashon glacial till (Qvt) (Kathy Troost, UW, written communication, 2015). The contact between the Qva and the Qvt varies between sharp and gradational (Laprade and Robinson, 1989; Troost and Booth, 2008). Exposures of Qva can be found bluffs and steep gullies that reach the upland (Kathy Troost, UW, written communication, 2015).

There is some variability in the bedding and depositional setting of the Qva; studies suggest that the Qva was deposited subaerially to subaqueously (Troost and Booth, 2008). Subaerial sedimentation is indicated by remnants of channels, gravel bars, and fine-grained lenses from braided streams, whereas subaqueous sedimentation is recognized by remnants of deltas, turbidites, and horizontal bedding in proglacial lakes, at the terminus of outwash streams (Troost and Booth, 2008). The Qva that is considered to have been subaqueously deposited contains foreset beds and cross-bedding that are steeply dipping at 30-40° (Kathy Troost, UW, personal communication, 2015). Cross-bedding is also common in subaerial deposits, but is generally not as tall as in the deltas (Kathy Troost, UW, written communication, 2015).

3.2.1 Hydrogeology

In the Mountlake Terrace area, the Qva forms an extensive, unconfined aquifer with a saturated thickness of about 100 ft, and an unsaturated thickness ranging from 20 to 100 ft (Golder Associates, 2008). The groundwater in the Qva discharges to surface water, primarily to Hall Creek and Lake Ballinger, via hydraulic connection with recessional outwash or alluvium (Golder Associates, 2008). Golder Associates (2008) states that Lake Ballinger appears to be underlain by Lawton Clay or pre-Fraser deposits, which have low permeability. Additionally, groundwater from the Qva may be recharging deeper aquifers in the area (Golder Associates, 2008). The rate of groundwater recharge ranges from 15 to 20 in/yr in areas where permeable outwash is exposed at the surface, and is less than 10 inches per year in areas that are capped by till or are urbanized (Golder Associates, 2008). Typical hydraulic conductivity data for glacial sediments, and a conceptual hydrogeologic model of geologic units located in the Mountlake Terrace area can be found in Appendix A.

As mentioned above, the basal portion of the Qva is often saturated with groundwater, which is retarded by the underlying less-permeable Qvlc, Qpf, or Qpg geologic units (Tubbs, 1974; Appendix A). If the contact between these units is exposed at the surface, the saturated zone in the Qva can be identified by seeps and springs (Miller, 1989). The contact between these hydrostratigraphic units has been identified as the location of frequent landslides in the Puget Lowland (Tubbs, 1974).

4.0 Geologic Setting

The following sections describe the regional, local, and structural geology in relation to the study area.

4.1 Regional Geology

The study area is located in the Puget Lowland section of the Salish Lowland physiographic province (Haugerud, 2004). The Puget Lowland is a structural and glacially-eroded trough centered between the Cascade Range to the east and the Olympic Mountains and Willapa Hills to the west (Troost and Booth, 2008; Moses, 2013; Figure 2). Major geographic features of the Puget Lowland include the San Juan Islands, the Puget Sound, and the Strait of Juan de Fuca. The Puget Lowland is characterized by a dynamic landscape that has been largely shaped by continental glaciations, tectonic activity, and volcanism (Troost and Booth, 2008). The geomorphic processes that occurred during glacial and interglacial periods have greatly influenced the modern topography of this region (Booth, 1994).

The Cordilleran Ice Sheet was a continental ice sheet that extended from southeastern Alaska, to northern Washington, and across to northwestern Montana during the Quaternary (about 2.59 million years ago (Mya) to present); there were been at least seven glacial advances during this time (Booth *et al.*, 2003; Troost and Booth, 2008). The Cordilleran Ice Sheet included the Puget, Okanogan, Columbia River, Purcell Trench, and Flathead Lobes, which extended into western Washington, north-central Washington, eastern Washington, northern Idaho, and northwestern Montana, respectively (Booth *et al.*, 2003). During the Vashon Stade of the Fraser Glaciation, the Puget Lobe extended farther south than Olympia, WA, and occupied the area between the Cascade Range and the Olympic Mountains (Thorson, 1979; Porter and Swanson, 1998; Troost and Booth, 2008; Figure 2). At its maximum extent, the Puget Lobe was as much as 3,300 ft (1,000 m) thick in the Seattle area, and 6,600 ft (2,000 m) in British Columbia (Porter and Swanson, 1998; Clague and James, 2002).

The surficial geology of the Puget Lowland consists predominantly of Vashon-aged (about 15,000 to 13,000 ya) glacial sediments, with intermittent exposures of Tertiary bedrock of Paleocene (about 66 to 56 Mya) to Oligocene (about 33.9 to 23 Mya) age (Moses, 2013). The generalized Quaternary section in the Puget Lowland consists of pre-Vashon glacial and interglacial deposits overlain by glaciolacustrine clays and silts, advance outwash sands, glacial till, and recessional outwash from the Vashon Stade (Galster and Laprade, 1991; Savage *et al.*, 2000; Figure 3). At least seven glacial advances have been documented in the Puget Lowland (Troost and Booth, 2008). Glacial loading has resulted in the over-consolidation of glacial and interglacial sediments, with the exception of Vashon recessional outwash, which was deposited as the glacier retreated (Galster and Laprade, 1991).

The slopes in the Puget Lowland are prone to landslides and other slope stability issues (Mullineaux *et al.*, 1965; Tubbs, 1975). There are several contributing factors for this, which include geologic and climatic conditions, as well as anthropogenic influences (Tubbs, 1974). The contact between the Qva and the less-permeable underlying units has been identified as the “slip-surface” for several large landslides in the Seattle area (Tubbs, 1974). Sixty-four percent of all historical (between 1909 and 1999) landslides in Seattle occurred within 150 ft of the Qva/Qvlc contact (Coe *et al.*, 2004). During periods of heavy precipitation, water can accumulate above silt and clay lenses within the Qva and above the confining layers that underlie the Qva (Miller, 1989). This occurrence consequently decreases the stability of the soils by elevating the pore fluid pressures between the grains in the Qva (Tubbs, 1974). The stratigraphic placement of the Qva, which is an aquifer, above less-permeable units, which create an aquitard, is a key factor in the landslides in this area (Tubbs, 1974).

4.2 Local Geology

The surficial geology in the vicinity of the project site is predominantly Vashon Stade glacial deposits, as is documented in the boring logs used in this study (King County, 2002; King County, 2004; GeoEngineers, 2015). A conceptual hydrogeologic model of the geologic units in the Mountlake Terrace area can be found in Appendix A (revised from Golder Associates, 2008). Glacial deposits of the Vashon Stade found in the study area include the following units: recessional outwash (Qvr), glacial till (Qvt), advanced outwash (Qva), and proglacial lacustrine deposits, which are formally referred to as the Lawton Clay (Qvlc). Also within the study reach is: Holocene alluvium (Qal), artificial fill (Qaf), and peat (Qp); pre-Fraser interglacial fluvial (Qpfnf), lacustrine (Qpfnl), wetland deposits (Qpfnw), and mass wastage deposits (Qpfnmw); and pre-Olympia glacial outwash (Qpogf), glaciolacustrine deposits (Qopgl), glaciomarine drift (Qpogm), glacial till (Qpogt, Qpogtm), and glacial diamict deposits (Qpogd). For the purposes of this paper, I amalgamated the pre-Fraser interglacial deposits and labeled them as Qpn. Similarly, I grouped the pre-Olympia glacial deposits together and labeled them as Qpg. In summary, the geology documented in the boring logs used in this study exemplify cycles of glacial and interglacial erosion and deposition.

4.3 Structural Geology

Tectonic activity in this area is occurring at both regional and local scales (Atwater *et al.*, 1995; Pratt *et al.*, 1997). At the regional scale, the convergence of the Juan de Fuca plate and the North American plate form the Cascadia Subduction Zone, which is capable of producing up to moment magnitude (M_w) 9.0 earthquakes (Wells *et al.*, 1998; Nedimovic *et al.*, 2003). Additionally, the northward movement of the Pacific plate is causing complex seismic strain (north-south shortening) to accumulate throughout western Washington and Oregon (Pratt *et al.*, 1997; Wells *et al.*, 1998; WA-DNR, 2015). This strain, in combination with glacial isostatic adjustment, has created several large fault systems in western Washington (Figure 2). These faults produce more than 1,000 earthquakes each year (Lasmanis, 1991). Thick

Quaternary deposits of glacial and interglacial sediments conceal many of these faults. Furthermore, dense vegetation and widespread urbanization obscure active fault traces, making it difficult to study faults in this area.

There are two major fault zones in proximity to the study site, the Seattle Fault Zone (SFZ) to the south, and the Southern Whidbey Island Fault Zone (SWIF), which may run through the project site (Blakely *et al.*, 2004; Troost and Booth, 2008; Barnett *et al.*, 2010). Both of these fault zones are poorly located, for reasons described above.

The SFZ is composed of a series of west-trending, south-dipping thrust faults that have resulted from north-south compression due to the convergence of the Pacific, Juan de Fuca, and North American plates (Johnson *et al.*, 2004; Lamb *et al.*, 2012). The Seattle fault is thought to be around 30 Myrs old, and is considered an active fault (Nelson *et al.*, 2014). There is evidence for several significant ruptures in the past 15,000 years, including 20 feet of vertical displacement during an earthquake dated 1,100 years ago (Atwater and Moore, 1992). The SFZ is about 31 mi long and is capable of producing up to Mw 7.0 earthquakes (Blakely *et al.*, 2002; Nelson *et al.*, 2014). The SFZ is recognized by Eocene (about 56 to 33.9 Mya) bedrock juxtaposed against Quaternary (about 2.59 Mya to present) glacial deposits, and large geophysical anomalies (Lasmanis, 1991; Blakely *et al.*, 2002). Three east-trending strands of the fault have been identified, although the exact boundaries of the SFZ are still under investigation (Johnson *et al.*, 1999; Blakely *et al.*, 2002).

The SWIF is another active fault in the Puget Lowland, and is capable of producing up to Mw 7.1 earthquakes (Sherrod *et al.*, 2008). Unlike the Seattle fault, the SWIF has a northwest-trend and a steep northeast-dip (Johnson *et al.*, 1996). The SWIF is thought to have originated in the early Eocene (about 56 to 47.8 Mya) as an arc-parallel strike-slip fault (Liberty and Pape, 2006). The SWIF may be as long as 93 mi long, originating in Victoria and extending to Seattle, where it may merge with the SFZ (Sherrod *et al.*, 2008). It has been identified through the use of seismic-reflection surveys, borehole data, and gravity and magnetic anomalies (Sherrod *et al.*, 2008). The SWIF is composed of several strands across a zone that is 3.75 to 6.8 mi wide (Johnson *et al.*, 1996). The strands have inferred dextral strike-slip, reverse, and thrust displacement (Johnson *et al.*, 1996). The SWIF was conceptualized by Johnson *et al.*, (1996) as an oblique, right-lateral strike-slip fault, that sometimes form transpressional flower structures, and by Brocher *et al.*, (2005) as an advancing wedge bound by roof and floor thrusts. Paleoseismological evidence suggests that the SWIF last ruptured about 2,700 years ago, and has produced at least four significant earthquakes since the retreat of the Vashon Glacier (Sherrod *et al.*, 2008). Evidence for recent activity includes stratigraphic offset and disruption, structural relief, displacement in Quaternary sediments, Quaternary folds, liquefaction features, and minor historical seismicity (Johnson *et al.*, 1996).

5.0 Methods

In this section I describe the methods that I used to obtain and analyze data, and create two- and three-dimensional subsurface models. In addition to studying the Qva in the study area for this project, the modeling methods described in this section can be applied to various other subsurface studies.

5.1 Data Acquisition

I began this study by reviewing existing data about the geology, topography, and hydrogeology of the area. During this stage, I gathered LiDAR images (U.S. Geological Survey, 2001), aerial photographs (Google Earth Pro, 2012), geologic maps (Booth *et al.*, 2004), and technical reports (King County, 2002; King County, 2004; Golder Associates, 2008; GeoEngineers, 2015). I used the Subsurface Geology Information System² published by the Washington State Department of Natural Resources (WA-DNR) to view available borehole information. I used the locations of boreholes with sufficient data to determine the boundaries of the study area and to draw the transect lines for the cross-sections. I contacted the appropriate consulting firms and government agencies to obtain the boring logs and geotechnical reports that were pertinent to my study. The boring logs and geotechnical reports that I used in my study were from GeoEngineers for the ST-LLE Project, and from Shannon & Wilson, HWA Geosciences, and CDM Smith for the Brightwater Project. These projects are described in detail in *Section 3.1*, and a summary of the borings used in this study can be found in Table 1. From the boring logs, I was able to obtain data on stratigraphic unit descriptions, soil classifications, elevations, blow counts, laboratory index test results, and groundwater observations. However, not all boring logs had this complete set of information. The boring logs also identified the geologic units that were observed while drilling, based on visual-manual classifications, laboratory testing, and/or age-determination. I used the labels found in the boring logs as a reference for identifying the Qva in my study.

5.1.1 Unit Thickness

To determine the thickness and variability of the Qva, I created a table of geologic contact elevations (Table 2) based on the information from the boring logs, which can be found in Appendix B. The vertical extent of this study was bound between the ground surface and 100 ft elevation, though not all borings reached this depth and many went deeper. Although the lower limit of the Qva is much shallower than 100 ft elevation, I chose to terminate the vertical extent of the study at this elevation because it provides sufficient context for the paleotopography that might influence the thickness of the Qva. I used Table 2 to develop five cross-sections, a block diagram, and a fence diagram, which are described in *Section 5.3*.

² The Subsurface Geology Information System originated from a database compiled by Kathy Troost and Aaron Wisher at the Pacific Northwest Center for Geologic Mapping Studies (GeoMapNW) at the University of Washington.

5.1.2 *Relative Density*

The soil density values that I used in this study were estimated using field data (N-values³) that were recorded at various depths during previous investigations. The N-values recorded for the ST-LLE Project were acquired using a SPT sampler, and were done in accordance with ASTM D 1586. However, the N-values recorded for the Brightwater Project were acquired using a D&M sampler. Although the D&M sampling method is different from the standard test, the blow counts still provide a relative indication of soil density and consistency (King County, 2002). SPT and D&M sampling are considered to be correlative with respect to blow counts, so long as the hammer weight is adjusted to the sampler size, so that the energy delivered to the subsurface is equivalent (Kathy Troost, UW, personal communication, 2015).

There are several variables that affect the integrity of N-value data; this includes drilling method, sampling method, and soil conditions. Different types of drilling methods influence the disturbance in the soil samples in different ways. For the purposes of this study, I analyzed N-value data from samples that were acquired during mud rotary drilling for the following reasons. Soil heave often occurs while drilling in water-bearing sands that are under confining pressures; this compromises the integrity of blow counts and soil samples (Nielsen, 2005). Mud rotary drilling reduces the pressure gradient by adding mud inside the auger, which minimizes heave in water-bearing sands, such as the Qva (Munch and Killey, 1985). Sampling method also effects the N-value. In this study, I use data collected by SPT and D&M sampling methods. Additionally, soil conditions influence the accuracy of blow counts. An inaccurate measure of the soil density can be measured if the soil sampler hits a large gravel or boulder that prevents the sampler from being driven into the soil. Also, if the refusal was met (i.e. the sampler did not penetrate 6-inches into the soil after 50 blows by the hammer), the N-value is recorded as the inches driven per 50 blows (ex: 50 blows for 4-inches), rather than the number of blows taken to drive the sampler the final 12- of 18-inches. For these reasons, I did not include N-values for samples that hit refusal. Only 14 of the 196 SPT and D&M samples were collected during mud rotary drilling, and were not met with refusal. These 14 samples provide the most reliable N-value data.

5.1.3 *Laboratory Test Results*

I used the information provided in the boring logs from previous investigations and their corresponding geotechnical reports to obtain laboratory test results of select Qva samples (King County, 2002; King County, 2004; GeoEngineers, 2015). The tests on these samples

³ Standard penetration resistance (N-value) is the number of blows it takes a 140 pound hammer, free-falling 30 inches, to drive a 2-inch OD SPT sampler the final 12- of 18-inches. The N-values provide a general understanding of the resistance to penetration and is a measure of the relative soil density.

were completed using the following standards: ASTM D 1140, for Percent Fines⁴ Determination; ASTM D 422 for Grain Size Distribution; ASTM D 2216, for Moisture Content Determination; and ASTM D 2487, for Classification of Soils.

5.1.4 Groundwater Conditions

My analysis of the groundwater conditions is limited by the amount of data that were readily available. I used information from the boring logs and geotechnical reports to obtain groundwater elevations. Some of the groundwater data in the boring logs and geotechnical reports were based on observations while drilling, while other data were measured with a vibrating wire piezometer (VWP), pressure transducer, or other measuring instruments. The approximate groundwater elevations, dates of record, and methods of measurement are annotated on the cross-sections (Figures 4-8) where data was available.

Single-well field hydraulic conductivity tests (slug tests) were performed at two wells in the study area, BW-6 and LLE-B11P, by Shannon & Wilson for the Brightwater Project and by GeoEngineers for the ST-LLE Project, respectively. The semi-log plots of water level versus time for the BW-6 slug test were provided in the King County (2002) geotechnical report. However, no interpretation or analysis of the test was provided.

5.2 Data Analysis

I used scatter plots to examine if the density, fines-content, and moisture content directly influenced each other. I plotted the N-values obtained during mud rotary drilling (see *Section 5.1.2*) against various other variables (percent fines, depth, and elevation) to determine if any of these variables directly influence the density of the soil. As a comparison, I evaluated the N-values obtained during mud rotary, Becker hammer, and hollow-stem auger drilling methods. I also evaluated moisture content and percent fines in relation to each other, to elevation, and to depth from ground surface. In addition, I conducted a volumetric analysis of the soil classifications within the Qva for all 27 borings.

5.3 Modeling

One of the main purposes of this research is to determine the three-dimensional (lateral and vertical) extent of the Qva. To accomplish this, I used *ArcGIS* and *EnterVol* to add a third dimension to previously completed two-dimensional studies. The following sections describe the methods that were used to generate a cross-section for each of the five transect lines, a block diagram, a fence diagram, and an isopach map of the Qva thickness. These models help illustrate the spatial variability of the Qva in the study area.

⁴ Fine grained sediments are defined by ASTM D 1140 as material finer than 75 μm , or as particles that can pass through a No. 200 sieve.

5.3.1 Cross-Sections

I used borehole data and various extensions of *ArcGIS* to create five cross-sections. The purpose of creating the cross-sections is to illustrate the spatial variability in two-dimensions along a transect line. I began this phase of modeling by marking and exporting the location of each borehole and transect line in *Google Earth* (Figure 1). I then collaborated with Gene Lohrmeyer at GeoEngineers to complete the following steps using *ArcGIS*. We first imported the locations of the borings and transect lines in *ArcGIS*. We then created a shapefile for each borehole and transect line, which we projected to the *NAD 1983 State Plane Washington North (ft)* coordinate system. We used a 10-meter digital elevation model (DEM) from the U.S. Geological Survey as an elevation datum (U.S. Geological Survey, 2001). We then interpolated the lines using their positions along the DEM to create an elevation profile for each of the five transects. I then digitally drew the stratigraphy for each cross-section, using data from the boring logs to infer the subsurface geology. I did this for each of the five cross-sections, shown in Figures 4-8.

5.3.2 Three-Dimensional Models

I collaborated with Gene Lohrmeyer at GeoEngineers to create block and fence diagrams, and an isopach map of the Qva, using *EnterVol*, which is an extension of *ArcGIS*.

To create the block diagram, we first georeferenced the borehole locations to the *NAD 1983 State Plane Washington North (ft)* coordinate system in *ArcGIS-ArcScene*, ESRI's three-dimensional viewing platform, and exported the data into our *EnterVol* map. We then created two new models to define the lateral and vertical extents of the study area. The first model used a shapefile with an xy-grid to define the two-dimensional (lateral) extent of the area to be analyzed. Next, we used elevation data to define the three-dimensional (vertical) extent of the area to be modeled; the top of the borings served as the upper extent and the bottom of the borings as the lower extent. We then added a third model to assign stratigraphic values to the three-dimensional model, based on the borehole data in Table 2. We automated the block diagram in *EnterVol* using an inverse distance weight algorithm and the three models described above. We then created a fence diagram by making slices of the block diagram along each of the five transect lines.

We also created an isopach map of the unit thickness. We did this by first isolating the Qva in the block diagram. Then we converted this segment of the diagram to a point cloud, and saved it as a shapefile. Next, we opened the shapefile in *ArcMap*, and added x and y values to the points. We used the natural neighbor interpolation to create a surface from these points based on elevation, using a 10 m grid cell size to match the DEM used in earlier steps. Finally, we converted the interpolated raster surface to 25 ft vector contours to illustrate the thickness of the Qva, as modeled by *EnterVol*.

6.0 Observations

6.1 Spatial Distribution

The surface of the Qva in the study area is found at a maximum elevation of 477 ft at boring LLE-B17 and at a minimum elevation of 247 ft at boring E-108 (Table 2, Figures 4-8). The depth to the top of the Qva varies from 0 to 109.8 ft below ground surface (bgs). The bottom of the Qva ranges in elevation from 406 ft at boring LLE-B17 to 178.5 ft at boring BW-4. The depth to the bottom of the Qva ranges from 53 to 279 ft bgs. The thickness of this unit varies from 21 to 242 ft in the three-dimensional models (Figures 9-12), and from 0 to 242 ft in the geotechnical boring logs (Table 1). The Qva thins to the east between borings BW-4 and BW-6, along Transect 1 (Figure 4). The thickness and continuity of the Qva is impacted by an apparent ridge at boring E-109, along Transect 2 (Figure 5). At this location the Qva pinches out, and pre-Fraser interglacial deposits (Qpn) are exposed at the surface. Along Transects 3 through 5, the Qva is thickest where the modern topography is high, and is thinnest where the topography is low (Figures 6-8, respectively).

The block diagram in Figure 9A shows the extent of the Qva, as modeled in *EnterVol*, and Figure 9B shows the block diagram with a vicinity map overlay for reference. There are discrepancies between the three-dimensional models created in *EnterVol* and the cross-sections (Figures 4-8). Similar to the cross-sections, the block and fence diagrams shows the Qva thinning to the east (Figures 9A and 10). However, there is no surface expression of the ridge that pinches out the Qva in the three-dimensional models. Similar to the cross-sections, the Qva in the block and fence diagrams is thickest where the topography is greatest (Figures 9A and 10). Figures 11A and 11B combine the Qva unit from the block diagram with the fence diagram to help illustrate the extent of the Qva in relation to other geologic units.

The three-dimensional models show that the Qva is thickest at the southwestern extent of the study area, as is illustrated in Figure 12. The Qva appears to thin to the east along Transects 1 and 2, to the north along Transect 3, and to the south along Transects 4 and 5 (Figure 12). The thickness of the Qva appear to decrease as a function of elevation along Transects 3, 4, and 5, which trend north-south. The Qva is thinnest in valleys and other topographic lows, and is thickest at modern topographic highs.

6.2 Physical Properties

6.2.1 Relative Density

The relative density of the Qva in the study area was evaluated based on review of SPT and D&M blow count data collected during the drilling of geotechnical borings. The relative density ranges from dense (N-value range: 30-50) to very dense (N-value: 50+), based on the 14 N-values collected during mud rotary drilling, described in *Section 5.2* (Appendix C). As a comparison, the relative density data collected during all drilling methods ranged from loose

(N-values range: 4-10) to very dense (N-values: 50+). There appears to be a slight correlation between density and fines content (Figures 13). I did not find a correlation between density and elevation (Figure 14). However, relative density appears to increase with depth from the ground surface (Figure 15). All N-Values recorded within the Qva can be found in the boring logs (Appendix B). Data regarding fines content, elevation, and depth can be found in the boring logs in Appendix B and in the borehole data summary sheet in Appendix C.

6.2.2 Grain Size

A wide range of grain sizes were recorded for the soil samples that had sieve analyses. The following is a summary of the content of soil samples that were collected from the Qva: gravel content ranged from 0 to 48.2%, the sand content ranged from 40.9 to 97.6%, and the fines content ranged from 1.3 to 36%. Silt and clay lenses are located throughout the Qva. The sample with the highest fines content (36%) was located 40 ft bgs and 7 ft below the top of the Qva, at boring LLE-10S. Results from sieve analyses can be found in Appendix C.

A volumetric analysis of the Qva show that this unit is composed of the following soil groups: 6.6% well-graded gravel (GW), 1.9% poorly-graded gravel (GP), 0.9% poorly-graded gravel with silt (GP-GM), 0.1% silty gravel (GM), 3.5% well-graded sand (SW), 2.7% well-graded sand with silt (SW-SM), 33% poorly-graded sand (SP), 37.3% poorly-graded sand with silt (SP-SM), 12.8% silty sand (SM), 0.9% lean silt (ML), and 0.3% lean clay (CL) (Figure 16A). The soil classifications follow ASTM D 2487, which is summarized in Appendix D. A volumetric analysis of the soil groups within the Qva of each boring is shown in Figure 16B and is summarized in Table 3.

I found a slight correlation between the fines content and the relative soil density (Figure 13). Additionally, all samples with greater than 10% fines content were dense to very dense. I did not find a correlation between fines content and depth nor elevation (Figure 17). Lab test results from sieve analysis and fines content determination can be found in the boring logs (Appendix B), and in the borehole data summary sheet (Appendix C).

6.2.3 Moisture Content

The natural moisture content of select samples from the Qva range from 3.0 to 35.4% natural moisture, by weight. There is a slight trend (exponential, $R^2 = 0.3226$) correlating depth from the ground surface to an increase in moisture content (Figure 18). Likewise, there is a slight linear trend ($R^2 = 0.2730$) correlating elevation and moisture content (Figure 18). The moisture content also appears to increase with an increase in fines content (Figure 19).

6.3 Groundwater Conditions

I found that the top of the water table in the Qva aquifer ranged from 231.9 to 458 ft in elevation, based on the 16 groundwater measurements that were recorded in the

geotechnical boring logs (Appendix B). Rising head and falling head slug tests⁵ were done at LLE-B11P on August 04, 2014. The results of the slug tests indicate that the geometric mean of hydraulic conductivity is 15.93 ft/d (5.62×10^{-3} cm/s), the storativity is 3.28×10^{-3} , and the estimated transmissivity is 738.58 ft²/d in the vicinity of this observation well (GeoEngineers, 2015). The groundwater at LLE-B11P changed from an elevation of 354.83 ft on August 12, 2014 to an elevation of 355.04 on March 13, 2015. During this time period, the lowest groundwater elevation was recorded at 354.74 ft on September 23, 2014, and the peak groundwater elevation was recorded on February 08, 2015 at 356.47 ft, for a difference of 1.73 ft between the summer and winter seasons. The groundwater elevation at LLE-B11P increased after significant precipitation events (Figure 20).

7.0 Analysis & Discussion

In this section I discuss the observations from this study in relation to data found in published literature (Table 4). I will also discuss how and why my findings deviate from published data on the Qva. These comparisons will help geologist, hydrogeologists, engineers, and environmental scientists conceptualize local variations in the Qva that may affect slope stability, groundwater dynamics, engineering properties, and migration of contaminants.

7.1 Spatial Distribution

In a previous study, it was determined that the Qva ranges from 50 to 200 feet in thickness in proximity to Lake Ballinger (Golder Associates, 2008). Additionally, Mullineaux *et al.* (1965) commented that glacial advance outwash is typically greater than 100 ft thick, and Troost and Booth (2008) found that the Qva ranges from absent to 400 ft thick. I found that the thickness of the Qva in the study area ranges from 0 to 242 ft. This is comparable to findings from published data (Table 4), although the thickness is greater than reported by Golder Associates (2008).

The elevation of the top of the Qva in the study area ranges from 247 to 477 ft. Troost and Booth (2008) found that the top of the Qva was deposited between elevations of 400 and 600 ft, and that the top of this unit is locally lower where subsequent erosion has occurred. At boring E-108, where the top of the Qva is at 247 ft elevation, there is a thick deposit of Qvr overlying the Qva (Figures 5 and 11A). This depression in the Qva may be an erosional feature formed by the advancing glacier, and later filled in with recessional outwash as the glacier retreated. It is also possible that this abnormality was caused by geomorphic or tectonic processes.

The automated three-dimensional models in this study show that the base of the Qva is not flat due to preexisting topography (Figures 4-9A). Troost and Booth (2008) found that the

⁵ I observed these tests, which were part of the ST-LLE project, while interning at GeoEngineers.

Qva fills paleotopographic valleys and channels, some of which are below sea level. The bottom elevation of the Qva in the study area ranges from 178 to 406 ft. Figures 11A-11B show the extent of the Qva in relation to other units.

The thickness of the Qva is greatest at topographic highs (Figures 4-8); Troost (2006) found this to be true over much of the Puget Lowland. This is likely due to a combination of pre-existing topography at the time of deposition and preservation from erosional forces. According to the models generated in this study, the topographic highs in the study area are capped by till, which is conceivably shielding the Qva from erosion. However, further explorations are needed to verify the location of the till in the study area.

7.1.1 Variability in Unit Thickness

The thickness of the Qva is impacted by an apparent ridge at boring E-109 on Transect 2 (Figure 5). There are two plausible explanations for the nature of this ridge. The first possibility is that the ridge represents a paleotopographic high, and that the Qva was either eroded or was never deposited at this location. Evidence for erosion includes the contact between Qvr and Qva, and the absence of Qvt at boring E-108 on Transect 2. A second hypothesis is that this ridge represents vertical displacement from a conjugate of the SWIF, which is proximal to the study area. Supporting evidence for this hypothesis includes: indications of off-set and movement recorded in boring logs, differences in stratigraphy between Transects 1 and 2, and geologic mapping of the SWIF close to the study site. Slickensides, fractures, and other indications of movement are documented in boreholes BW-4, BW-5, and BW-6, on Transect 1, and in E-105, MW-4, E-106, E-107, E-108, MW-5, E-109, E-110, and E-211 on Transect 2. Interglacial mass wasting deposits (Qpfnmw) are also recorded in boring E-108 on Transect 2 (Figure 5, Appendix B). Slickensides represent past shearing displacement between two surfaces, and may indicate faulting, persistent landslide movement, or stress relief from isostatic rebound as the result of glacial ice melting (Miller, 1989). The slickensides found in this area could have formed under any one of the three conditions listed above, or by a combination of those conditions. However, it is also possible that the slickensides, which were recorded in the Qvlc, Qpn, and Qpg, were created while drilling. The stratigraphic relationships along Transects 1 and 2 are not consistent (Figures 4 and 5, respectively). Transect 1 shows that the bottom of the Qva is in contact with Qvlc and Qpg, and that Qpg overlies Qpn. However, in Transect 2, the bottom of the Qva is in contact with Qvlc and Qpn, which overlie Qpg. The ages of the Qpn and Qpg have not been determined, so it is unclear if the stratigraphic relationship between the Qpn and Qpg in these two transects is undisturbed, or if it represents an unconformity or off-set. Finally, Sherrod *et al.* (2008) identified lineaments of the SWIF close to the study site using magnetic and gravity anomalies. However, the exact locations of the lineaments are not well-constrained. In summary, there are two possibilities to define the nature of the ridge that the

Qva pinches out against. However, further research is needed to determine the nature of this ridge.

7.2 Physical Properties

The relative density of the 14 Qva samples measured in the study area are consistent with values published by Glaster and Laprade (1991). I found that the density of the Qva generally increases with increasing fines content. Theoretically, this makes sense. “Clean” sands could have a lower blow count than “dirty” sands, because fine-grained sediments can fill void space and give the sands cohesion. However, the sample size that I used in this study was limited, and this correlation may be due to random chance. I also found that the density of the Qva generally increases with depth. This correlation can be explained by the increase in compressive forces on the sediment with depth, making them more compact. However, not all of the very dense (N-value of 50+) soil samples contained a significant percentage of fine-grained material, and some of the samples were located near the surface. Therefore, factors other than fines content and depth influence the density of the Qva. Weathering, bioturbation, stress relief, and downslope movement are a few factors that may reduce soil density, while cobbles and boulders may prevent the sampler from advancing or may increase blow counts.

I found that the Qva in the study area is composed of about 89.3% sand, 9.5% gravel, 0.9% silt, and 0.3% clay. I did not find a correlation between fines content and depth. Despite the coarsening-up facies model of the Qva, I would not expect there to be a correlation between fines content and depth because the depositional environments (high-energy braided streams with subaqueous termini) were dynamic and complex; therefore, the sediments were not uniformly distributed based on grain size.

I found that the natural moisture content of samples collected for the Qva in the study area increase with an increase in fines content and depth, and a decrease in elevation. I would expect these correlations for the following reasons: the fines content likely aids in water retention via adsorption and cohesion, and moisture content probably increases with depth and decreases with elevation as the result of gravity and proximity to groundwater.

7.3 Groundwater Conditions

Variations in the top and bottom elevations of the Qva will influence the flow patterns of groundwater. The thickness of the Qva may also influence the depth to water (see boring E-107, Figure 5). I found that the saturated thickness of the Qva ranges from 0 to 102 ft within the study area. This is comparable to the findings in Golder Associates (2008), which states that the saturated thickness ranges from 10 to 100 ft in the Mountlake Terrace area (Table 4). I found that the depth to the saturated Qva aquifer ranged from 7 to 221 ft bgs in the study area. It was reported in the King County (2002) geotechnical report that the groundwater

elevation varies, although they found that soils 20 to 70 ft bgs were generally saturated (King County, 2002).

Golder Associates (2008) states that the potential for infiltration is good in areas where the Qva is exposed at the surface and a sufficient unsaturated thickness exists. At boring LLE-B11P, the Qva is exposed at the ground surface. However, the groundwater was recorded at 7.4 ft bgs on August 12, 2014. The Qva at this location is 46.5 ft thick; therefore, approximately 84% of the total thickness of the unit is saturated at LLE-B11P. The groundwater elevation at boring LLE-B11P peaked following large precipitation events (Figure 20). This suggests that the aquifer is responding to meteoric water. However, the seasonal variations in water level are less than 2 ft at this location. This low seasonal flux is likely attributed to the aquifer being semi-confined, either by silt lenses within the Qva or by a nearby Qvt cap. The Qva at this location ranges in group classification from SP-SM to SM; the silt content may attribute to the semi-confined aquifer conditions. These data were recorded between August 12, 2014 and February 08, 2015; a longer study may show larger seasonal variation in groundwater flux.

8.0 Conclusions

The characteristics of the Qva make it an important geologic unit. The Qva has high frictional shear strength and low compressibility, which provides good support for foundations and other developments (Gurtowski and Boirum, 1989). The Qva is also an important hydrostratigraphic unit because it forms an extensive, unconfined aquifer (Golder Associates, 2008). However, the Qva is also susceptible to slope instability, erosion, seepage, and contamination. Studying the spatial distribution and physical properties of the Qva will benefit geologists, hydrogeologists, engineers, and environmental scientists with respect to decision making, prediction, and mitigation.

Understanding the spatial distribution of the Qva is significant to geologists, hydrogeologists, engineers, and environmental scientists. The contact between the Qva and the Qvlc is a known “slip-surface” for several large landslides in the Seattle Area (Tubbs, 1974), so documenting the location of this contact is important for geologists working on slope stability issues. Additionally, anomalies in the Qva, such as the one found at boring E-109, may provide insight to other geologic concerns. The top and bottom elevations of the Qva significantly influence the flow patterns of groundwater, and consequently, the migration of any contaminants that leach into the groundwater. Likewise, the variability in thickness of the Qva will directly affect the hydraulic conductivity, storativity, and transmissivity of groundwater in the aquifer. Knowledge of the spatial distribution of the Qva is also significant to engineers for the purposes of construction design and feasibility planning.

The spatial distribution of the Qva in the study area is comparable to generalized published data for the Qva across the Puget Lowland. The top of the Qva ranges in elevation from 247

to 477 ft. This is consistent with Troost and Booth (2008), which states that the top of the Qva ranges from 400 to 600 ft in elevation, and at lower elevations where there has been subsequent erosion. The thickness of the Qva ranges from 0 to 242 ft in the study area, which is within the limits of Qva measured in other studies (Table 4). The three-dimensional models generated using *EnterVol* show that the Qva is not a homogeneous unit in terms of lateral and vertical distribution (Figures 9A-11B). The Qva thins to the east in Transect 1 (Figure 4), and pinches out against a ridge in Transect 2 (Figure 5). In addition to locally affecting groundwater flow patterns and aquifer properties, this apparent anomaly in the Qva on Transect 2 may have broader geologic implications relating to paleotopography or a regional fault system.

The physical properties (density, grain size distribution, and moisture content) of the Qva also have significant implications on the geology, hydrogeology, engineering, and environmental sciences. For example, the hydraulic conductivity of the Qva aquifer will be directly affected by the porosity of the soil it is traveling through. Additionally, silt and clay lenses within the Qva may create perched aquifers or cause seepage, which creates the potential for issues relating to slope stability and erosion. The density and grain size distribution will also affect engineering properties, such as excavatability, angle of repose, and cohesion.

I found that the relative density of samples analyzed in this study ranged from loose to very dense (Appendix C), with the loose end of the range probably resulting from heave. I found that all samples that had greater than 10% fines content were either dense or very dense (Figure 13). Although the Qva is primarily composed of sand, I found that this unit contains an assortment of grain sizes. Individual soil samples contained as much as 48.3% gravel, 97.6% sand, and as much as 36% fine sediment (Appendix C). A volumetric analysis of the soil groups show that the Qva is composed of the following classifications, listed from greatest to least volume: 37.3% SP-SM, 33% SP, 12.8% SM, 6.6% GW, 3.5% SW, 2.7% SW-SM, 1.9% GP, 0.9% GP-GM, 0.9% ML, 0.3% CL, and 0.1% GM (Figures 16A-B). I also found that the natural moisture content from samples collected in the study area range from 3.0 to 35.4%, and increase as a function of depth and fines content (Figures 18-19).

Groundwater dynamics considerably influence the geology and engineering properties of the Qva. Elevated pore-pressures caused by large precipitation events are known to destabilize slopes (Tubbs, 1975). Groundwater also affects the feasibility of developing in the Qva. Although groundwater data were scarce, I found that the elevation of the water table in the Qva ranges from 231.9 to 458 ft. The saturated thickness of the Qva (0-102.13 ft) is comparable to findings from other investigations (Table 4). However, I found that the depth to the saturated aquifer (7.59-221.3 ft bgs) was much greater in the study area than was reported in the King County (2002) geotechnical report, which states that the depth to groundwater is generally 20-70 ft bgs. The results of rising- and falling-head slug tests at

boring LLE-B11P determined the following groundwater characteristics at this location: mean hydraulic conductivity (15.93 ft/d), storativity (3.28×10^{-3}), and transmissivity (738.58 ft²/d). The seasonal variation in groundwater elevation at boring LLE-B11P was 1.73 ft, as recorded between August 12, 2014 and March 13, 2015. The Qva at this location is silty. The silt content at this location could be partially confining the aquifer, and thus minimizing the seasonal flux. However, data from a pressure transducer installed at this location shows that the aquifer is responding to precipitation events.

Subsurface models can be used to gain a better understanding the relationships between geologic units. In this study, I used *EnterVol* to try to model the spatial variability of the Qva. While this program has some limitations, I found that it useful for creating three-dimensional models that illustrate the relationships between the Qva and the other geologic units. Overall, *EnterVol* produced what I needed, and I would recommend it for other subsurface studies.

9.0 Limitations and Assumptions

This study is limited to publicly available data, and the accuracy of those data. The data include, but are not restricted to: geotechnical boring logs, geologic maps, technical reports, memorandums, aerial photographs, and geospatial data.

Although the available subsurface data were sufficient, this study could have benefited from additional borehole data. Transects 1, 2, and 5 are the most reliable, because these transects have a greater concentration of borehole data. Transects 3 and 4 incorporate only 2 boreholes each, leaving much more room for interpretation. Additionally, groundwater data were not as abundant nor as readily available as I had anticipated; my analysis could have been made stronger by additional data in this field.

I assume in my analysis of soil density that the blow counts recorded using SPT and D&M sampling methods are correlative, although the hammer weight, sampler size, and sample depth may affect the consistency of the data.

The final product of this research project will be applicable to the Qva only within the study area, and should not be considered representative of the Qva elsewhere in the Puget Lowland. However, the methods used in this study could be applied to other investigations.

9.1 Software Limitations

While the usability and viability of *EnterVol* was satisfactory for this study, the software has some limitations. This program automates subsurface models based on borehole data input by the user. However, the ability for the user to make interpretations or add corrections to the models is somewhat limited. This issue was encountered in my models, at boring E-109. Although the data that I input showed that the Qpn was exposed at the surface in boring E-

109, the automated model showed the Qva at the surface instead. Additionally, the isopach map (Figure 12) show “bulls-eye” patterns; this is an unusual geologic pattern that might actually be an artifact of the interpolation algorithm used to make the models. It is possible that the geology was too complex to be modeled in this area. It is also possible that the algorithm used to interpolate the geology did not capture every fine detail. Discrepancies between the actual and modeled values of the thickness of the Qva could have been reduced if more borehole information were available. “Dummy” borings can be used as an aid to increase the user’s ability to make interpretations, or to fill in voids where borehole information is scarce; however, I did not use this approach because I wanted to compare the automated models to the hand-drawn cross-sections. I am uncertain of the complexity of the models that *EnterVol* is able to produce. I am also uncertain of the full capabilities of this program, which should be explored in future studies.

10.0 Recommended Future Studies

10.1 Local Characterization of the Vashon Advance Outwash

Additional studies should be conducted to compare the engineering properties (bulk density, coefficient of friction, cohesion, etc.) of the Qva locally to that of generalized published data (Table 4). Additional engineering properties that could be tested include: triaxial shear strength, residual strength, dry and wet densities, and stability of cut slopes. It would also be useful to determine the angle of internal friction with relation to fines content. The angle of internal friction is significantly less in silts than it is in sands (Koloski *et al.*, 1989). This information would be useful to engineers who have project designs in the Qva, so that they can determine how the fines content may affect the stability of a slope or excavation.

10.2 Locating Southern Whidbey Island Fault Traces

There is potential evidence for a segment of the Southern Whidbey Island Fault in Transect 2, where the Qva pinches out along a ridge. This hypothesis is supported by the presence of slickensides, brecciated textures, shear zones, and mass wasting deposits along the same transect (Figure 5). However, locating strands of the SWIF was not in the scope of my research, so I did not investigate this in great detail. I recommend that future studies be done using new and existing geotechnical borings, and the *ArcGIS/EnterVol* modeling methods described above, to locate strands of the SWIF near the King/Snohomish County border or elsewhere. Future research should also incorporate geophysical methods to identify and locate the fault, should it exist here. Ideal locations for the geophysical research would be in the Holyhood Cemetery, which intersects Transect 2, and at the Nile Golf Course, which is located between Lake Ballinger and Transect 5. These areas are minimally developed, and will not have much interference from underground utilities. Since the SWIF has obscure boundaries and is an active fault capable of producing up to Mw 7.1 earthquakes, I think it is in the best interest of the community to locate and constrain the lineaments of this fault.

11.0 References Cited

- American Society for Testing and Materials, 1985, D 422, Standard Test Method for Particle-Size Analysis of Soils: Annual Book of ASTM Standards, v. 04.08, p. 8.
- American Society for Testing and Materials, 1985, D 1140, Standard Test Methods for Determining the Amount of Material Finer than 75- μm (No. 200) Sieve in Soils by Washing: Annual Book of ASTM Standards, v. 04.08, p. 6.
- American Society for Testing and Materials, 1985, D 1586, Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils: Annual Book of ASTM Standards, v. 04.08, p. 9.
- American Society for Testing and Materials, 1985, D 1587, Standard Practice for Thin-Walled Sampling of Soils for Geotechnical Purposes: Annual Book of ASTM Standards, v. 04.08, p. 4.
- American Society for Testing and Materials, 1985, D 2216, Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass: Annual Book of ASTM Standards, v. 04.08, p. 7.
- American Society for Testing and Materials, 1985, D 2487, Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System): Annual Book of ASTM Standards, v. 04.08, p. 11.
- Atwater, B. F. and Moore, A. L. (1992). A Tsunami About 1000 Years Ago in Puget Sound, Washington. *Science*, v. 258, p. 1614-1617.
- Atwater, B. F., Nelson, A. R., Clague, J. J., Carver, G. A., Yamaguchi, D. K., Bobrowsky, P. T., Bourgeois, J., Darienzo, M. E., Grant, W. C., Hemphill-Haley, E., Kelsey, H. M., Jacoby, G. C., Nishenko, S. P., Palmer, S. P., Peterson, C. D., & Reinhart, M. A. (1995). Summary of coastal geologic evidence for past great earthquakes at the Cascadia subduction zone. *Earthquake spectra*, 11(1), 1-18.
- Barnett, E. A., Haugerud, R. A., Sherrod, B. L., Weaver, C. S., Pratt, T. L., & Blakely, R. J. (2010). *Preliminary atlas of active shallow tectonic deformation in the Puget Lowland, Washington*. U. S. Geological Survey.
- Bjerg, P. L., & Christensen, T. H. (1993). A field experiment on cation exchange-affected multicomponent solute transport in a sandy aquifer. *Journal of contaminant hydrology*, 12(4), 269-290.

- Blakely, R. J., Sherrod, B. L., Wells, R. E., Weaver, C. S., McCormack, D. H., Troost, K. G., & Haugerud, R. A. (2004). *Cottage Lake Aeromagnetic Lineament: A Possible Onshore Extension of the Southern Whidbey Island Fault, Washington*. US Department of the Interior, US Geological Survey.
- Blakely, R. J., Wells, R. E., Weaver, C. S., & Johnson, S. Y. (2002). Location, structure, and seismicity of the Seattle fault zone, Washington: Evidence from aeromagnetic anomalies, geologic mapping, and seismic-reflection data. *Geological Society of America Bulletin*, 114(2), 169-177.
- Booth, D. B. (1994). Glaciofluvial infilling and scour of the Puget Lowland, Washington, during ice-sheet glaciation. *Geology*, 22(8), 695-698.
- Booth, D. B., Cox, B. F., Troost, K. G., & Shimel, S. A. (2004). Draft Composite Geologic Map of the Sno-King Area, 1:24,000. Map.
- Booth, D. B., Troost, K. G., Clague, J. J., & Waitt, R. B. (2003). The Cordilleran ice sheet. *Developments in Quaternary Sciences*, 1, 17-43.
- Brocher, T. M., Blakely, R. J., Wells, R. E., Sherrod, B. L., & Ramachandran, K. (2005, December). The transition between NS and NE-SW directed crustal shortening in the central and northern Puget Lowland: New thoughts on the southern Whidbey Island Fault. In *AGU Fall Meeting Abstracts* (Vol. 1, p. 06).
- City of Mountlake Terrace. Mountlake Terrace Municipal Code: Title 16 Environment, Chapter 16.15 Critical Areas. City of MLT, Apr. 2015. Web. Accessed March 2015. <http://www.codepublishing.com/WA/MountlakeTerrace/>
- Coe, J. A., Michael, J. A., Crovelli, R. A., Savage, W. Z., Laprade, W. T., & Nashem, W. D. (2004). Probabilistic assessment of precipitation-triggered landslides using historical records of landslide occurrence, Seattle, Washington. *Environmental & Engineering Geoscience*, 10(2), 103-122.
- Easterbrook, D. J. (1969). Pleistocene chronology of the Puget Lowland and San Juan Islands, Washington. *Geological Society of America Bulletin*, 80(11), 2273-2286.
- Clague, J. J., & James, T. S. (2002). History and isostatic effects of the last ice sheet in southern British Columbia. *Quaternary Science Reviews*, 21(1), 71-87.
- Galster, R. W., & Laprade, W. T. (1991). Geology of Seattle, Washington, United States of America. *Bulletin of the Association of Engineering Geologists*, 28(3), 235-302.

- GeoEngineers, Inc. Geotechnical Considerations Report: Lynnwood Link Extension Project. Seattle, WA: *GeoEngineers*, 2015. Print.
- Golder Associates. Technical Memorandum: Hydrogeologic Conditions, Greater Hall Lake, Hall Creek, Chase Lake, Echo Lake, Lake Ballinger and McAleer Creek Watershed. Redmond, WA: *Golder Associates*, 2008.
- Google Inc. Google Earth Pro (Version 7.1) [Software]. (2012). Available from: <https://www.google.com/earth/download/gep/agree.html>
- Gurtowski, T. M., & Boirum, R. N. (1989). Foundations and Excavations for High-Rise Structures in Downtown Seattle. *Engineering Geology in Washington, Volume II*. Ed. William T. Laprade and William D. Evans, Jr. Olympia, WA: Washington Dept. of Natural Resources Division of Geology and Earth Resources. p. 651-665. Print.
- Haugerud, R.A. (2004). Cascadia - Physiography, U.S. Geological Survey Geological Investigation Series I-2689, scale: 1:2,000,000. Web. Accessed May 2015. <http://pubs.usgs.gov/imap/i2689>
- Johnson, S. Y., Blakely, R. J., Stephenson, W. J., Dadisman, S. V., & Fisher, M. A. (2004). Active shortening of the Cascadia forearc and implications for seismic hazards of the Puget Lowland. *Tectonics*, 23(1).
- Johnson, S. Y., Dadisman, S. V., Childs, J. R., & Stanley, W. D. (1999). Active tectonics of the Seattle fault and central Puget Sound, Washington—Implications for earthquake hazards. *Geological Society of America Bulletin*, 111(7), 1042-1053.
- Johnson, S. Y., Potter, C. J., Miller, J. J., Armentrout, J. M., Finn, C., & Weaver, C. S. (1996). The southern Whidbey Island fault: an active structure in the Puget Lowland, Washington. *Geological Society of America Bulletin*, 108(3), 334-354.
- King County. Dept. of Natural Resources. Wastewater Treatment Division. "CSI Geotechnical Data Report, Brightwater Project Conveyance System." *King County*, May 2002. PDF.
- King County. Dept. of Natural Resources. Wastewater Treatment Division. "Predesign Geotechnical Data Report, Conveyance System." *King County*, February 2004. PDF.
- Koloski, J. W., Schwarz, S. D., & Tubbs, D. W. (1989). Geotechnical properties of geologic materials. *Washington Division of Geology and Earth Resources Bulletin*, 78.
- Lamb, A. P., Liberty, L. M., Blakely, R. J., Pratt, T. L., Sherrod, B. L., & van Wijk, K. (2012). Western limits of the Seattle fault zone and its interaction with the Olympic Peninsula, Washington. *Geosphere*, 8(4), 915-930.

- Laprade, W. T., and Robinson, R. A. (1989). Foundation and Excavation Conditions in Washington. *Engineering Geology in Washington, Volume I*. Ed. Richard W. Galster. Olympia, WA: Washington Dept. of Natural Resources Division of Geology and Earth Resources. p. 19-26. Print.
- Lasmanis, R. (1991). The geology of Washington. *Rocks and Minerals*, 66(4), 262-277.
- Liberty, L. M., and Pape, K.M. (2006) Seismic characterization of the Seattle and southern Whidbey Island fault zones in the Snoqualmie River Valley, Washington.
- Miller, J. A. (1989). Landslide Stabilization in an Urban Setting, Fautleroy District, Seattle, Washington. *Engineering Geology in Washington, Volume II*. Ed. William T. Laprade and William D. Evans, Jr. Olympia, WA: Washington Dept. of Natural Resources Division of Geology and Earth Resources. p. 681-690. Print.
- Moses, L. J. (2013). The Geology of Washington State. Olympia, WA. Washington State Department of Natural Resources. PDF.
- Moses, L. J. (2008). The Ross Point landslide: An instrumental record of landslide reactivation. *Reviews in Engineering Geology*, 20, 167-181.
- Mullineaux, D. R., Waldron, H. H., & Rubin, M. (1965). Stratigraphy and Chronology of Late Interglacial Time in the Seattle Area, Washington. *Geological Survey Bulletin*, 1194-0.
- Munch, J. H., & Killey, R. W. (1985). Equipment and methodology for sampling and testing cohesionless sediments. *Groundwater Monitoring & Remediation*, 5(1), 38-42.
- Nedimović, M. R., Hyndman, R. D., Ramachandran, K., & Spence, G. D. (2003). Reflection signature of seismic and aseismic slip on the northern Cascadia subduction interface. *Nature*, 424(6947), 416-420.
- Nelson, A. R., Personius, S. F., Sherrod, B. L., Kelsey, H. M., Johnson, S. Y., Bradley, L. A., & Wells, R. E. (2014). Diverse rupture modes for surface-deforming upper plate earthquakes in the southern Puget Lowland of Washington State. *Geosphere*, GES00967-1.
- Nielsen, D. M. (Ed.). (2005). *Practical handbook of environmental site characterization and ground-water monitoring*. CRC press.
- Porter, S. C., & Swanson, T. W. (1998). Radiocarbon age constraints on rates of advance and retreat of the Puget lobe of the Cordilleran ice sheet during the last glaciation. *Quaternary Research*, 50(3), 205-213.

- Pratt, T. L., Johnson, S., Potter, C., Stephenson, W., & Finn, C. (1997). Seismic reflection images beneath Puget Sound, western Washington state: The Puget Lowland thrust sheet hypothesis. *Journal of Geophysical Research: Solid Earth (1978–2012)*, 102(B12), 27469-27489.
- Savage, W. Z., Morrissey, M. M., & Baum, R. L. (2000). *Geotechnical properties for landslide-prone Seattle; area glacial deposits* (No. 2000-228). US Department of the Interior, US Geological Survey: Open File Report 00-228, 2000.
- Sherrod, B. L., Blakely, R. J., Weaver, C. S., Kelsey, H. M., Barnett, E., Liberty, L., Meagher, K. L., & Pape, K. (2008). Finding concealed active faults: Extending the southern Whidbey Island fault across the Puget Lowland, Washington. *Journal of Geophysical Research: Solid Earth (1978–2012)*, 113(B5).
- Schuster, J. Eric. Compiler. *Geologic Map of Washington*. Map. 1:2,252,800, Olympia, WA: Washington State Department of Natural Resources, 2013. Web.
- Thorson, R. M. (1979). Ice-sheet glaciation of the Puget Lowland, Washington, during the Vashon Stage (late Pleistocene). *Quaternary Research*, 13(3), 303-321.
- Troost, K. G. (2006). Spatial predictability of Quaternary deposits in the central Puget Lowland. *Proceedings, SAGEEP Conference, April 2006*, Seattle, WA. p 260-273.
- Troost, K. G, and Booth, D. B. (2008). Geology of Seattle and the Seattle area, Washington. *Landslides and Engineering Geology of the Seattle, Washington Area: Geological Society of America Reviews in Engineering Geology*. Ed. Baum, R. L., Godt, J. W., and Highland, L. M. Boulder, CO: The Geological Society of America, Inc., 20, p. 1-36.
- Troost, K.G., Booth, D.B., Wisher, A.P., and Shimel, S.A. (2005). The geologic map of Seattle – a progress report: U.S. Geological Survey Open-File Report, 2005-1252, scale 1:24,000. <http://pubs.usgs.gov/of/2005/1252/>
- Tubbs, D. W. (1974) *Landslides in Seattle*. Washington Division of Geology and Earth Resources Information Circular, 52 (1974): 15.
- Tubbs, D. W. (1975). *Causes, mechanisms and prediction of landsliding in Seattle* (Doctoral dissertation, University of Washington).
- U.S. Geological Survey (USGS). “Edmonds East 7.5’ Quad.” 10-meter Digital Elevation Model. U.S. Geological Survey: WA-DNR Number 1022 (2001). Accessed Feb. 2015. <http://gis.ess.washington.edu/data/raster/tenmeter/byquad/seattle/index.html>.

- Washington State. Dept. of Natural Resources. Washington Division of Geology and Earth Resources. *Earthquakes in Washington. Geologic Hazards and Mapping*. WA-DNR, 2015. Web. Feb. 2015. Accessed at <https://fortress.wa.gov/dnr/geology/?Theme=subsurf>
- Washington State. Dept. of Natural Resources. Washington Division of Geology and Earth Resources. (2015). *Subsurface Geology Information System. Washington State Geologic Information Portal*. T. J. Walsh, W. J. Gerstel, P. T. Pringle, and S. P. Palmer. Preparers. WA-DNR, 2015. Web. Feb. 2015.
- Wells, R. E., Weaver, C. S., & Blakely, R. J. (1998). Fore-arc migration in Cascadia and its neotectonic significance. *Geology*, 26(8), 759-762.
- Zhou, Y. (2006, Dec). *Soils and Foundations: Reference Manual – Volume I*. U.S. Department of Transportation, Federal Highway Administration. Publication No. FHWA NHI-06-088.

12.0 Figures

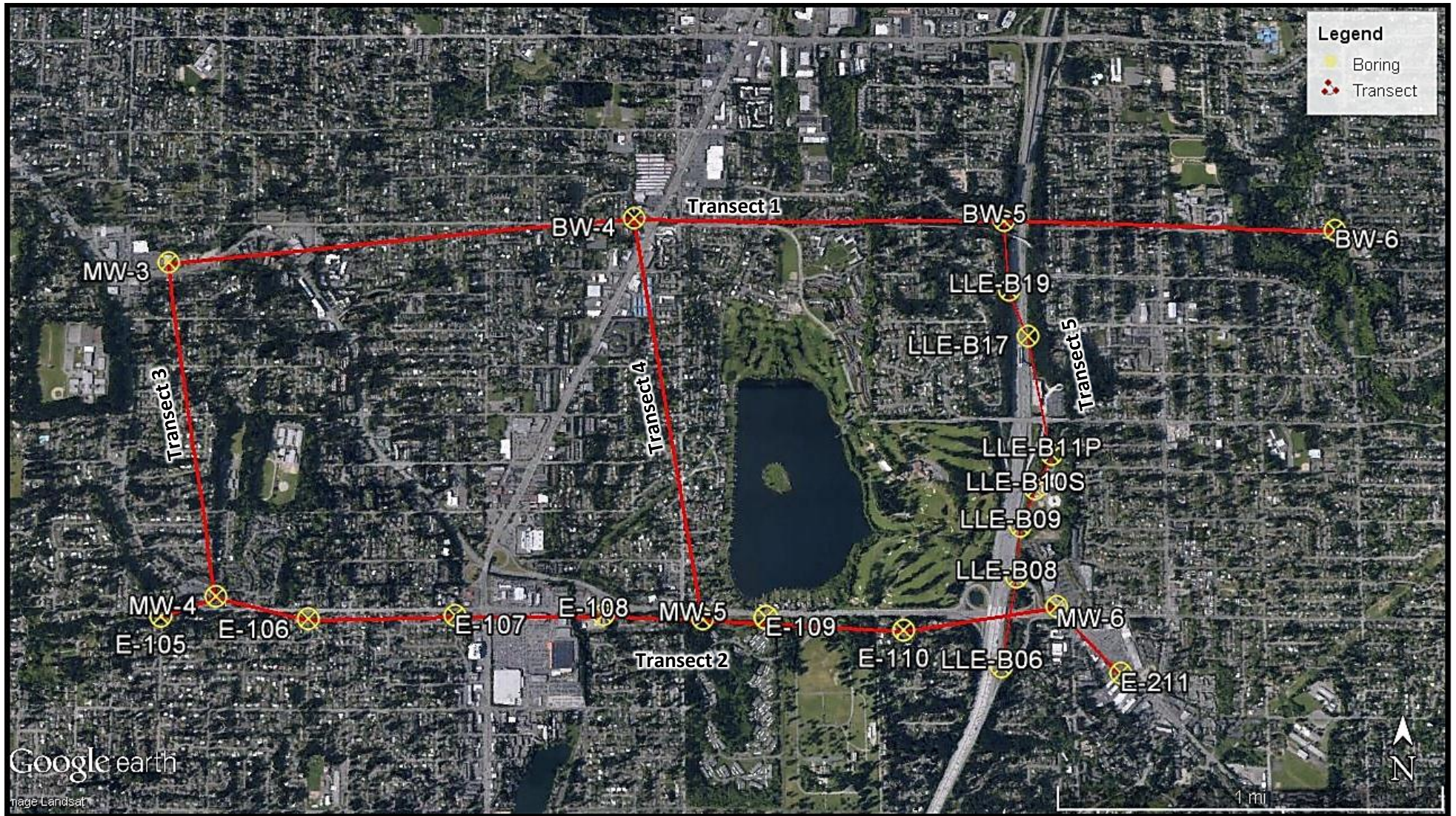


Figure 1, Vicinity Map. Transect lines are shown in red and significant borings are represented by a yellow cross in a circle. Borings prefixed with MW- were completed by HWA Geosciences for the Brightwater Project in 2002, with a BW- by Shannon & Wilson for the Brightwater Project in 2002, with an E- by CDM Smith for the Brightwater Project in 2003, and with an LLE-B by GeoEngineers for the Sound Transit-LLE Project in 2014. Lake Ballinger is located near the center of the study area. Interstate-5 is located at the eastern edge of the study area, along Transect 5.

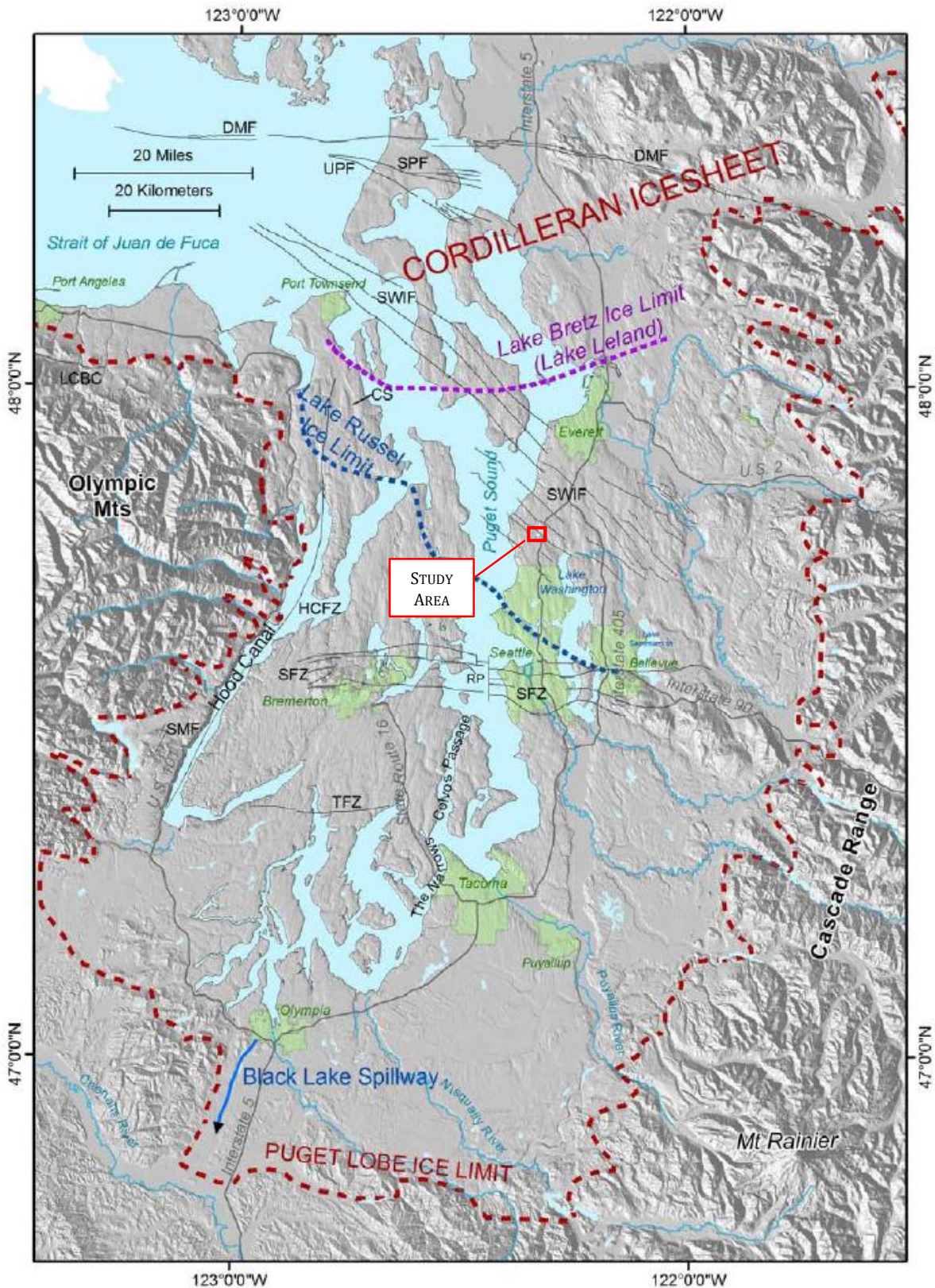


Figure 2, Regional Map. The study area is outlined in red. Highly populated cities are shown in green. The extent of the Puget Lobe of the Cordilleran Ice Sheet is indicated by the dark red, dashed line. Quaternary fault traces and lineaments are labeled and shown as gray lines. Notice the proximity of the Southern Whidbey Island Fault Zone (SWIF) to the study area. Other abbreviations listed on the map are: CS – Chimacum spillway, DMF – Devils Mountain fault zone, HCFZ – Hood Canal Fault Zone, LCBC – Lake Creek-Boundary Creek fault, RP – Restoration Point, SFZ – Seattle fault zone, SMF – Saddle Mountain fault, SPF – Strawberry Point fault, TFZ – Tacoma fault zone, and UPF – Utsalady Point fault. (Image modified from Troost and Booth, 2008)

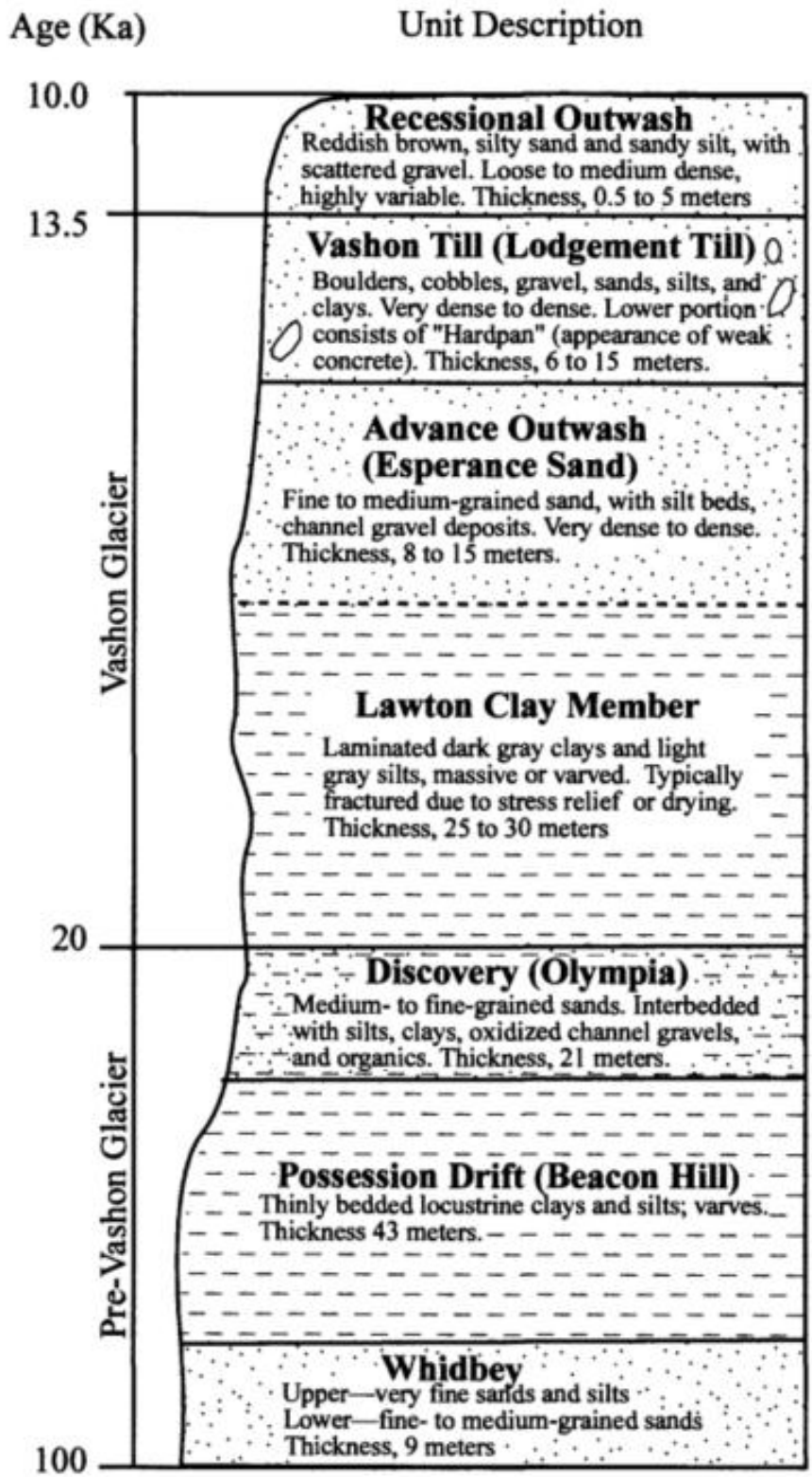


Figure 3, Generalized Quaternary Geologic Section. (Image from Galster and Laprade, 1991)

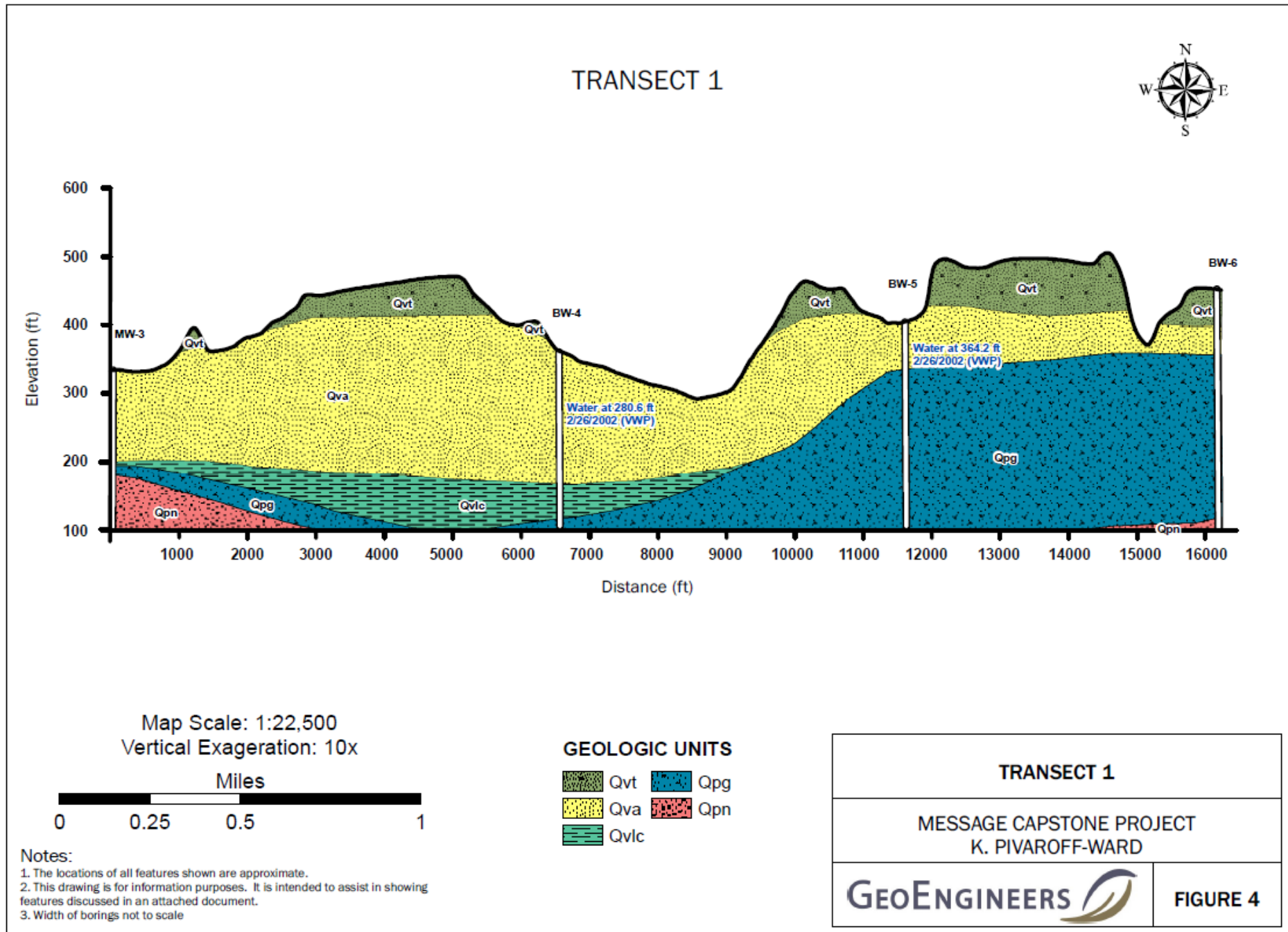


Figure 4, Cross-Section 1. Transect 1 is approximately 3.1 mi in length, and includes borings MW-3, BW-4, BW-5, and BW-6. The Qva thins to the east, where there appears to be a paleotopographic ridge composed of pre-Olympia glacial deposits.

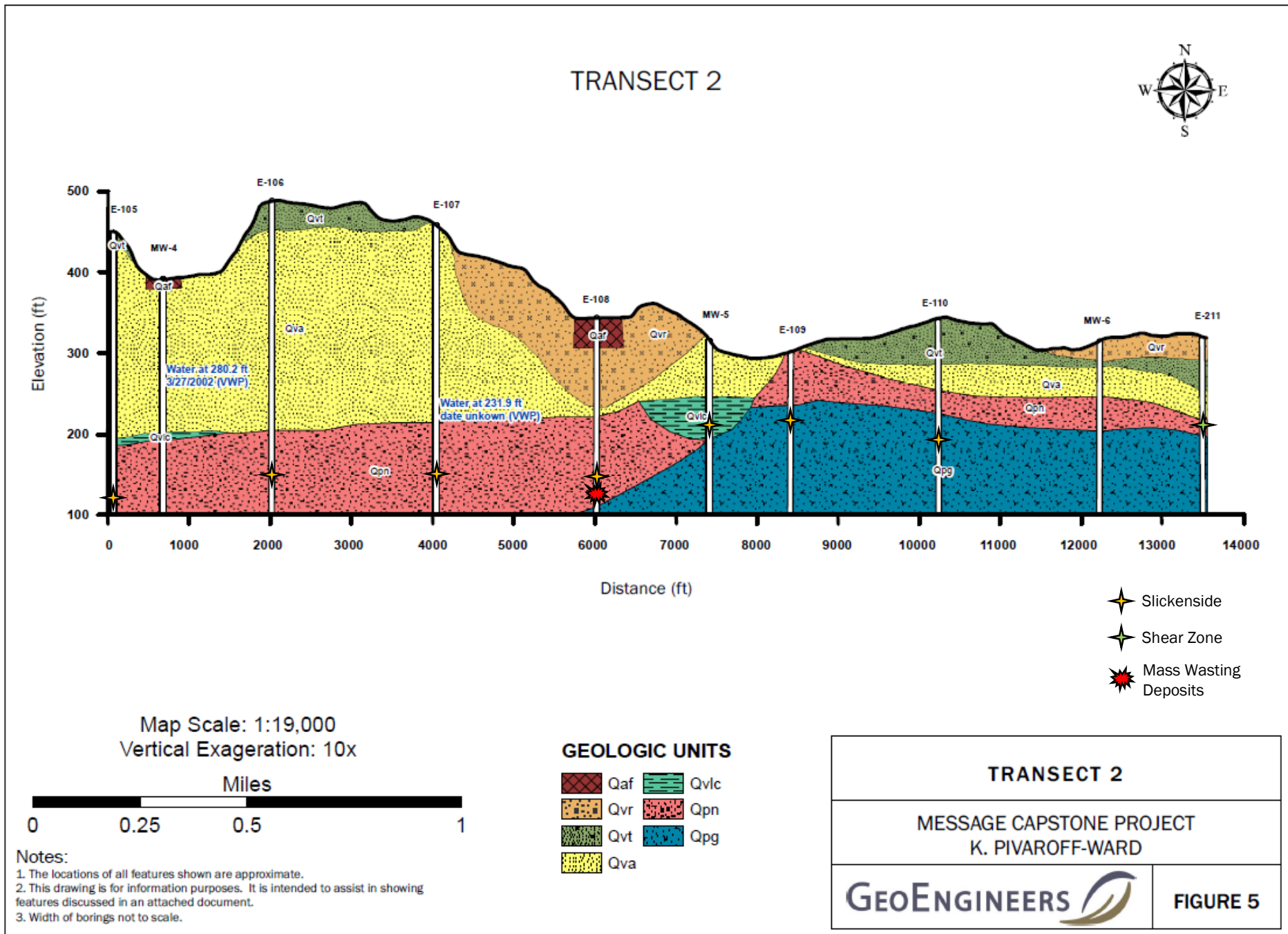


Figure 5, Cross-Section 2. Transect 2 is approximately 2.56 mi in length, and includes borings E-105, MW-4, E-106, E-107, E-108, MW-5, E-109, E-110, MW-6, and E-211. The Qva pinches out against a ridge composed of pre-Fraser interglacial deposits. There is significant evidence for soil disturbance along this transect, including slickensides, brecciated textures, shear zones, mass wasting deposits, and fractures. It is unclear whether this ridge is a paleotopographic feature, or whether it was formed from active tectonics. Further research should be done to determine the nature of this ridge.

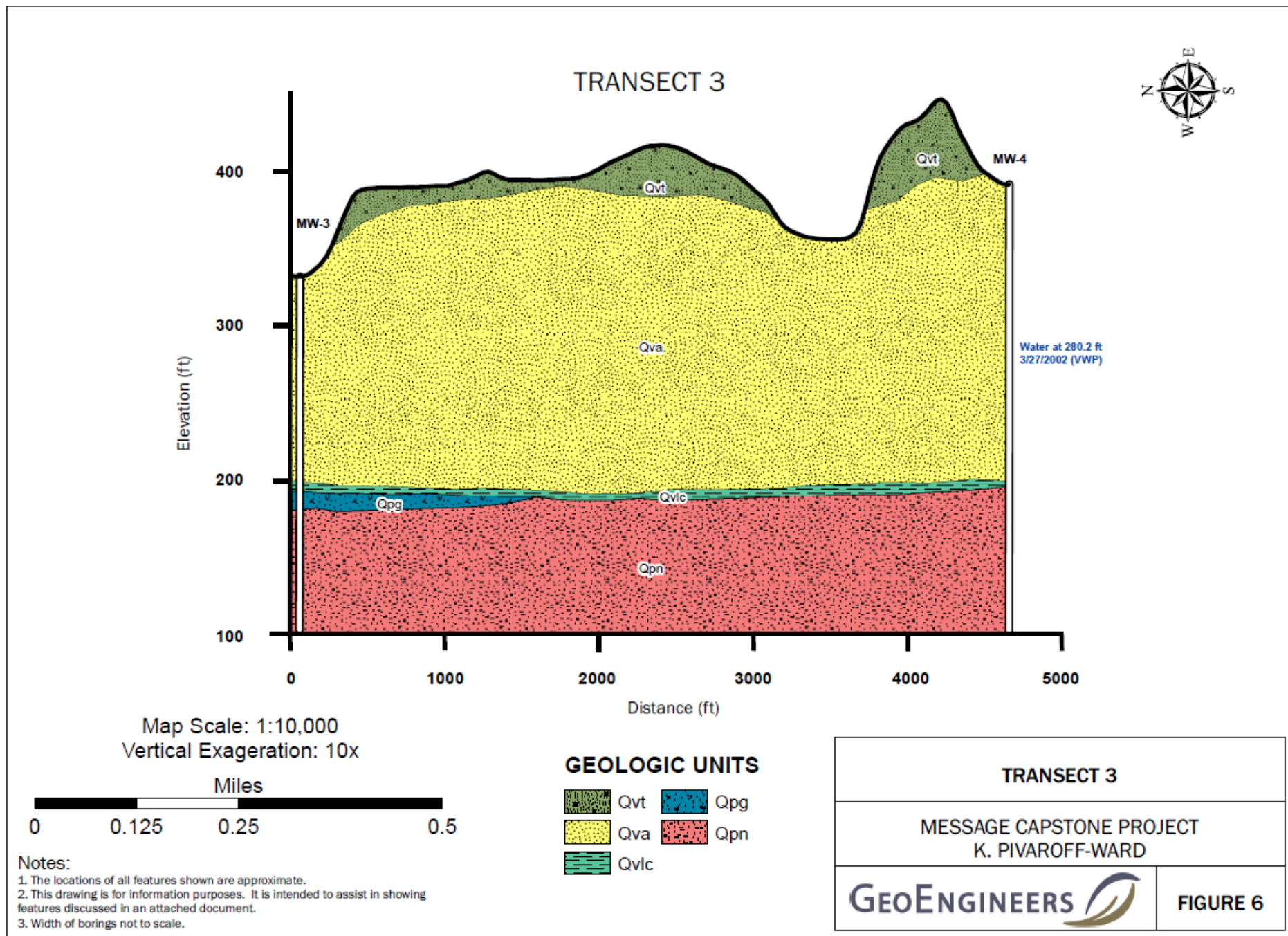


Figure 6, Cross-Section 3. Transect 3 is approximately 0.88 mi in length, and includes borings MW-3 and MW-4.

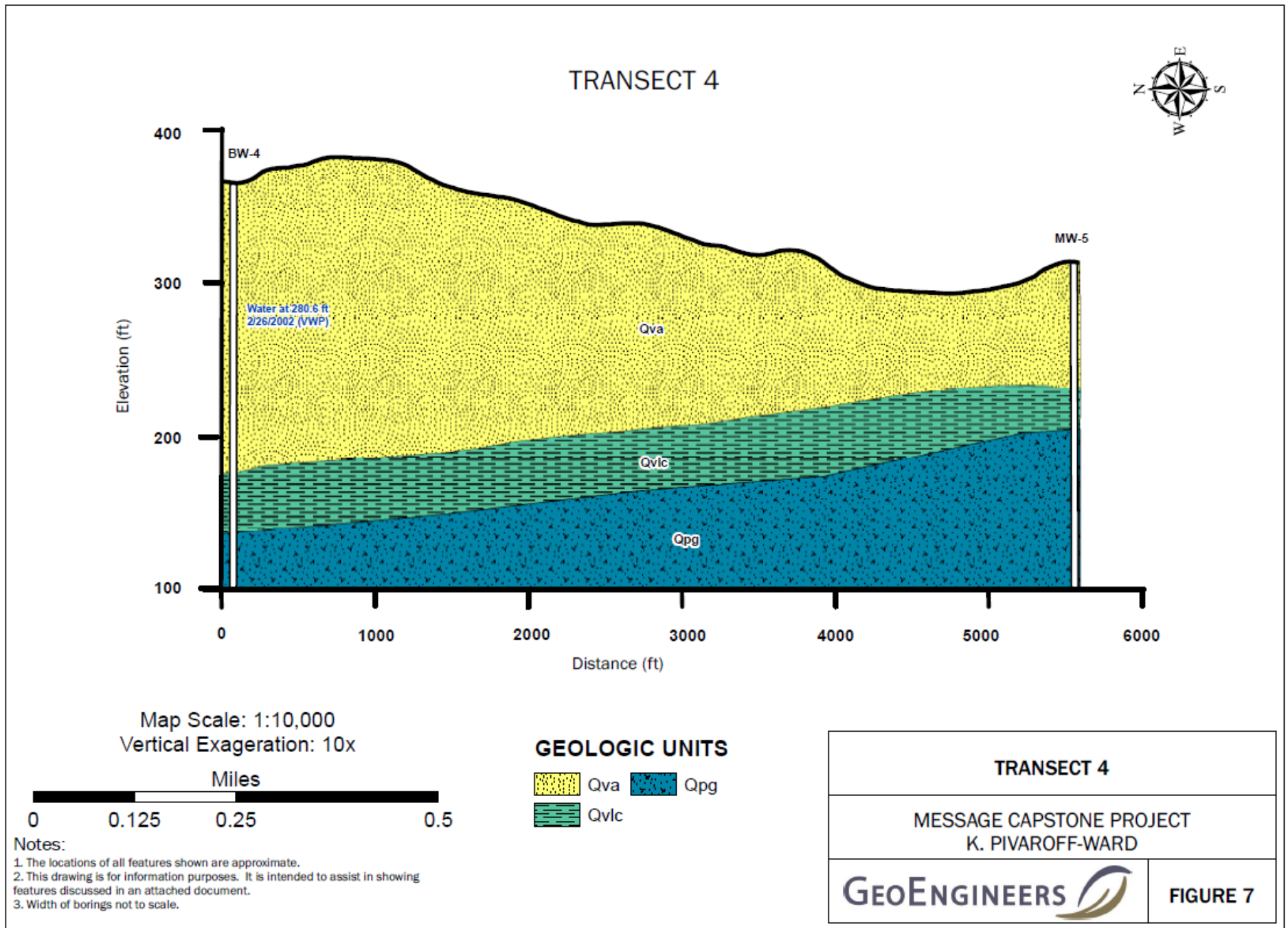


Figure 7, Cross-Section 4. Transect 4 is approximately 1.1 mi in length, and includes borings BW-4 and MW-5.

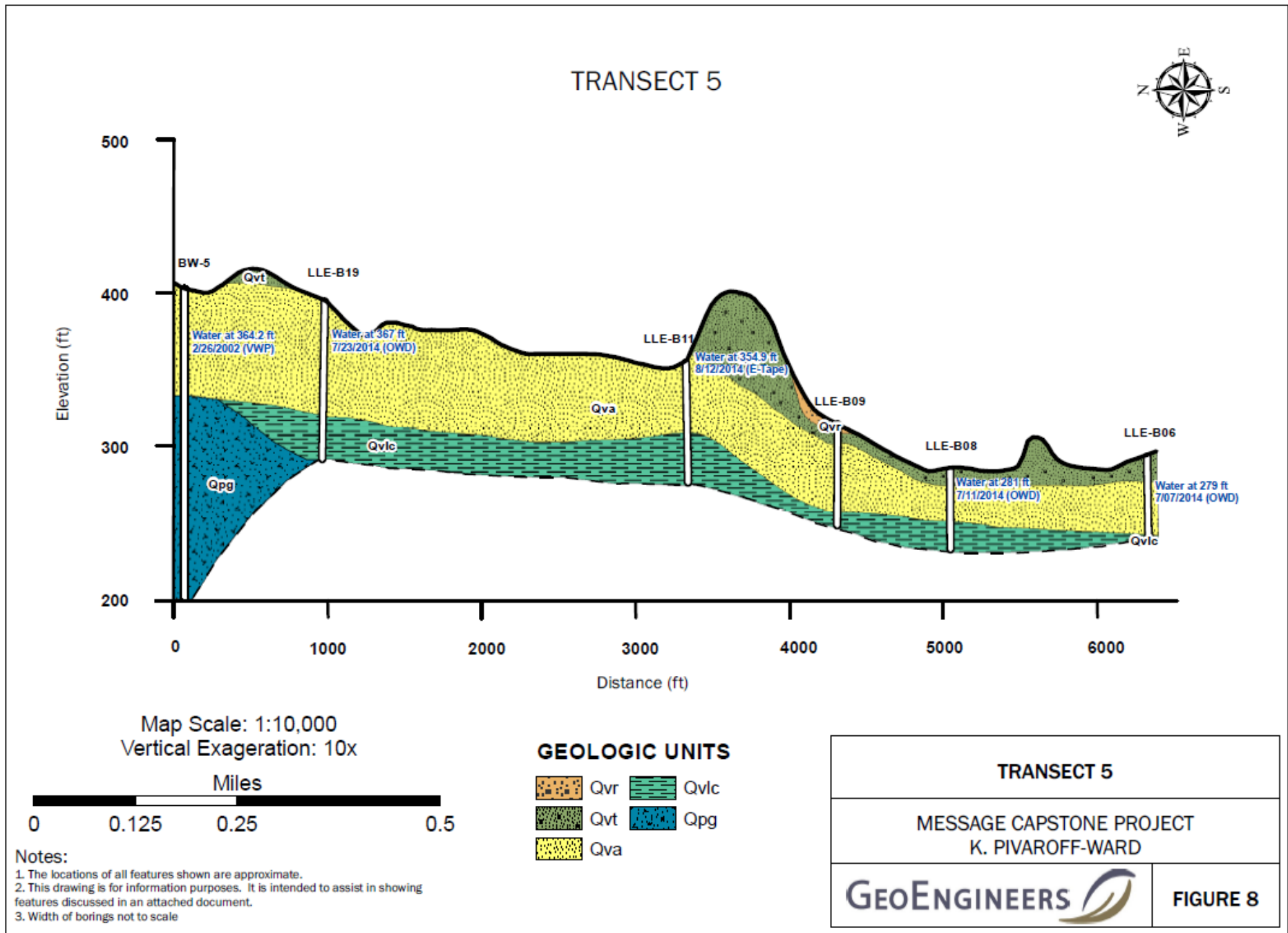


Figure 8, Cross-Section 5. Transect 5 is approximately 1.20 mi in length, and includes borings BW-5, LLE-B19, LLE-B17, LLE-B11, LLE-B09, LLE-B08, and LLE-B06. The apparent bend at boring LLE-B11P may represent a fold.

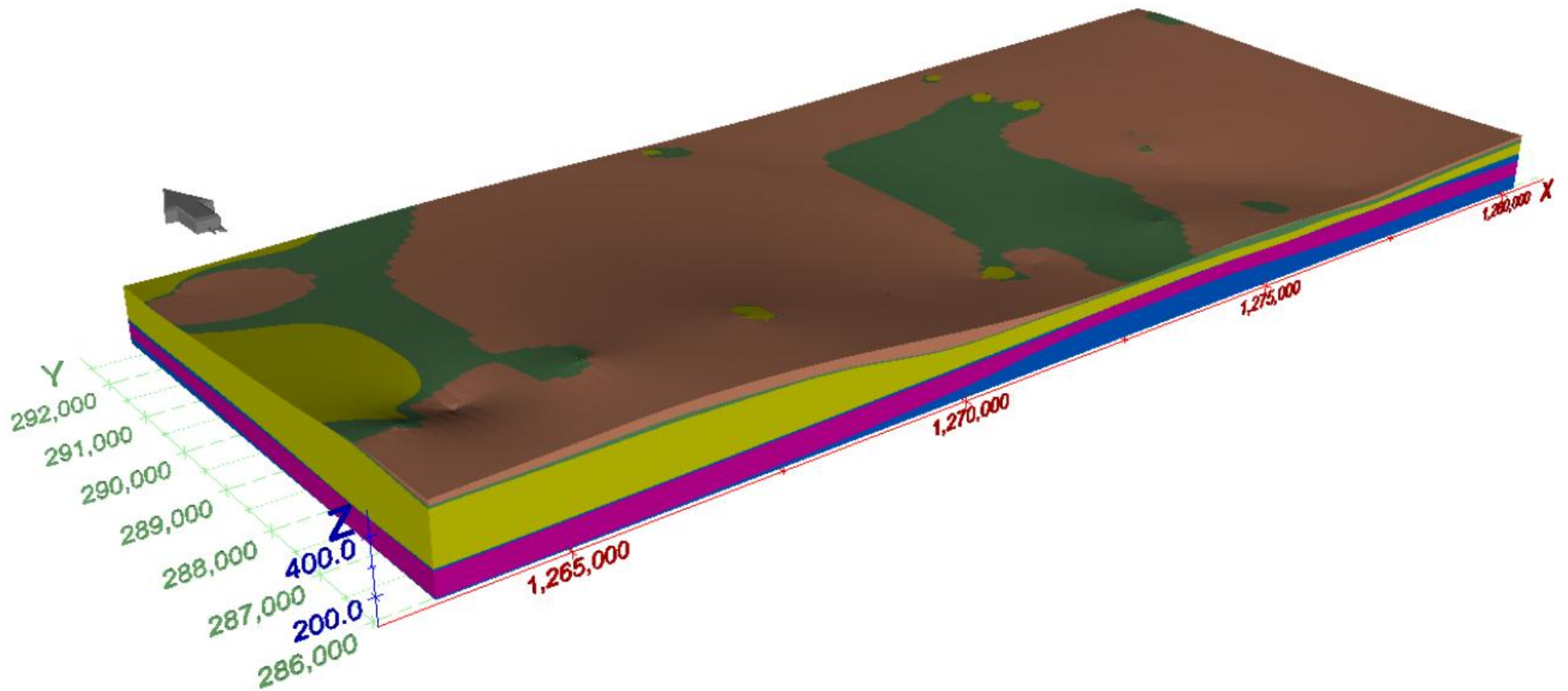


Figure 9A, Block Diagram. This block diagram was produced using EnterVol and the geologic contacts database (Table 2) that I constructed using borehole data. The units on the y- and x- axes are northing and easting, respectively. The unit on the z-axis is elevation (ft). The geologic units shown are: pre-Olympia glacial (Qpg) in blue, pre-Fraser interglacial (Qpn) in pink, Lawton clay (Qvlc) in teal, Vashon advance outwash (Qva) in yellow, Vashon till (Qvt) in green, and recessional outwash (Qvr) in orange. This view shows Transect 2 paralleling the x-axis, and Transect 3 paralleling the y-axis.

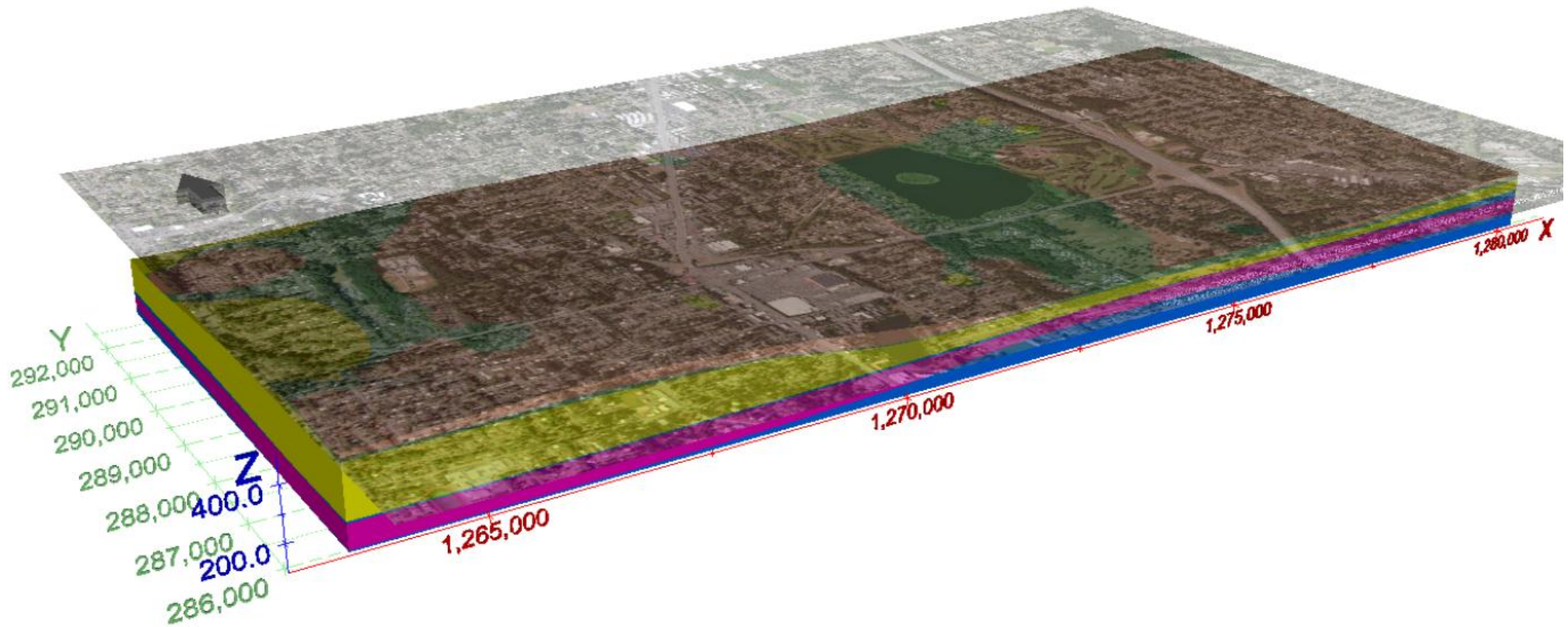


Figure 9B, Block Diagram with Vicinity Map Overlay. This block diagram was produced using EnterVol and the geologic contacts database (Table 2) that I constructed using borehole data. The units on the y- and x- axes are northing and easting, respectively. The unit on the z-axis is elevation (ft). The geologic units shown are: pre-Olympia glacial (Qpg) in blue, pre-Fraser interglacial (Qpn) in pink, Lawton clay (Qvlc) in teal, Vashon advance outwash (Qva) in yellow, Vashon till (Qvt) in green, and recessional outwash (Qvr) in orange. This view shows Transect 2 paralleling the x-axis, and Transect 3 paralleling the y-axis. Lake Ballinger is located near the center of the map, with Interstate-5 located east of the lake.

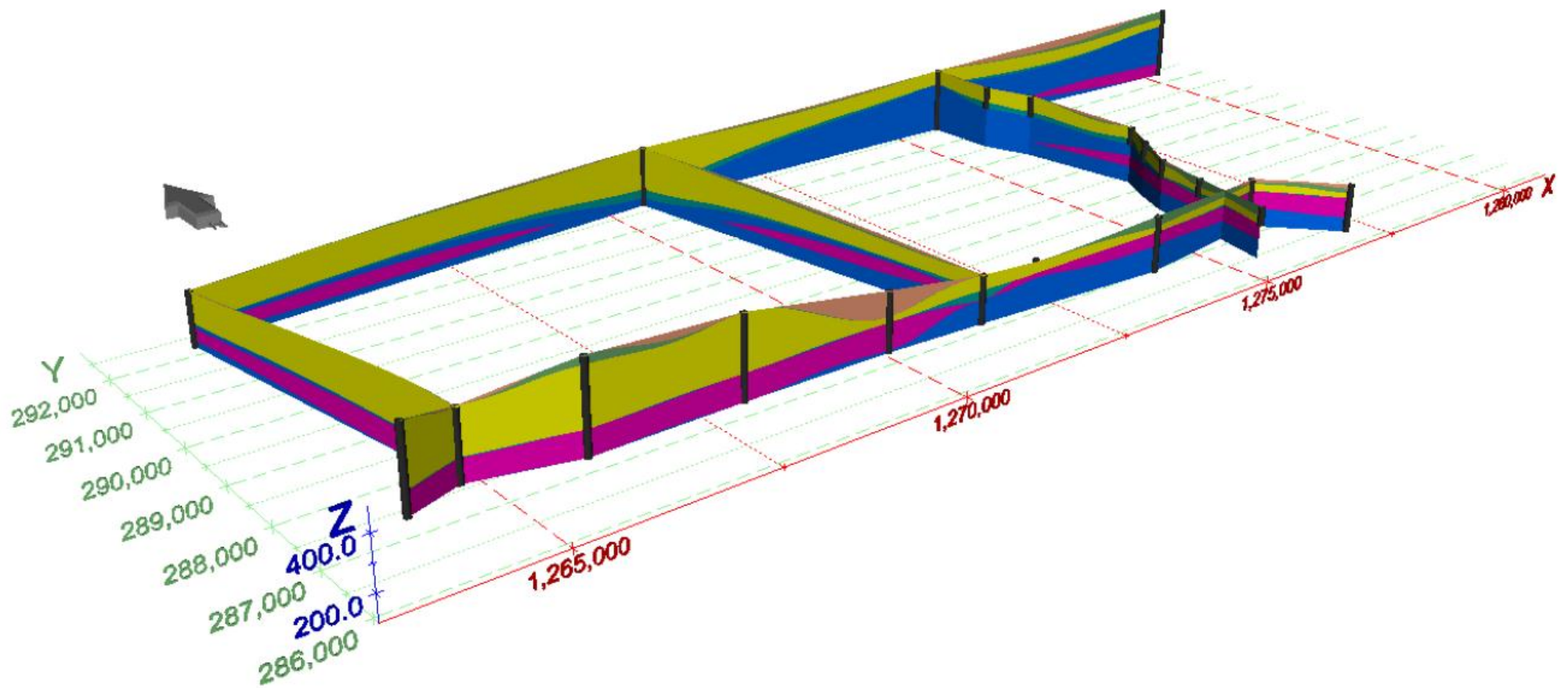


Figure 10, Fence Diagram. This fence diagram was produced using EnterVol and the geologic contacts database (Table 2) that I constructed from borehole data. The units on the y- and x- axes are northing and easting, respectively. The unit on the z-axis is elevation (ft). The geologic units shown are: pre-Olympia glacial (Qpg) in blue, pre-Fraser interglacial (Qpn) in pink, Lawton clay (Qvlc) in teal, Vashon advance outwash (Qva) in yellow, Vashon till (Qvt) in green, and recessional outwash (Qvr) in orange. The black bars show the location and depth of each of the boreholes used in this study. This view shows Transect 1 as the northern-most cross-section, Transect 2 paralleling the x-axis, Transect 3 paralleling the y-axis, Transect 4 in the center, and Transect 5 on the far right.

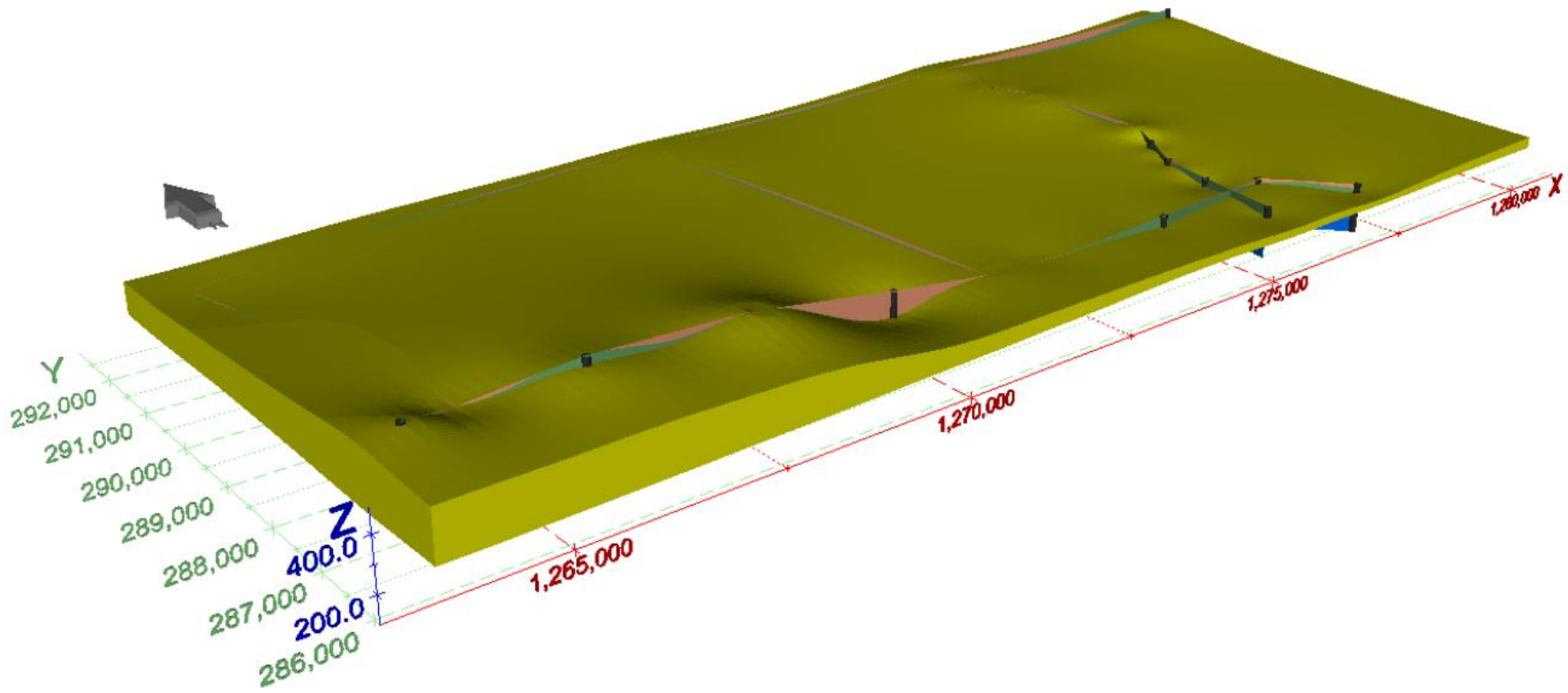


Figure 11A, Qva with Fence Diagram (From Above). This diagram was produced using EnterVol and the geologic contacts database (Table 2) that I constructed from borehole data. The units on the y- and x- axes are northing and easting, respectively. The unit on the z-axis is elevation (ft). The geologic units shown are: pre-Olympia glacial (Qpg) in blue, Vashon advance outwash (Qva) in yellow, Vashon till (Qvt) in green, and recessional outwash (Qvr) in orange. The black bars show the location and depth of each of the boreholes used in this study. This view shows Transect 1 as the northern-most cross-section, Transect 2 paralleling the x-axis, Transect 3 paralleling the y-axis, Transect 4 in the center, and Transect 5 on the far right. This view helps illustrate the extent of the Qva in relation to the Qvr and Qvt.

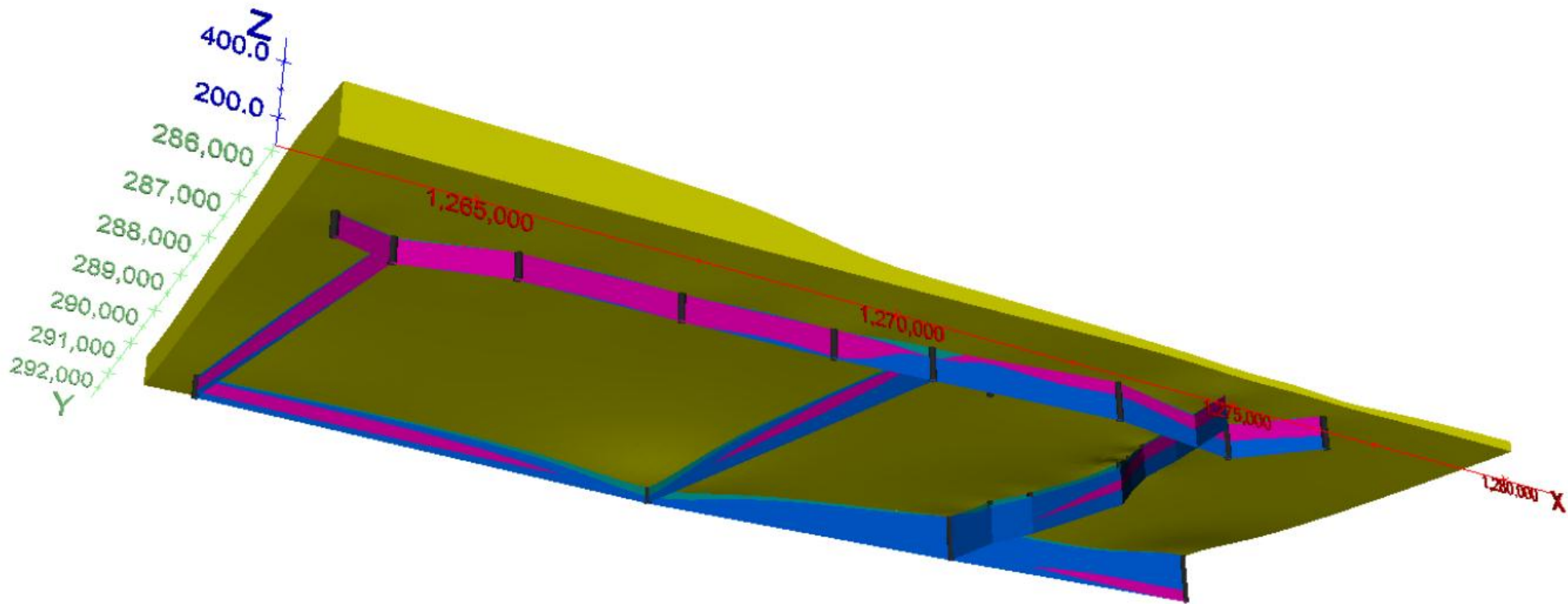


Figure 11B, Qva with Fence Diagram (From Below). This diagram was produced using EnterVol and the geologic contacts database (Table 2) that I constructed from borehole data. The units on the y- and x- axes are northing and easting, respectively. The unit on the z-axis is elevation (ft). The geologic units shown are: pre-Olympia glacial (Qpg) in blue, pre-Fraser interglacial (Qpn) in pink, Lawton clay (Qvlc) in teal, and Vashon advance outwash (Qva) in yellow. The black bars show the location and depth of each of the boreholes used in this study. This view shows Transect 1 as the northern-most cross-section, Transect 2 paralleling the x-axis, Transect 3 paralleling the y-axis, Transect 4 in the center, and Transect 5 on the far right. This view helps illustrate the extent of the Qva in relation to the Qvlc, Qpn, and Qpg.

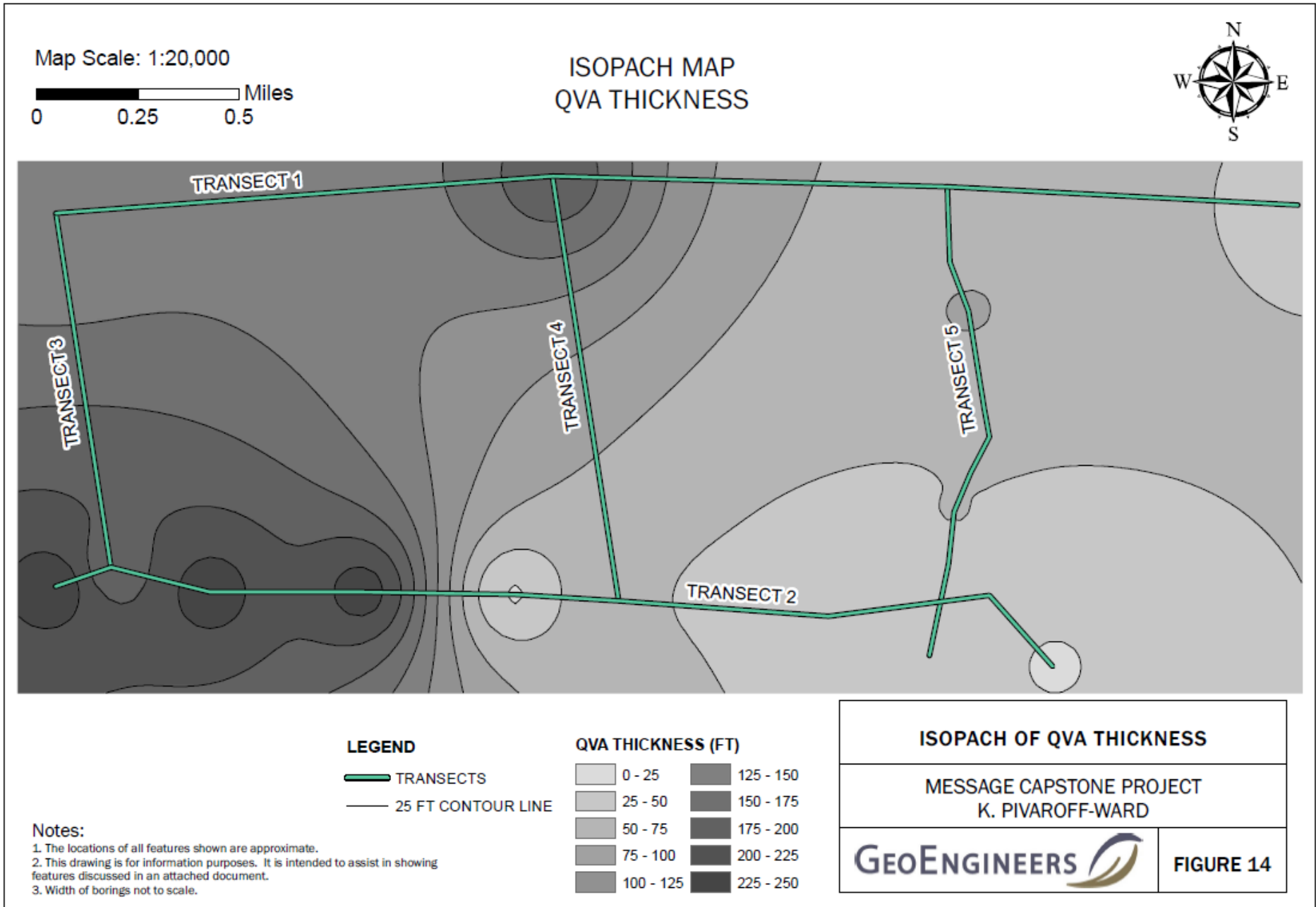


Figure 12, Isopach Map of Qva Thickness. The thickness of the Qva is mapped using 25 ft contours, which are based on the model produced in EnterVol. The Qva is thickest in the western corner of the study area, and thins to southeast. There is a steep gradient intersecting Transect 2, between Transects 3 and 4; at this location, there is a thick deposit of Qvr. The Qva is thickest where modern topography is high. The circular contours on the isopach map may indicate topographic highs and lows; however, they could also be artifacts of the model, which would indicate that the EnterVol did not accurately depict the thickness of the Qva.

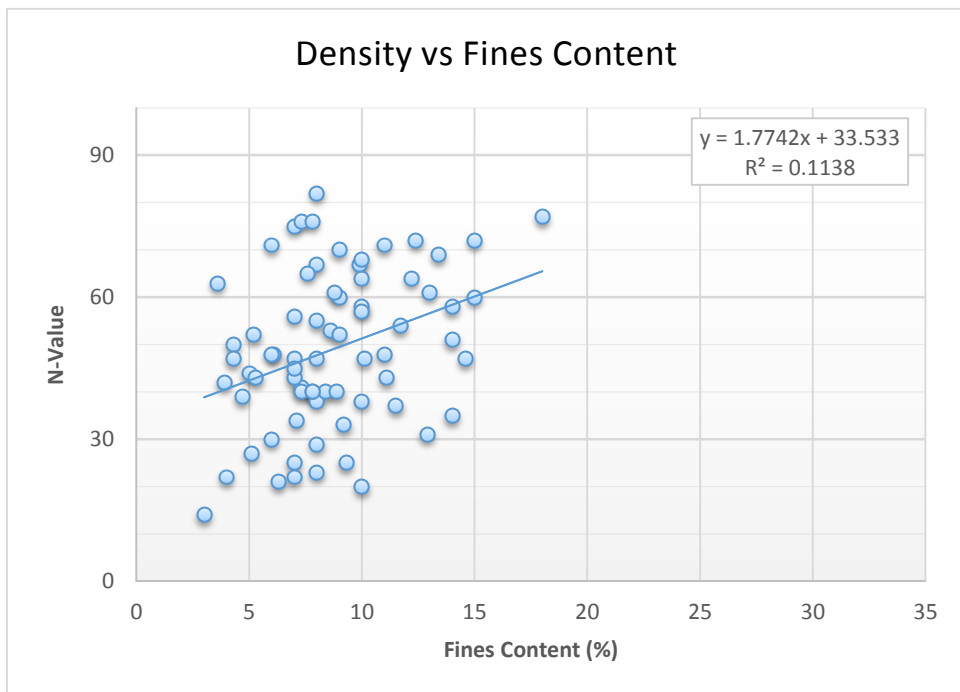
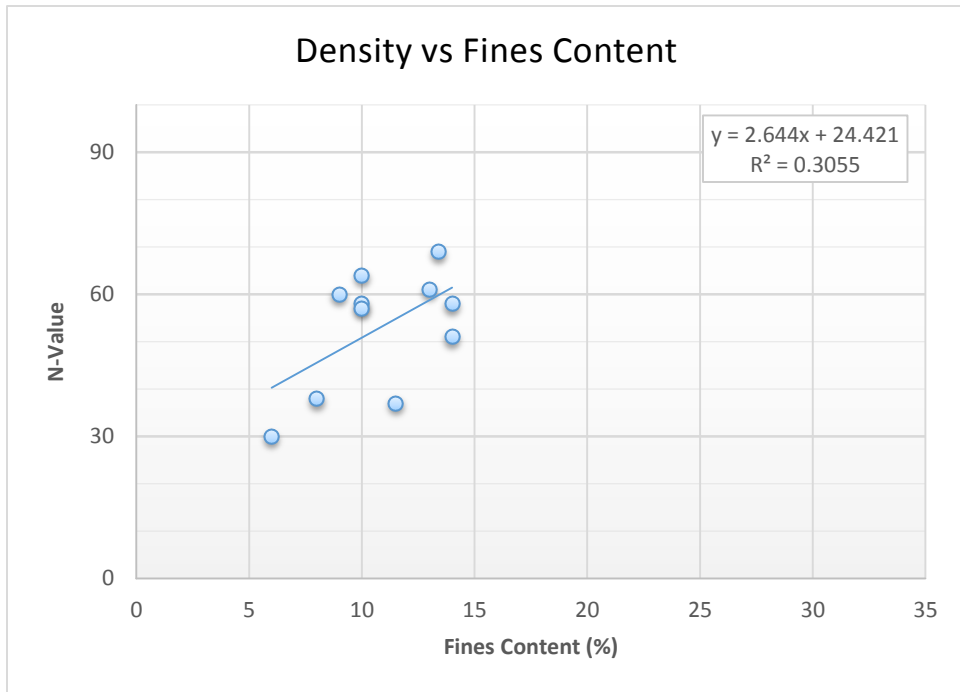


Figure 13, Density vs. Fines Content. These figures show the relative density of select soil samples as a function of fines content in the soil. Only samples collected using mud rotary drilling methods, which provide the most reliable density data in water-bearing sands, are shown in the top figure. Samples collected using mud rotary, Becker hammer, and hollow stem auger drilling methods are shown in the lower figure. For both of these figures, I used a linear regression line to show the trend in these datasets.

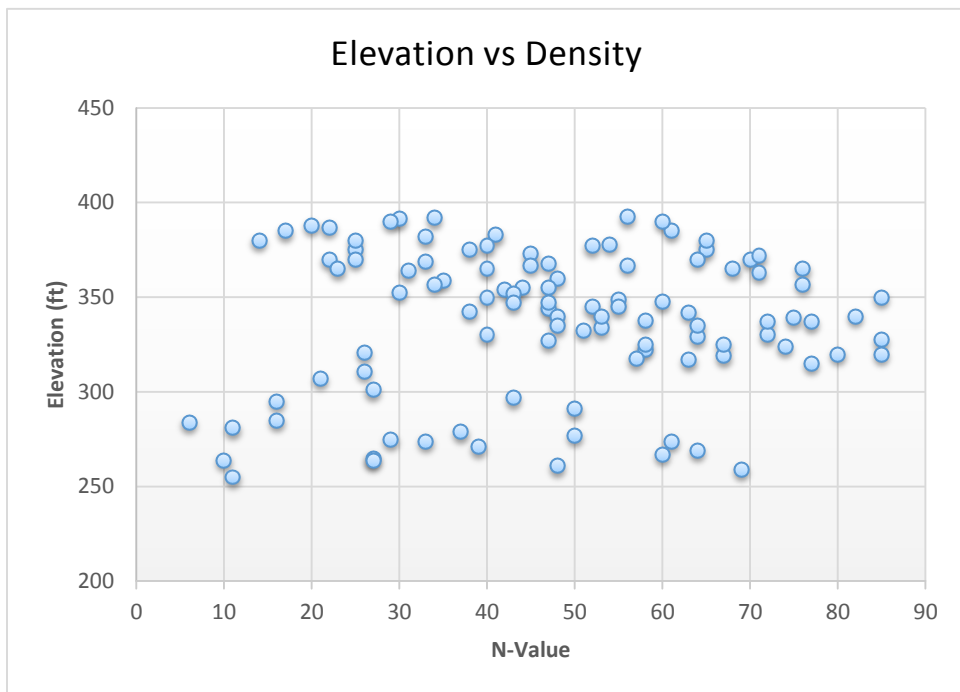
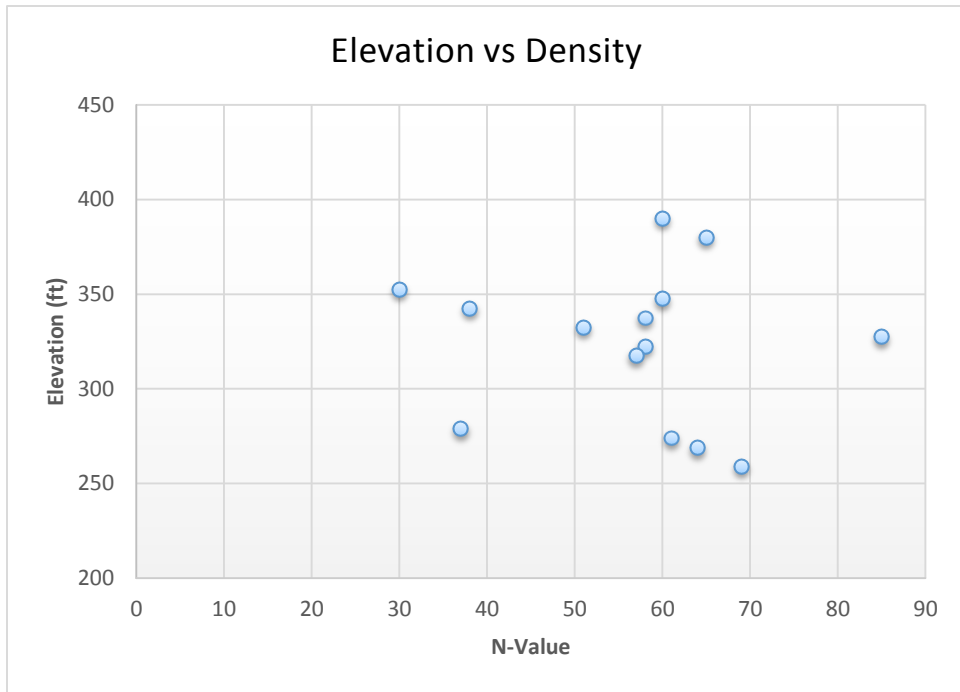


Figure 14, Elevation vs. Density. These figures show the relative density of select soil samples as a function of elevation (NAVD88 datum). Only samples collected using mud rotary drilling methods, which provide the most reliable density data in water-bearing sands, are shown in the top figure. Samples collected using mud rotary, Becker hammer, and hollow stem auger drilling methods are shown in the lower figure.

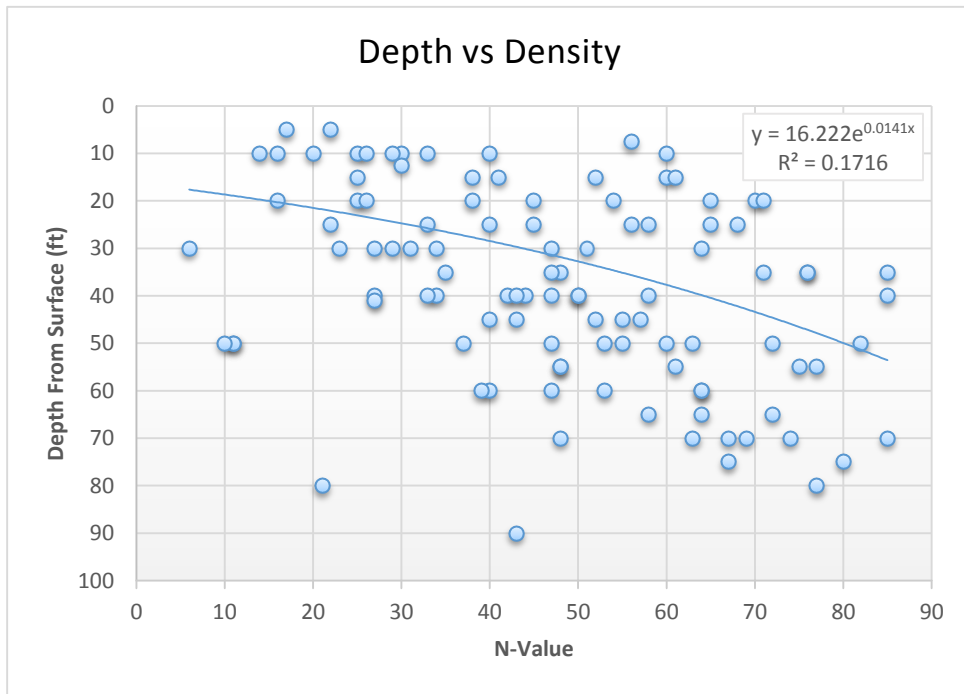
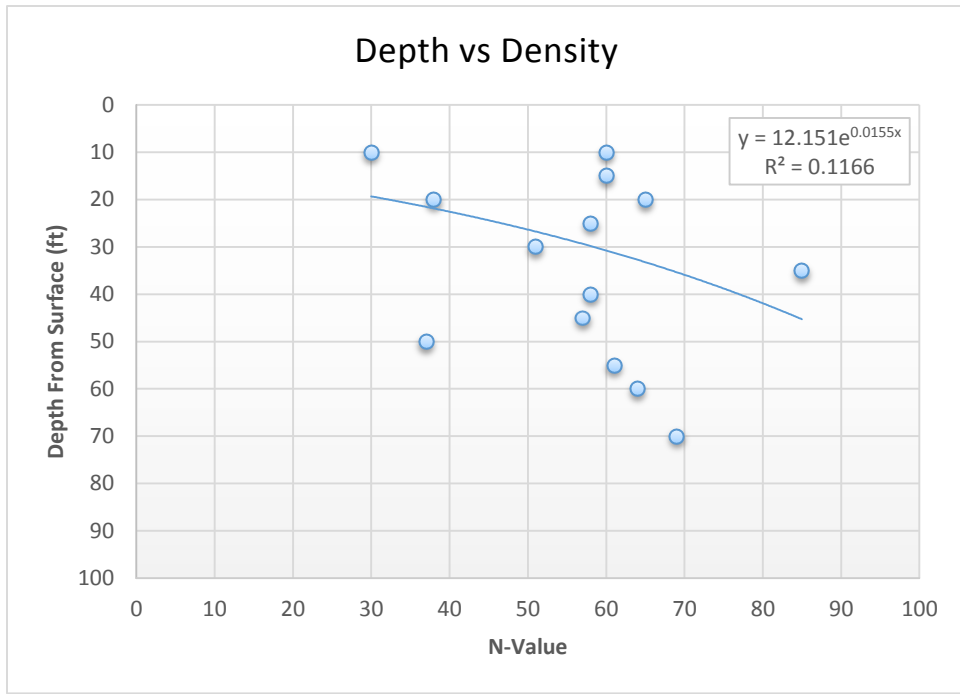


Figure 15, Depth vs. Density. These figures show the relative density of select soil samples as a function of depth from the surface of the ground. Only samples collected using mud rotary drilling methods, which provide the most reliable density data in water-bearing sands, are shown in the top figure. Samples collected using mud rotary, Becker hammer, and hollow stem auger drilling methods are shown in the lower figure. For both of these figures, I used an exponential regression curve to show the trend in these datasets.

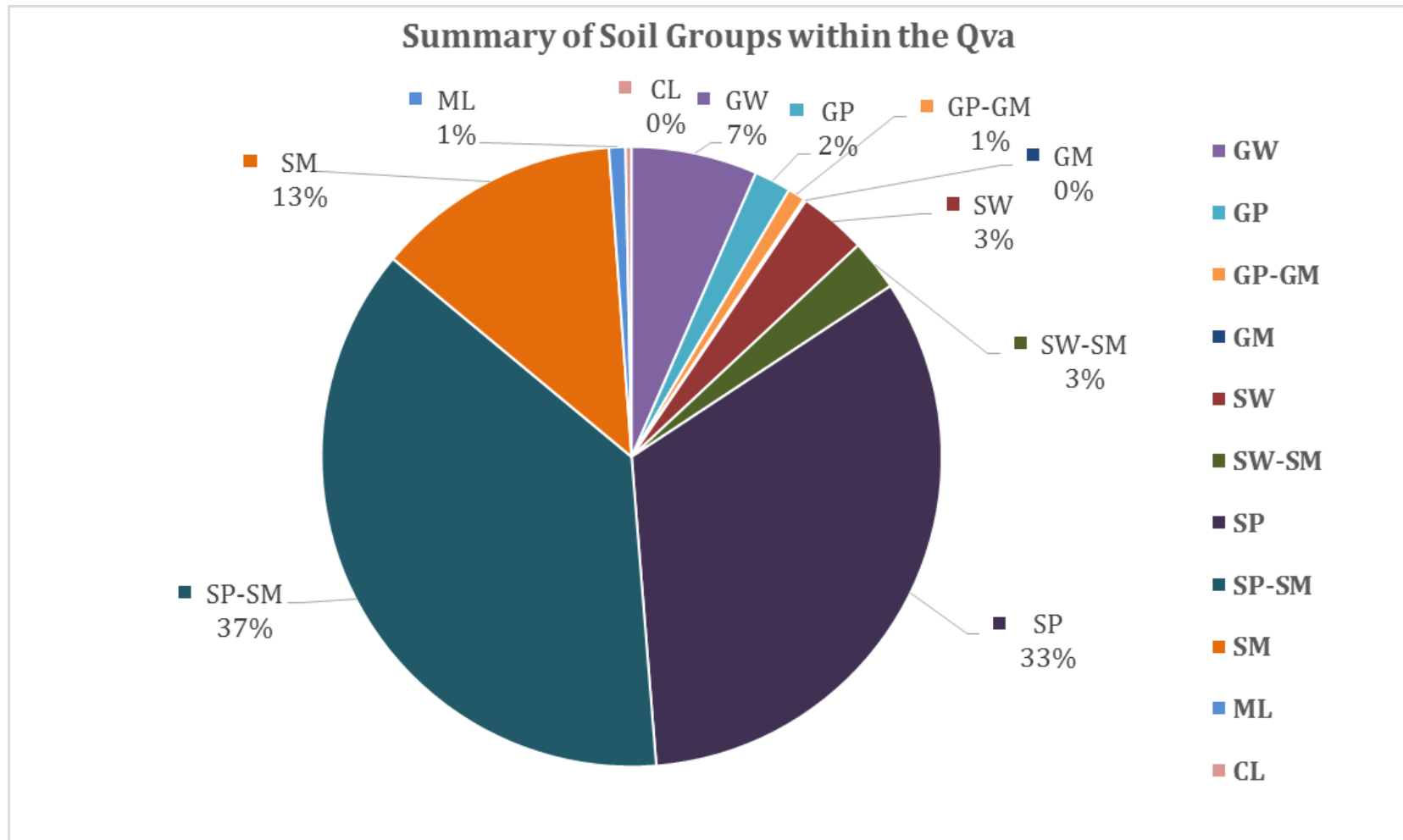


Figure 16A, Summary of Soil Classifications within the Qva. This chart summarizes the percentages of the soil groups found within the Qva, as recorded in the geotechnical boring logs. Gravel comprises about 9.5% of the total volume of the Qva, sand about 89.3%, and silt/clay about 1.2%. The abbreviations are as listed: well-graded gravel (GW), poorly-graded gravel (GP), poorly-graded gravel with silt (GP-GM), silty gravel (GM), well-graded sand (SW), well-graded sand with silt (SW-SM), poorly-graded sand (SP), poorly-graded sand with silt (SP-SM), silty sand (SM), lean silt (ML), and lean clay (CL).

Volumetric Analysis of Soil Groups within the Qva

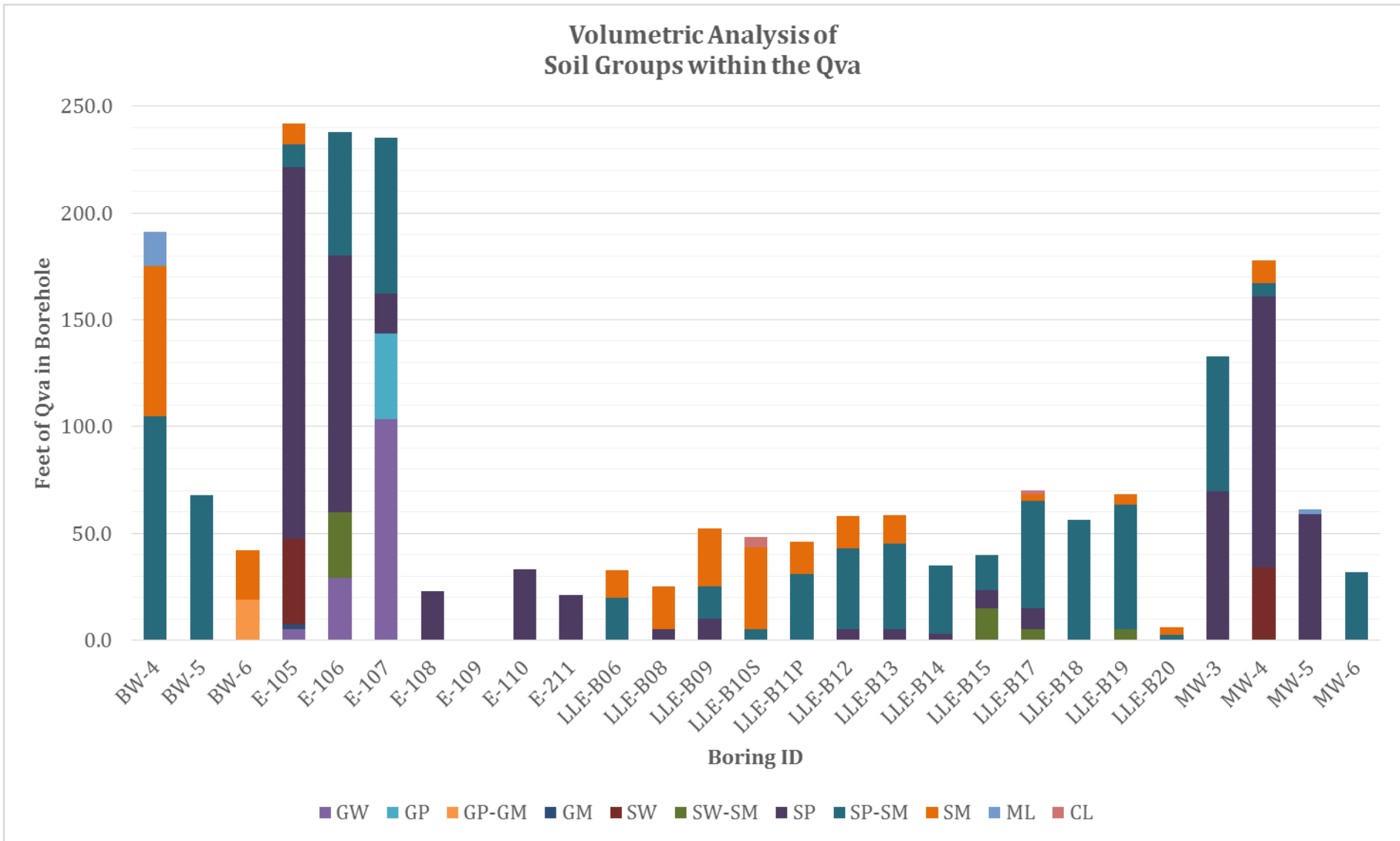


Figure 16B, Summary of Soil Classifications within the Qva. This chart summarizes the percentages of the soil groups found within the Qva, as found in each borehole. Gravel comprises about 9.5% of the total volume of the Qva, sand about 89.3%, and silt/clay about 1.2%. The abbreviations are as listed: well-graded gravel (GW), poorly-graded gravel (GP), poorly-graded gravel with silt (GP-GM), silty gravel (GM), well-graded sand (SW), well-graded sand with silt (SW-SM), poorly-graded sand (SP), poorly-graded sand with silt (SP-SM), silty sand (SM), lean silt (ML), and lean clay (CL).

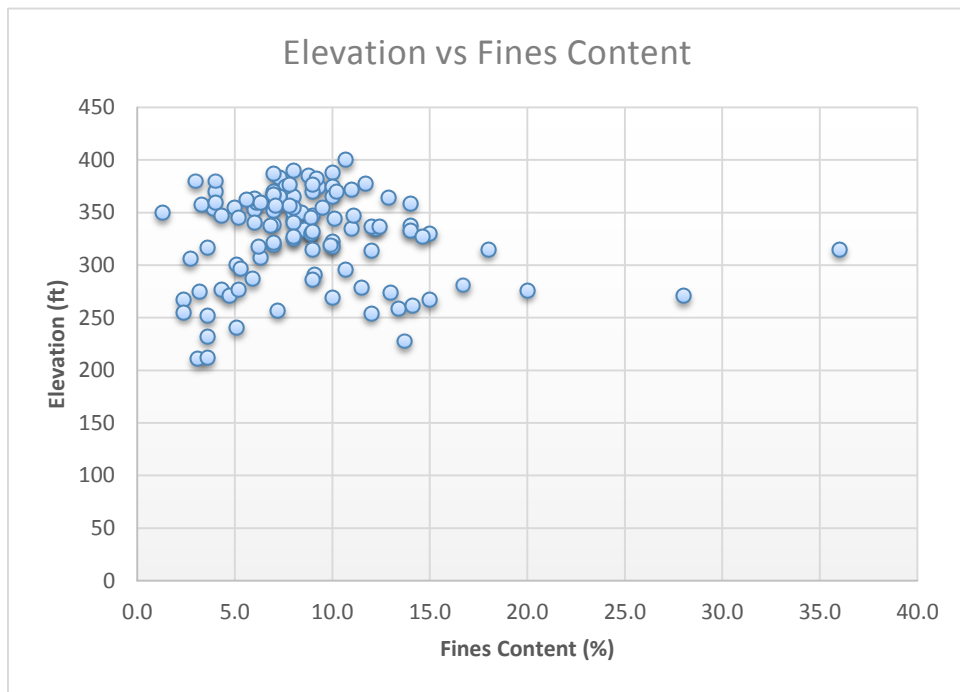
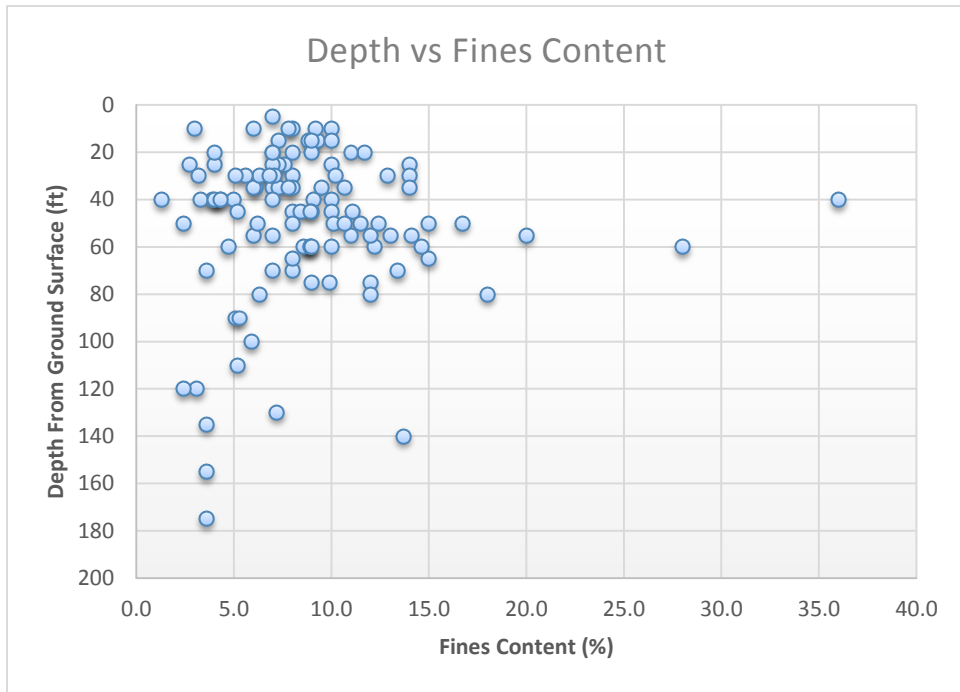


Figure 17, Depth and Elevation vs. Fines Content. These figures show the relationship between depth and fines content (top), and elevation and fines content (bottom), in select samples from within the Qva.

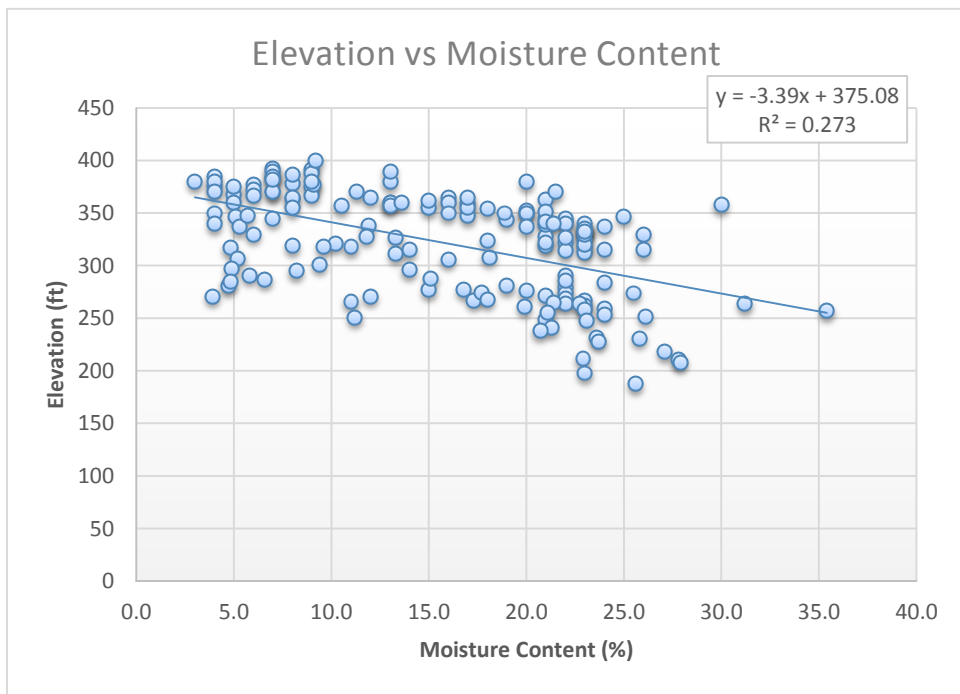
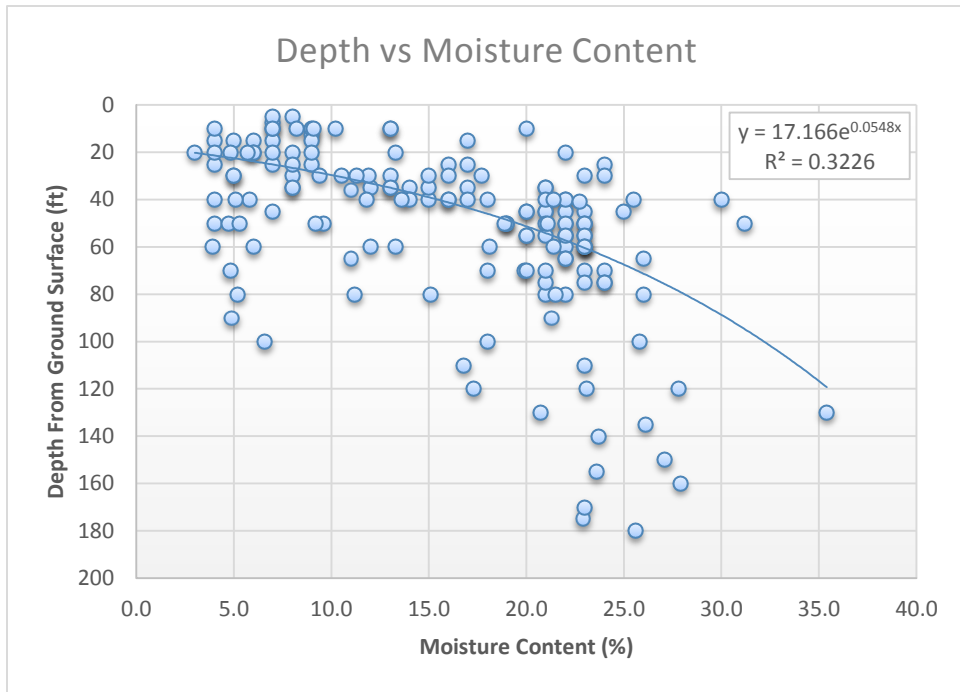


Figure 18, Depth and Elevation vs. Moisture Content. These figures show the relationship between depth and moisture content (top), and elevation and moisture content (bottom), in select samples from within the Qva. In the top chart, I used an exponential regression curve to show the relationship between depth and moisture content. In the bottom chart, I used an order-two polynomial line to show the trend.

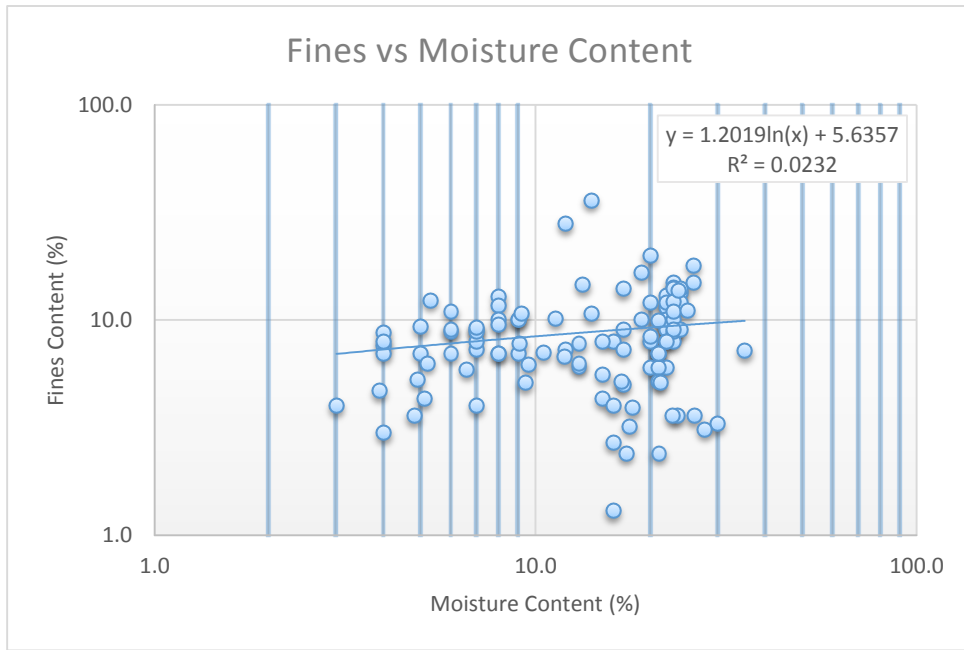


Figure 19, Fines Content vs Moisture Content. This figure shows the relationship between fines content and moisture content in select samples from within the Qva. This chart is best viewed on a log-log scale. I used a logarithmic trend line to show the subtle relationship between fines content and moisture content.

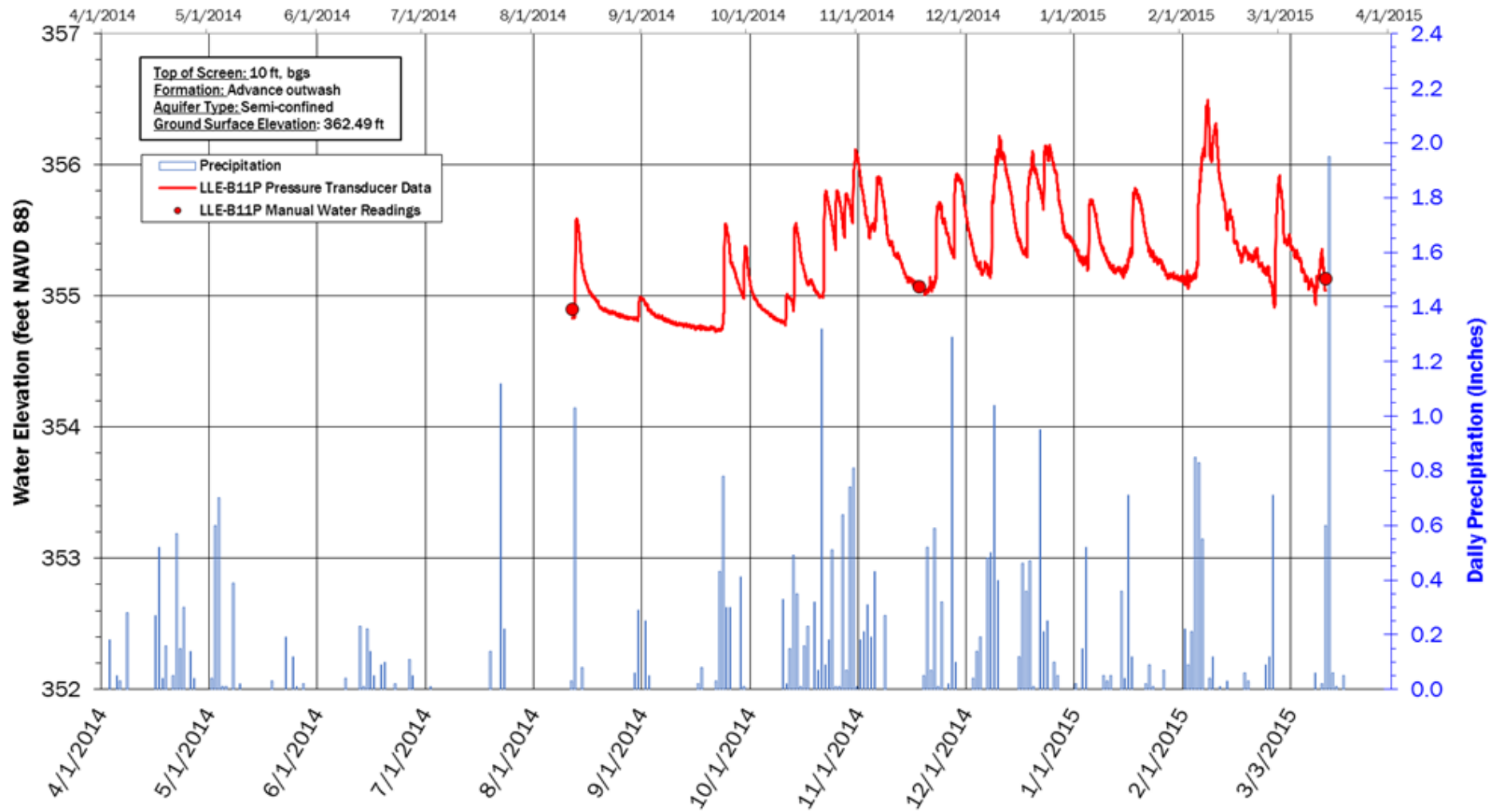


Figure 20, Groundwater Observations at Boring LLE-B11P. Groundwater observations were recorded using a pressure transducer at a well that was installed at boring LLE-B11P. The well is positioned in the advance outwash, and the top of the screen is 10 ft bgs. The surface elevation is 362.49 ft at this location. The chart shows that the peak groundwater elevation at this location was recorded at 356.47 ft on February 08, 2015, and the minimum groundwater elevation was recorded at 354.74 ft on September 23, 2014. The groundwater elevation appears to increase following large precipitation events. (Image from GeoEngineers, 2015).

13.0 Tables

Table 1, Summary of Geotechnical Borings. This table is a summary of the existing geotechnical borings that were used in this study.

Boring ID	Surface Elevation (NAVD88, ft)	Borehole Depth (ft)	Drilling Method	Qva Thickness (ft)	Contractor, Project (Year)
BW-4	368.0	366.4	Mud Rotary	191.0	Shannon & Wilson, Brightwater Project (2002)
BW-5	400.0	391.2	Mud Rotary	68.0	
BW-6	450.0	430.5	Mud Rotary	42.0	
E-105	452.7	535.0	Rotosonic Coring	242.0	CDM Smith and Subconsultants, Brightwater Project (2003)
E-106	485.6	566.0	Wireline	238.0	
E-107	453.2	548.0	Wireline/Mud Rotary	235.0	
E-108	356.8	346.0	Wireline Coring	23.0	
E-109	298.7	260.0	Wireline Coring	0.0	
E-110	348.4	438.0	Wireline Coring	33.0	
E-211	317.1	280.0	Wireline Coring	21.0	
LLE-B06	329.0	80.4	Mud Rotary	--	GeoEngineers, Inc., Sound Transit - LLE (2014)
LLE-B08	317.0	81.5	Hollow Stem Auger	25.0	
LLE-B09	331.0	81.5	Hollow Stem Auger	52.5	
LLE-B10S	354.0	81.5	Mud Rotary	--	
LLE-B11P	362.5	81.5	Mud Rotary	46.0	
LLE-B12	394.0	81.0	Hollow Stem Auger	--	
LLE-B13	395.0	81.5	Hollow Stem Auger	--	
LLE-B14	398.0	41.0	Hollow Stem Auger	--	
LLE-B15	400.0	41.5	Hollow Stem Auger	--	
LLE-B17	484.0	101.5	Hollow Stem Auger	70.0	
LLE-B18	390.0	61.0	Hollow Stem Auger	--	
LLE-B19	392.0	101.5	Hollow Stem Auger	68.5	
LLE-B20	432.0	41.5	Hollow Stem Auger	--	
MW-3	331.0	369.0	Becker Hammer	133.0	HWA Geosciences, Brightwater Project (2002)
MW-4	387.0	446.5	Becker Hammer	178.0	
MW-5	305.0	352.0	Becker Hammer	61.0	
MW-6	314.0	360.5	Becker Hammer	32.0	

Table 2, Geologic Contacts Database. This table shows the location of each boring, and the elevation of the bottom of each geologic unit with respect to each boring. I terminated this study at an elevation of 100 ft, although not all borings reached this depth. Blank spaces in the table indicate the absence of a geologic unit within a boring. Notice the anomalous nature of boring E-109, which does not contain any deposits from the Vashon Stade, but instead shows that Qpn is observed in the first 71 ft of the boring.

Boring ID	Northing	Easting	Surface Elevation (ft)	Qvr	Qvt	Qva	Qvlc	Qpg_1	Qpn_1	Qpnmw	Qpn_2	Qpg_2	Bottom Elevation (ft)
BW-4	292702.53	1270571.18	368			177	136	100					100
BW-5	292558.92	1275711.57	400			331		100					100
BW-6	292329.00	1280284.89	450		407	366		164	100				100
E-105	287350.73	1264077.49	453		438	196	193		100				100
E-106	287281.71	1266085.31	486		445	207			100				100
E-107	287286.32	1268070.00	454			214			100				100
E-108	287250.64	1270073.56	357	247		224			141	116	101	100	100
E-109	287168.00	1272288.00	299						228			100	100
E-110	286963.17	1274157.64	349		297	272			213			100	100
E-211	286326.37	1277098.94	317	295	274	253			164			100	100
LLE-B08	287645.34	1275729.20	317		279	254	236						236
LLE-B09	288336.35	1275804.25	331	318	308	256	250						250
LLE-B11P	289308.55	1276263.13	363			310	281						281
LLE-B17	290943.92	1275989.00	484			406	382						382
LLE-B19	291584.06	1275750.55	392			320	291						291
MW-3	292217.63	1264080.88	331			198	194	181	129			100	100
MW-4	287602.69	1264800.59	387			201	197		100				100
MW-5	287165.87	1271424.47	305			242	201					100	100
MW-6	287239.93	1276263.15	314	295	292	260			181			100	100

Table 3, Summary of the soil groups found within the Qva in each boring. The abbreviations are as follows: well-graded gravel (GW), poorly-graded gravel (GP), poorly-graded gravel with silt (GP-GM), silty gravel (GM), well-graded sand (SW), well-graded sand with silt (SW-SM), poorly-graded sand (SP), poorly-graded sand with silt (SP-SM), silty sand (SM), lean silt (ML), and lean clay (CL).

Boring ID	Qva Measured in Boring (ft)	GW	GP	GP-GM	GM	SW	SW-SM	SP	SP-SM	SM	ML	CL
BW-4	191.0	--	--	--	--	--	--	--	105.0	70.0	16.0	--
BW-5	68.0	--	--	--	--	--	--	--	68.0	--	--	--
BW-6	42.0	--	--	19.0	--	--	--	--	--	23.0	--	--
E-105	242.0	5.0	--	--	2.5	40.0	--	174.0	10.5	10.0	--	--
E-106	238.0	29.0	--	--	--	--	31.0	120.0	58.0	--	--	--
E-107	235.0	103.5	40.0	--	--	--	--	18.5	73.0	--	--	--
E-108	23.0	--	--	--	--	--	--	23.0	--	--	--	--
E-109	0.0	--	--	--	--	--	--	--	--	--	--	--
E-110	33.0	--	--	--	--	--	--	33.0	--	--	--	--
E-211	21.0	--	--	--	--	--	--	21.0	--	--	--	--
LLE-B06	32.5	--	--	--	--	--	--	--	20.0	12.5	--	--
LLE-B08	25.0	--	--	--	--	--	--	5.0	--	20.0	--	--
LLE-B09	52.5	--	--	--	--	--	--	10.0	15.0	27.5	--	--
LLE-B10S	48.5	--	--	--	--	--	--	--	5.0	38.5	--	5.0
LLE-B11P	46.0	--	--	--	--	--	--	--	31.0	15.0	--	--
LLE-B12	58.0	--	--	--	--	--	--	5.0	38.0	15.0	--	--
LLE-B13	58.5	--	--	--	--	--	--	5.0	40.0	13.5	--	--
LLE-B14	35.0	--	--	--	--	--	--	3.0	32.0	--	--	--
LLE-B15	40.0	--	--	--	--	--	15.0	8.5	16.5	--	--	--
LLE-B17	70.0	--	--	--	--	--	5.0	10.0	50.0	3.5	--	1.5
LLE-B18	56.5	--	--	--	--	--	--	--	56.5	--	--	--
LLE-B19	68.5	--	--	--	--	--	5.0	--	58.5	5.0	--	--
LLE-B20	6.0	--	--	--	--	--	--	--	2.5	3.5	--	--
MW-3	133.0	--	--	--	--	--	--	69.5	63.5	--	--	--
MW-4	178.0	--	--	--	--	34.0	--	127.0	6.0	11.0	--	--
MW-5	61.0	--	--	--	--	--	--	59.0	--	--	2.0	--
MW-6	32.0	--	--	--	--	--	--	--	32.0	--	--	--
Total Feet:	2094.0	137.5	40.0	19.0	2.5	74.0	56.0	691.5	781.0	268.0	18.0	6.5
Percent Total:	100.0%	6.6%	1.9%	0.9%	0.1%	3.5%	2.7%	33.0%	37.3%	12.8%	0.9%	0.3%

Table 4, Comparative Results. This table summarizes the findings from this study, and compares them to published sources.

Parameter with Respect to Qva	Findings From This Study	Published Data (Source)	Notes
Elevation Range (Qva Surface)	477-247 ft	400-600 ft (Troost and Booth, 2008)	General glacial stratigraphy
Unit Thickness	0 - 242 ft	>100 ft (Mullineaux, 1965); 50-200 ft (Golder Associates, 2008); 0-400 ft (Troost and Booth, 2008)	General glacial stratigraphy; As observed in the Mountlake Terrace area; General glacial stratigraphy
Relative Density	dense to very dense	dense to very dense (Galster and Laprade, 1991)	General glacial stratigraphy
Friction Angle	N/A	30-40° (Koloski, <i>et al</i> , 1989); About 32° (Miller, 1989)	General glacial outwash properties; General glacial outwash properties
Cohesion	N/A	0-1000 psf (Koloski, <i>et al</i> , 1989)	General glacial outwash properties
Dry Density	N/A	115-130 pcf (Koloski, <i>et al</i> , 1989); 110 pcf (Miller, 1989)	General glacial outwash properties; General Qva properties
Wet Density	N/A	120 pcf (Miller, 1989)	General Qva properties
Relative Erodability	N/A	Low-Medium (Koloski, <i>et al</i> , 1989)	General glacial outwash properties
Excavation Difficulty	N/A	Low-Medium (Koloski, <i>et al</i> , 1989); Easy (Laprade and Robinson, 1989)	General glacial outwash properties; General glacial outwash properties
Moisture Sensitivity	N/A	Low-Medium (Koloski, <i>et al</i> , 1989)	General glacial outwash properties
Foundation Support	N/A	1500-3000 psf (Koloski, <i>et al</i> , 1989)	General glacial outwash properties
Cut Slopes	N/A	50-70% (Koloski, <i>et al</i> , 1989)	General glacial outwash properties
Aquifer Depth	7.59-221.3 ft bgs	20-70 ft bgs (King County, 2002)	General depth to saturated soils along Brightwater alignment
Saturated Thickness	0-102.13 ft	10-100 ft (Golder Associates, 2008)	Aquifer thickness in the Mountlake Terrace area during a 2008 study
Mean Hydraulic Conductivity	15.93 ft/day	40 ft/d (Golder Associates, 2008) 0.33-328 ft/d (Galster and Laprade, 1991)	As measured in the Mountlake Terrace area; General Qva properties
Groundwater Recharge Rate	N/A	15-20 in/yr where Qva exposed and <10 in/yr where Qva is capped by till or pavement (Golder Associates, 2008)	Recharge rate in the Mountlake Terrace area during a 2008 study
Permeability	N/A	0.01-10 ft/min (Koloski, <i>et al</i> , 1989) 0.0001-0.2 ft/min (Laprade and Robinson, 1989)	General glacial outwash properties

14.0 Appendices

Appendix A

Summary of Hydraulic Conductivity Data for Glacial Sediments and Conceptual Hydrogeologic Model (Adapted from Golder Associates, 2008)

Appendix A, Summary of Hydraulic Conductivity Data for Glacial Sediments and Conceptual Hydrogeologic Model. (From Golder Associates, 2008)

Summary of Hydraulic Conductivity Data for Glacial Sediments

Aquifer Unit	Hydraulic Conductivity (ft/d)		
	Minimum	Median	Maximum
Alluvium	3.6	88	3,200
Recessional Outwash	0.08	180	1,800
Advance Outwash	0.18	40	2,800
Pre-Vashon Aquifers	0.22	31	1,800

Conceptual Hydrogeologic Model

Geologic Unit	Hydrostratigraphic Unit	Saturated Thickness (feet)	Groundwater Recharge (in/yr)	Groundwater Discharge	Median Hydraulic Conductivity (ft/d)	Potential for Infiltration
Alluvium	Alluvia and Recessional Outwash Aquifer	0-40(?)	20-24	To Streams and Lake Ballinger	180	Low - Limited unsaturated thickness and good connection to surface water
Recessional Outwash						
Vashon Till	Aquitard	na	<10 where exposed on surface	na	53	Low - Low Permeability
Advance Outwash	Aquifer	10-100	15-20 where exposed at surface, <10 when under till	To Streams and to Pre-Vashon Aquifers	40	Good where exposed at ground surface and sufficient unsaturated thickness exists. Low where present under till unless till removed.
Lawton Clay	Aquitard	na	na	na	7	Low - Low Permeability
Pre-Vashon	Aquifers and Aquitards	Variable	<10	Puget Sound	31	Low - deep, water supply aquifer

Appendix B

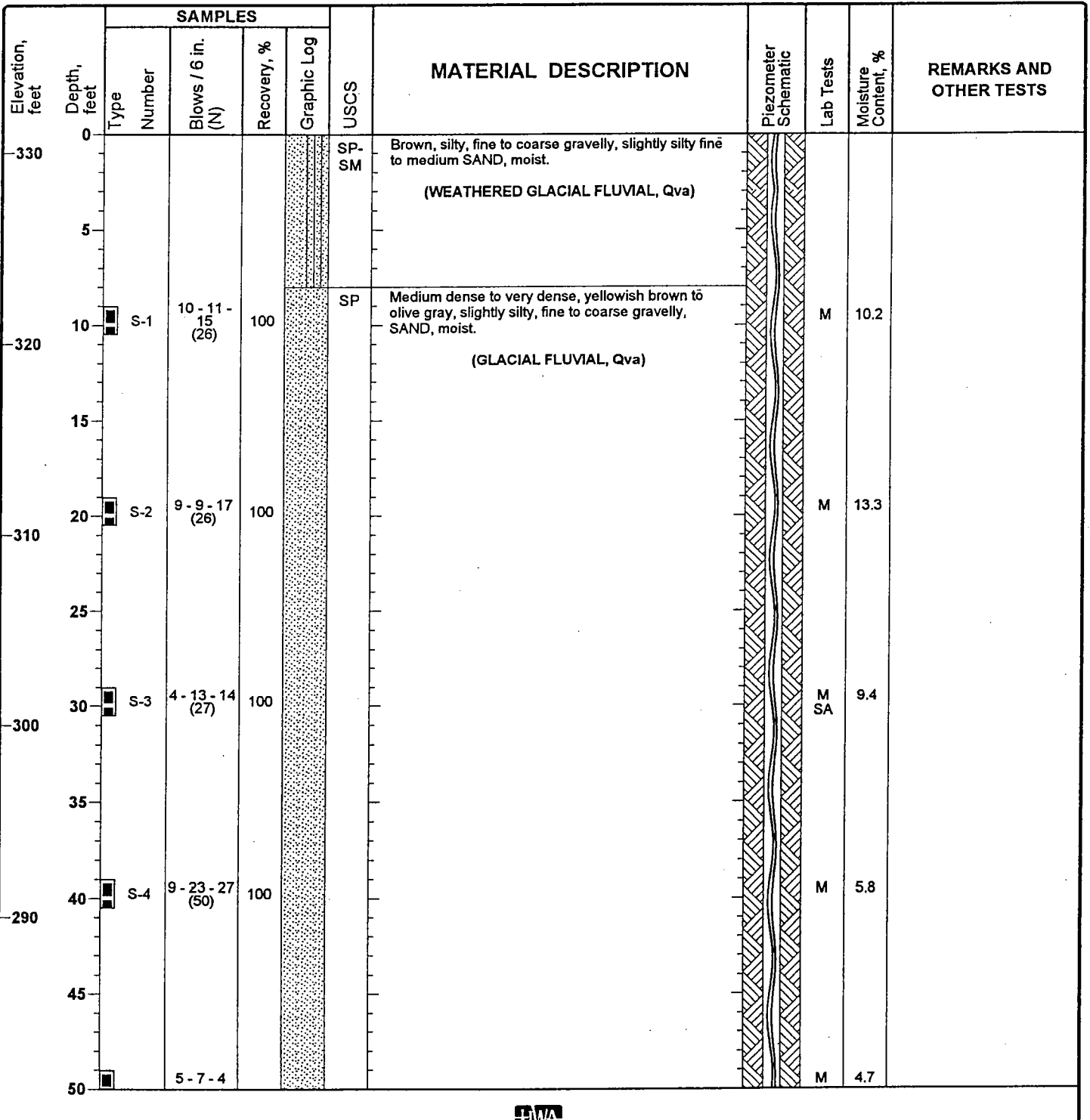
Geotechnical Boring Logs

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-3

Sheet 1 of 7

Date(s) Drilled: 1/3/02 - 1/7/02	Geotechnical Consultant: HWA GeoSciences Inc.	Logged By: BKH, BWT	Checked By: MLR/SEG
Drilling Method/ Rig Type: Becker Hammer/ Truck	Drilling Contractor: Layne Christensen Company	Total Depth of Borehole: 369.0 feet	
Drill Bit Size/Type: Dual Wall Reverse Circ.	Hammer Weight/Drop (lbs/in.): 300#, 30"	Ground Surface Elevation/Datum: 331 feet / NAVD88	
Location: On Edmonds Way.	Coordinates: N. 47.79084 W. 122.36443	Elevation Source: Plan	



(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-4.1

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 3

Sheet 2 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
280	50	S-5		(11)	100						
270	60	S-6		5 - 16 - 23 (39)	100				M SA	3.9	
260	70	S-7		7 - 12 - 36 (48)	100	SP	Dense, grayish brown, gravelly, fine to coarse SAND, wet.		M	19.9	
250	80	S-8				SP-SM	Dense, brown, slightly silty to clean, slightly gravelly, SAND, wet. Sand is fine to medium. Gravels are fine to coarse.		M	11.2	
240	90	S-9							M SA	21.3	
230	100	S-10							M	25.8	

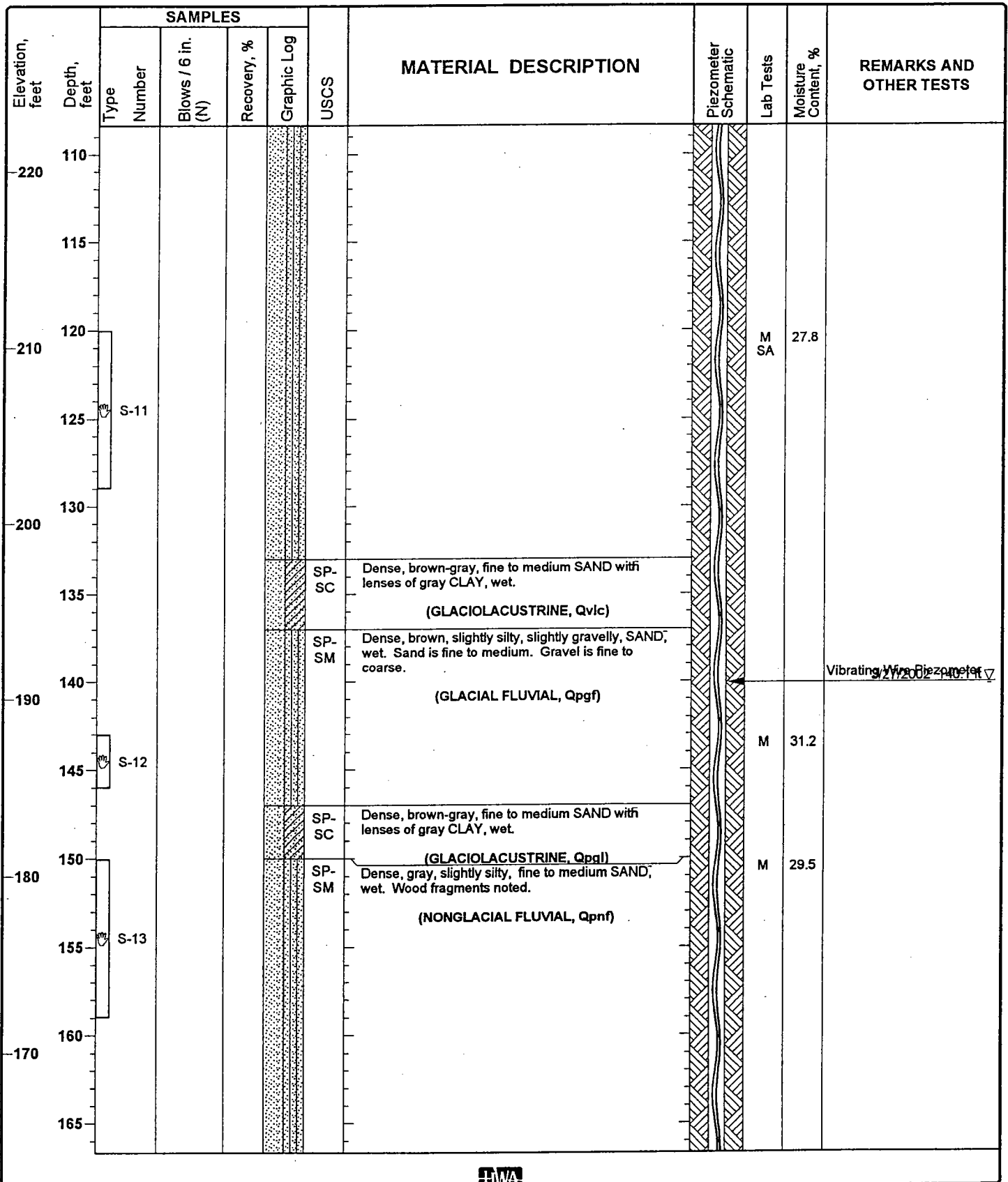
(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\19915349K.GPJ 5/1/02

Figure: A-4.2

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 3

Sheet 3 of 7



(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9815348K.GPJ 5/1/02



Figure: A-4.3

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-3

Sheet 4 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
160	170					SP-SM	Dense, gray, slightly silty, fine SAND, wet.		M	33.2	
	175		S-14								
150	180								M SA	39.1	
	185		S-15			SP-SC	Dense, gray, slightly silty, fine SAND, wet with lenses of gray, clay. Wood chunks noted.				
	190					SP-SM	Dense, gray, slightly silty fine SAND, wet. Wood fragments noted.				
140	195										
	200										
130	205		S-16			CL-ML	Hard, gray, silty CLAY, wet. (GLACIOLACUSTRINE, Qpgl)		M	37.5	
	210		S-17	8 - 14 - 23 (37)	100	CL	Hard, dark gray, CLAY, moist.		M SA AL HA	23.1	
120	215					CL-ML	Hard gray, SILT and CLAY, moist. Several thin interbedded fine SAND lenses.				
	220		S-18	12 - 12 - 20 (32)	100				M SA HA	29.3	
110	225										

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9815349K.GPJ 5/1/02



HWA GEOSCIENCES INC.

Figure: A-4.4

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-3

Sheet 5 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
225											
100	230	G-19a G-19b G-19c		9 - 18 - 24 (42)	100 100 100					26.4 31.9 27.9	
235						SM	Dense, gray, slightly silty, fine SAND, wet. Heaving. Interbedded thin silt/clay lenses. (GLACIAL FLUVIAL, Qpgf)		M SA DD M SA AL DD HA		
240						SP-SC	Dense to hard, interbedded, gray, CLAY, moist and silty SAND, wet. (GLACIOLACUSTRINE, Qpgl)				
245						SM	Dense, gray, slightly silty, fine to medium SAND, wet. (GLACIAL FLUVIAL, Qpgf)				
80	255	S-20				CL	Hard, dark gray, CLAY, moist. Occasional thin light gray, silty sand lenses, moist. (GLACIOLACUSTRINE, Qpgl)		M SA HA	29.2	Standpipe Piezometer
70	260	S-21		5 - 8 - 15 (23)	100						3/27/2002 257.81 ft 1/30/2002 257.99 ft
265											
60	270	S-22		6 - 12 - 23 (35)	100					30.8	
275											
50	280	S-23		6 - 10 - 13 (23)	100	CL	Very stiff, dark gray, silty CLAY, moist. Visible thin light gray banding.		M SA AL	26.3	

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9915348K.GPJ 5/1/02



Figure: A-4.5

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-3

Sheet 6 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
285		S-24		6 - 9 - 12 (21)	100			M SA	27.7		
290	40	S-25		8 - 11 - 14 (25)	100			M SA AL HA	24.9		
295		S-26a S-26b S-26c		8 - 18 - 36 (54)	100 100 100	SP-SM	Very dense, dark gray, slightly silty to silty, fine to medium SAND, wet. Heaving. (GLACIAL FLUVIAL, Qpgf) From 298 to 299, SILT present.	M SA M SA M SA DS DD HA M M SA	18.6 23.1 25.2		
300	30	S-27		4 - 15 - 37 (52)	100			DD HA M M SA	22.8		
305		S-28		0 - 0 - 0 (0)	100			M SA	25.8		
310	20	S-29		5 - 11 - 15 (26)	100	SP-SM	Medium dense, dark gray, slightly silty to clean, fine to medium SAND, wet. Heaving.	M SA	21.6		
315		S-30		5 - 10 - 20 (30)	100			M	27.4		
320	10										
325		S-31				SP	Medium dense, dark gray, clean, fine to coarse SAND, wet.	M	27.9		
330	0	S-32						M SA	23.8		
335		S-33 S-34a S-34b S-34c S-34d S-35 S-36		12 - 16 - 32 (48)	100 100 100 100	CL	Hard, dark olive gray to gray CLAY with fine sand lenses, moist.	M M SA AL HA M SA M SA	23.7 20.8 22.2 24.5 23.2 25.5 25.9		
340	-10					SP-SM	(NONGLACIAL LACUSTRINE, Qpnl) Dense, gray, clean to slightly silty, fine to medium SAND, wet. <<(NONGLACIAL FLUVIAL, Qpnf)				

(Ver.1.1 Jan02RWSP-RWSP.GLB.RWSP.GDT) H:\GINTW\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-4.6

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 3

Sheet 7 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
345								AL M SA HA M M			
		☐	S-37					M SA	26.2		
350	-20					OL	Hard, brown, SILT with chunks of compressed PEAT, wet. (OLDER PEAT, Qpnp)				
		☐	S-38					M SA	37.2		
355		☐	S-39a	70	100			AL HA M SA	30.4		
		☐	S-39b		100			AL HA M SA	31.0		
		☐	S-39c		100			AL HA M SA	29.1		
		☐	S-40			SP-SM	Medium dense, gray, clean to slightly silty, fine to medium SAND, wet. Occasional piece of decomposed wood. (NONGLACIAL FLUVIAL, Qpnf)				
360	-30							M SA M SA M SA	25.3		
		☐	S-41				Encountered large piece of wood that appears to be driftwood.	M	32.4		
		☐	S-42								
370	-40						Bottom of boring at 369 feet. 2" piezometer installed from 295 to 315 feet. Vibrating wire piezometer installed at 250 feet.				
375											
380	-50										
385											
390	-60										
395											
400											

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-4.7

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-3	S-1	9.0 - 10.5	10.2													SP-SM	yellowish-brown, poorly graded SAND with silt and gravel
MW-3	S-2	19.0 - 20.5	13.3													SM	yellowish-brown, silty SAND with gravel
MW-3	S-3	29.0 - 30.5	9.4									1.1	93.7	5.1		SP-SM	grayish-brown, poorly graded SAND with silt
MW-3	S-4	39.0 - 40.5	5.8													SP	grayish-brown, poorly graded SAND with gravel
MW-3	S-5	49.0 - 50.5	4.7													SP	grayish-brown, poorly graded SAND
MW-3	S-6	59.0 - 60.5	3.9									6.3	89.0	4.7		SP	grayish-brown, poorly graded SAND
MW-3	S-7	69.0 - 70.5	19.9													SP	dk. grayish-brown, poorly graded SAND
MW-3	S-8	77.0 - 79.0	11.2													SP-SM	dk. grayish-brown, poorly graded SAND with silt
MW-3	S-9	87.0 - 89.0	21.3									8.3	86.7	5.1		SP-SM	olive-brown, poorly graded SAND with silt
MW-3	S-10	97.0 - 99.0	25.8													SP-SM	dk. grayish-brown, poorly graded SAND with silt
MW-3	S-11	120.0 - 129.0	27.8									7.6	89.3	3.1		SP	grayish-brown, poorly graded SAND
MW-3	S-12	143.0 - 146.0	31.2													SP-SM	dk. grayish-brown, poorly graded SAND with silt
MW-3	S-13	150.0 - 159.0	29.5													SP-SM	gray, poorly graded SAND with silt
MW-3	S-14	170.0 - 180.0	33.2													SP-SM	gray, poorly graded SAND with silt
MW-3	S-15	180.0 - 190.0	39.1									0.1	7.7	92.1		CL	gray, lean CLAY
MW-3	S-16	202.0 - 208.0	37.5													CL-ML	gray, silty lean CLAY
MW-3	S-17	209.0 - 210.5	23.1							40	23		0.2	99.8		CL	gray, lean CLAY
MW-3	S-18	219.0 - 220.5	29.3										1.5	98.5		CL	gray, lean CLAY
MW-3	S-19a	229.0 - 229.5	26.4													SM	gray, silty SAND
MW-3	S-19b	229.5 - 230.0	31.9	122.2	92.6									95.4		CL-ML	gray, silty CLAY

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWAGEOSCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF MATERIAL PROPERTIES

PAGE: 1 of 3

PROJECT NO.: 99153-490

FIGURE: A-4.21

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-3	S-19c	230.0 - 230.5	27.9	120.4	94.1					39	20		20.7	79.3	3.6	CL	gray, lean CLAY with sand
MW-3	S-20	254.0 - 259.0	29.2										6.9	93.1		CL	gray, lean CLAY
MW-3	S-21	259.0 - 260.5	40.1							84	30			99.7		CH	gray, fat CLAY
MW-3	S-22	269.0 - 270.5	30.8			644				68	26		5.3	94.7	7	CH	gray, fat CLAY
MW-3	S-23	279.0 - 280.5	26.3							42	21			99.9		CL	gray, lean CLAY
MW-3	S-24	285.0 - 286.5	27.7											100.0		CL	gray, lean CLAY
MW-3	S-25	289.0 - 290.5	24.9							45	26		0.0	100.0	5.4	CL	gray, lean CLAY
MW-3	S-26a	295.0 - 295.5	18.6													SP-SM	dark gray, poorly graded SAND with silt
MW-3	S-26b	295.5 - 296.0	23.1	131.8	107.1			10	39.8				37.6	62.4		ML	gray, SILT, sand lenses
MW-3	S-26c	296.0 - 296.5	25.2													SP-SM	dark gray, poorly graded SAND with silt
MW-3	S-27	299.0 - 300.5	22.8									0.0	85.2	14.7		SM	gray, silty SAND
MW-3	S-28	305.0 - 306.5	25.8											35.9		SM	gray, silty SAND
MW-3	S-29	309.0 - 310.5	21.6											9.8		SP-SM	dark gray, poorly graded SAND with silt
MW-3	S-30	314.0 - 315.0	27.4													SP-SM	dark gray, poorly graded SAND with silt
MW-3	S-31	325.0 - 326.0	27.9													SP-SM	gray, poorly graded SAND with silt
MW-3	S-32	328.0 - 329.0	23.8									0.8	97.5	1.8		SP	dark gray, poorly graded SAND
MW-3	S-33	334.0 - 335.0	23.7													CL	dark olive gray, lean CLAY, sand layers
MW-3	S-34a	335.0 - 335.5	20.8							40	22			97.8	5	CL	gray, lean CLAY
MW-3	S-34b	335.5 - 336.0	22.2										3.0	97.0		CL	olive gray, lean CLAY
MW-3	S-34c	336.0 - 336.5	24.5							29	26			97.6		ML	gray, SILT, interbedded with lean CLAY

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWA
GEO SCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

**SUMMARY OF
MATERIAL PROPERTIES**

PAGE: 2 of 3

PROJECT NO.: 99153-490

FIGURE: A-4.22

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-3	S-34d	336.5 - 337.0	23.2			472							1.3	98.7	2.3	ML	dark olive-gray, SILT
MW-3	S-35	337.0 - 337.5	25.5													CL	olive gray, lean CLAY, sand layers
MW-3	S-36	338.0 - 339.0	25.9													SP-SM	olive gray, poorly graded SAND with silt
MW-3	S-37	347.0 - 348.0	26.2									92.3	7.7			SP-SM	gray, poorly graded SAND with silt
MW-3	S-38	353.0 - 355.0	37.2							41	30	5.8	94.2	8.2		ML	olive-gray, SILT
MW-3	S-39a	355.0 - 355.5	30.4							35	30	7.1	92.9	9.2		ML	olive gray, SILT
MW-3	S-39b	355.5 - 356.0	31.0										91.6	2.8		ML	olive-gray, SILT
MW-3	S-39c	356.0 - 356.5	29.1										97.8			ML	olive-gray, SILT
MW-3	S-40	358.0 - 359.0	25.3									0.3	88.8	10.9		SP-SM	gray, poorly graded SAND with silt
MW-3	S-41	364.0 - 365.0	32.4													SP-SM	olive gray, poorly graded SAND with silt, wood pieces
MW-3	S-42	367.5 - 369.0															

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 1 of 8

Date(s) Drilled	1/25/02 - 2/1/02	Geotechnical Consultant	HWA GeoSciences Inc.	Logged By	BKH	Checked By	MLR/SEG
Drilling Method/ Rig Type	Becker Hammer/ Truck	Drilling Contractor	Layne Christensen Company	Total Depth of Borehole	446.5 feet		
Drill Bit Size/Type	Dual Wall Reverse Circ.	Hammer Weight/Drop (lbs/in.)	300#, 30"	Ground Surface Elevation/Datum	387 feet / NAVD88		
Location	Firdale Village	Coordinates	N. 47.77823 W. 122.36113	Elevation Source	Plan		

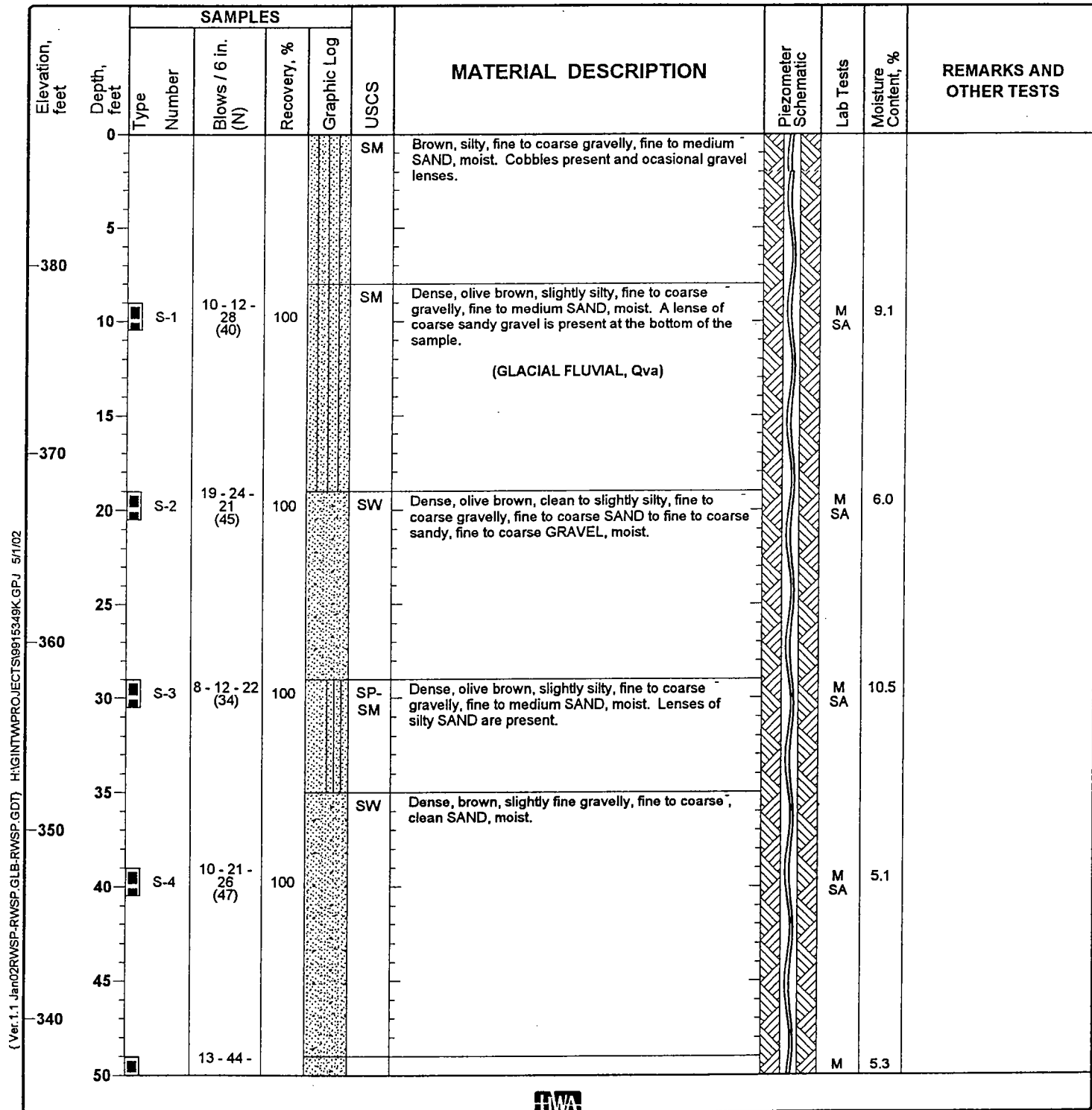


Figure: A-5.1

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 2 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
50		S-5		28 (72)	100	SW	Very dense, grayish brown, silty, fine to medium SAND, moist. Large gravel/cobble present in middle of sampler.		SA		
55											
330											
60		S-6		22 - 24 - 23 (47)	100	SP	Dense, brownish gray, slightly fine to coarse gravelly, fine to medium SAND, moist. Some coarse sand present. A large fractured cobble is present in the middle of the sampler. The bottom 3 inches of the sample becomes fine sand.		M SA	13.3	
65											
320											
70		S-7		13 - 28 - 35 (63)	100	SP	Very dense, brownish gray, fine to medium SAND, moist.		M SA	4.8	
75											
310											
80		S-8		8 - 13 - 8 (21)	100	SP	Medium dense, grayish brown, fine to medium SAND, moist.		M SA	5.2	
85											
300											
90		S-9		9 - 19 - 24 (43)	100	SP	Dense, grayish brown, slightly fine gravelly, fine to medium SAND, moist.		M SA	4.9	
95											
290											
100		S-10		18 - 36 - 50/6 (100+)	100	SP	Very dense, grayish brown, slightly fine gravelly, fine to medium SAND, moist to wet.		M SA	6.6	
105											
280											

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTWP\PROJECTS\19915349K\GPJ 5/1/02

Figure: A-5.2

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 3 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
110	110	S-11	9 - 30 - 50/5 (100+)	100	[Stippled]	SP	Very dense, grayish brown, fine to medium SAND, wet. Bottom 3 inches of sample was fine to coarse SAND, wet.	M SA	16.8		
120	120	S-12	4 - 24 - 50/5 (100+)	100	[Stippled]	SP	Very dense, brown, slightly fine gravelly, fine to coarse SAND, wet. 3 feet of heave present at 119 feet.	M SA	17.3		
130	130	S-13			[Stippled]	SP	5 feet of heave present at 129 feet. No sample taken.	M SA	35.4		
135	135	S-14			[Stippled]	SP	Brown, slightly fine gravelly, fine to medium SAND, wet.	M SA	26.1		
155	155	S-15			[Stippled]	SP	Cuttings: Gray, slightly fine to coarse gravelly, fine to medium SAND, wet. Organics, including wood, and some coarse sand present.	M SA	23.6		
160	160				[Stippled]		Cuttings: Gray, slightly fine to coarse gravelly, fine to coarse SAND, wet. Wood pieces present.				
165	165				[Stippled]		Cuttings: Same as above - gray SAND, wet.				

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTWP\PROJECTS\9515349K.GPJ 5/1/02

Figure: A-5.3

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 4 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
220											
170							Cuttings: Gray, slightly fine to coarse gravelly, fine to medium SAND, wet.		M SA	22.9	
175		S-16									
210											
180											
185											
200						ML	Hard, dark gray, fine sandy, SILT, moist. (GLACIOLACUSTRINE, Qv1c)				
190		S-17a S-17b	8 - 22 - 28 (50)	100 100		SP-SM	Very dense, dark gray, slightly silty to silty, fine SAND, wet. (NONGLACIAL FLUVIAL, Qpnf)		M SA HA M SA	21.4 23.5	
195							Cuttings: Gray, fine to medium SAND, wet. Pieces of wood present.				
190											
200											
205							Cuttings: Dark gray, slightly silty to silty, fine to medium SAND, wet.		M SA	28.3	
180		S-18									
210							Cuttings: Dark gray, slightly silty to silty, fine SAND, wet.		M SA	34.4	
215		S-19									
170											
220							Cuttings: Similar as above - silty fine SAND, wet.				
225											

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9915348K.GPJ 5/1/02



HWA GEOSCIENCES INC.

Figure: A-5.4

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 5 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
160	225	S-20	2-13-28 (41)	100		ML	Dark gray, slightly silty, fine SAND, wet. Organics, including wood fragments, present.		M SA	26.5	
230							(NONGLACIAL LACUSTRINE, Qpnl)				
235							Cuttings: Silty SAND to sandy SILT. Some organics present.				
150	240	S-21	9-18-30 (46)	100		CL	Hard, dark gray, fat CLAY, moist.		M SA AL	24.2	
245							Cuttings: hard, dark gray, CLAY with light gray seams.				
140	250	S-22a S-22b	9-13-28 (41)	93 133		ML CL	Hard, dark gray, CLAY, moist. Dense, dark gray, fine sandy SILT, moist to wet. Light gray seams and fine sandy clay lenses present.		M SA AL HA M SA	27.7 27.5	
255											
130	260	S-23	5-10-34 (44)	100			Cuttings: Clay with sand and silt lenses.		M SA HA	29.7	
265							Cuttings: Hard, dark gray, lean to fat CLAY, moist. Light gray seams present.				
120	270	S-24	10-13-18 (31)	100			Hard, dark gray, lean CLAY, moist. Light gray seams present.		M AL	21.9	
275							Cuttings are silty SAND, wet between 273 and 276 feet.				
110	280	S-25a S-25b S-25c	8-8-12 (20)	100 100 100		CL SM ML SM	Very stiff, dark gray, CLAY, moist. Medium dense, dark gray, silty fine SAND, wet. Very stiff, dark gray, fine sandy SILT, moist to wet. Dark gray, SILT, wet.		M SA AL HA M	31.0 27.7 30.6	

(Ver. 1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-5.5

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 6 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
285						CL	Hard, dark gray, lean CLAY, moist.		SA M SA		
290	290	S-26		8 - 12 - 18 (30)	100				M SA AL HA	22.7	
295											
300	300	S-27		7 - 11 - 21 (32)	100	SM	Dense, dark gray, slightly silty, fine SAND, wet. Cuttings: Dark gray, silty SAND, wet.		M	25.1	
305											
310	310	S-28		9 - 12 - 18 (30)	100	CL-ML	Hard, dark gray, lean to fat CLAY, moist.		M SA AL HA	22.8	Standpipe Piezometer
315											1/5/2002 314 ft ▽
320	320	S-29		8 - 10 - 16 (26)	100		At 315, cuttings contain a thin lens of hard, greenish gray, laminated CLAY, moist. The Lens is very brittle. Very stiff, dark gray, CLAY, moist. Light gray seams present.		M	32.7	
325	325	S-30		7 - 12 - 16 (28)	100		Very stiff, dark gray, CLAY, moist. Light gray seams present.		M SA AL HA	25.7	
330	330	S-31		8 - 18 - 40 (58)	100	ML	Very dense, dark gray, fine sandy SILT to silty fine SAND, wet.		M SA	45.7	
335	335	S-33		8 - 11 - 18 (29)	100	CL	Very stiff, dark gray, lean CLAY, moist. Light gray seams present. At the top of the clay was a thin, hard, silt lens. The lens was about 1 mm thick and contained organics.		M SA AL HA	26.0	
340	340	S-34		9 - 13 - 18 (31)	100				M	26.4	

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\91915349K.GPJ 5/1/02



HWA GEOSCIENCES INC.

Figure: A-5.6

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 7 of 8

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9915349K.GPJ 5/1/02

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
345	40	S-35a S-35b	4 - 11 - 22 (33)	100 100	SP- SM	Dense, dark gray, silty fine SAND, wet.		M SA	28.8 28.1		
350		S-36	3 - 11 - 20 (31)	100	ML	Hard, dark gray, SILT to fine sandy SILT, moist to wet.		M SA HA M SA	28.1		
355	30	S-37a S-37b	5 - 13 - 23 (36)	93 133	SM CL	Dense, dark gray, silty fine SAND, wet. Lenses of organics present. Hard, dark gray, lean CLAY, moist.		M SA M	24.3 23.4		
360		S-38a S-38b	15 - 25 - 47 (72)	100	ML	Hard, greenish gray, fine sandy SILT, moist to wet. Organics present.		M SA AL M SA HA M SA	17.1 23.4		
365	20	S-39			SP	Dark gray, fine to coarse gravelly, slightly silty, fine to medium SAND, wet. Organics, including pieces of wood, present. Lenses of silt/clay present in sand. (NONGLACIAL FLUVIAL, Qpnf)		M SA HA M SA	23.8		
370		S-40				5 feet of heave present at 369 feet. Dark gray, fine to medium SAND, wet. Pieces of wood present.		M SA	23.8 25.0		
375	10	S-41									
380		S-42									
385	0	S-43	7 - 12 - 27 (39)	100	ML SM	Cuttings: Light greenish gray, SILT, moist with organics. Very dense, gray, silty fine SAND, moist. Abundant organics, including pieces of wood, present.		M SA HA M SA	35.9 31.0		
390		S-44	16 - 50 - 50/3 (100+)	100		Dense, dark gray with light gray and greenish gray streaks, silty, fine SAND, moist. Abundant organics, including pieces of wood, present.		M SA HA	25.3		
395	-10	S-45			SP	Heaving sand present. No SPT sample taken. Cuttings consist of dark gray, fine to medium SAND, wet.		M SA	22.1		
400								M	20.8		

Figure: A-5.7

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 4

Sheet 8 of 8

(Ver.1.1 Jan02RWSP-RWSP.GLB.RWSP.GDT) H:\GINT\PROJECTS\9815349K.GPJ 5/1/02

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
400		S-46					Heaving sand present. No SPT sample taken. Cuttings consist of dark gray, fine to medium SAND, wet.		SA HA		
		S-47				CL	Cuttings: Brown to gray, CLAY, moist. According to the driller, this clay lens is only 1.5 to 2 feet thick.		M SA	32.9	
405		S-48				SP	35 feet of heave present at 405 feet. No SPT sample taken. Cuttings: Gray, gravelly, fine to coarse SAND, wet. Gravel is fine.		M SA	14.8	
-20											
410		S-49				CL- ML	Cuttings: Gray, SILT, moist. (NONGLACIAL LACUSTRINE, Qngl)		M	28.9	
415		S-50	11 - 35 - 50/5 (100+)	100			Hard, gray, organic SILT, moist. Abundant compressed peat layers present. Compressed peat is platy and breaks into sheets.		M AL	32.7	
-30											
420						SP	Heaving sands present at 420 feet.				
425											
-40		S-51a S-51b S-51c	18 - 36 - 50/5 (100+)	100 100 100		OL	Hard, gray, organic SILT, moist. Abundant wood fragments/organics present.		M M SA M SA HA M	25.6 34.9 32.7	
430		S-52					Cuttings: Brown, compressed peat. Cuttings: Gray silt.			47.5	
435		S-53 S-54	35 - 50/2 (100+)	83		GP	Cuttings: Dark gray, fine to coarse sandy, GRAVEL, wet.		M M SA	7.7 8.2	
-50		S-55 S-56	30 - 50/3 (100+)	83			(NONGLACIAL FLUVIAL, Qpnf) Very dense, gray, fine to coarse GRAVEL, wet. Cuttings: Dark gray, fine to coarse sandy, fine to coarse GRAVEL, wet.		M SA	4.9	
440											
445		S-57 S-58 S-59	7 - 16 16	100			Very dense, gray, fine sandy GRAVEL, moist.		M SA M SA	5.1 13.3	
-60							Bottom of hole at 446.5 feet. 2" piezometer installed from 404 to 424 feet. Vibrating wire piezometer installed at 226 feet.				
450											
455											
-70											



HWA GEOSCIENCES INC.

Figure: A-5.8

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (pcf)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-4	S-1	9.0 - 10.5	9.1											7.8		SP-SM	olive-brown, poorly graded SAND with silt and gravel
MW-4	S-2	19.0 - 20.5	6.0									33.4	59.6	7.0		SP-SM	olive-brown, poorly graded SAND with silt and gravel
MW-4	S-3	29.0 - 30.5	10.5											7.1		SP-SM	olive-brown, poorly graded SAND with silt
MW-4	S-4	39.0 - 40.5	5.1									16.4	79.3	4.3		SP	grayish-brown, poorly graded SAND with gravel
MW-4	S-5	49.0 - 50.5	5.3											12.4		SM	olive-brown, silty SAND
MW-4	S-6	59.0 - 60.5	13.3									4.3	81.2	14.6		SM	grayish-brown, silty SAND
MW-4	S-7	69.0 - 70.5	4.8											3.6		SP	grayish-brown, poorly graded SAND
MW-4	S-8	79.0 - 80.5	5.2									2.6	91.1	6.3		SP-SM	lt. olive-brown, poorly graded SAND with silt
MW-4	S-9	89.0 - 90.5	4.9											5.3		SP-SM	olive-gray, poorly graded SAND with silt
MW-4	S-10	99.0 - 100.5	6.6									5.6	88.6	5.9		SP-SM	olive-gray, poorly graded SAND with silt
MW-4	S-11	109.0 - 110.5	16.8											5.2		SP-SM	olive-gray, poorly graded SAND with silt
MW-4	S-12	119.0 - 120.5	17.3									2.4	95.2	2.4		SP	olive-gray, poorly graded SAND
MW-4	S-13	129.0 - 129.0	35.4											7.2		SP-SM	brown, poorly graded SAND with silt
MW-4	S-14	133.0 - 136.0	26.1											3.6		SP	lt. grayish-brown, poorly graded SAND
MW-4	S-15	155.0 - 160.0	23.6									0.6	95.9	3.6		SP	gray, poorly graded SAND
MW-4	S-16	170.0 - 180.0	22.9									4.8	91.6	3.6		SP	gray, poorly graded SAND
MW-4	S-17a	189.0 - 190.0	21.4							NP	NP		6.6	93.4	2.9	ML	gray, SILT
MW-4	S17b	190.0 - 190.5	23.5											7.3		SP-SM	gray, poorly graded SAND with silt
MW-4	S-18	205.0 - 209.0	28.3									5.0	79.1	15.9		SM	gray, silty SAND
MW-4	S-19	210.0 - 219.0	34.4											10.0		SP-SM	gray, poorly graded SAND with silt

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 1 of 4

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (pcf)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-4	S-20	226.0 - 228.5	26.5											92.5		ML	gray, SILT
MW-4	S-21	239.0 - 240.5	24.2							36	27			100.0		ML	gray, SILT
MW-4	S-22a	249.0 - 250.2	27.7							59	25		0.0	100.0		CH	gray, fat CLAY
MW-4	S-22b	250.2 - 250.5	27.5											99.4		ML	olive-gray, SILT
MW-4	S-23	259.0 - 260.5	29.7							NP	NP		0.0	100.0		ML	olive-gray, SILT
MW-4	S-24	269.0 - 270.5	21.9							39	21					CL	gray, lean CLAY
MW-4	S-25a	279.0 - 279.5	31.0							57	25		5.3	94.7		CH	gray, fat CLAY
MW-4	S-25b	279.5 - 280.0	27.7											29.8		SM	gray, silty SAND
MW-4	S-25c	280.0 - 280.5	30.6											99.3		ML	dk. olive-gray, SILT
MW-4	S-26	289.0 - 290.5	22.7							46	22		0.0	100.0	2.5	CL	gray, lean CLAY
MW-4	S-27	299.0 - 300.5	25.1													SP-SM	gray, poorly graded SAND with silt
MW-4	S-28	309.0 - 310.5	22.8							43	22			100.0	3.2	CL	gray, lean CLAY
MW-4	S-29	319.0 - 320.5	32.7													CH	gray, fat CLAY
MW-4	S-30	325.0 - 326.5	25.7							48	26		0.0	100.0	2.8	CL	gray, lean CLAY
MW-4	S-31	329.0 - 330.5	45.7										34.6	65.4		ML	dark gray, sandy SILT
MW-4	S-32	332.5 - 333.0	33.7													CL	gray, lean CLAY
MW-4	S-33	335.0 - 336.5	26.0							50	26		0.9	99.1		CH	gray, fat CLAY
MW-4	S-34	339.0 - 340.5	26.4													CL	gray, lean CLAY
MW-4	S-35a	345.0 - 345.5	28.8											31.0		SM	dark gray, silty SAND
MW-4	S-35b	345.5 - 346.5	28.1										3.8	96.2		ML	dk. olive-gray, SILT

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW- 4	S-36	349.0 - 350.5	28.1											95.3		ML	dk. olive-gray, SILT
MW- 4	S-37a	355.0 - 356.2	24.3											43.3		SM	dark gray, silty SAND
MW- 4	S-37b	356.2 - 356.5	23.4													CL	dk. olive-gray, lean CLAY
MW- 4	S-38a	359.0 - 359.5	17.1							36	21			99.9		CL	gray, lean CLAY
MW- 4	S-38b	359.5 - 360.5	23.4									8.8	91.2			ML	gray, SILT
MW- 4	S-39	362.0 - 365.0	23.8									1.0	93.5	5.5		SP-SM	gray, poorly graded SAND with silt
MW- 4	S-40	369.0 -	23.8													SP	gray, poorly graded SAND
MW- 4	S-41	370.0 - 374.0	25.0									0.0	92.6	7.4		SP-SM	gray, poorly graded SAND with silt
MW- 4	S-42	383.0 - 384.0	35.9										18.6	81.4		ML	gray, SILT with sand
MW- 4	S-43	385.0 - 386.5	31.0											86.6		ML	olive-gray, SILT
MW- 4	S-44	390.0 - 391.5	25.3										23.8	76.2		ML	olive-gray, SILT with sand
MW- 4	S-45	395.0 - 397.0	22.1											17.4		SM	olive-gray, silty SAND
MW- 4	S-46	399.0 - 401.0	20.8									0.8	89.7	9.6		SP-SM	gray, poorly graded SAND with silt
MW- 4	S-47	403.0 - 404.0	32.9											91.9		CL	olive-brown, lean CLAY
MW- 4	S-48	405.0 - 406.0	14.8									21.8	76.3	1.9		SP	gray, poorly graded SAND
MW- 4	S-49	411.0 - 412.0	28.9													ML	dk. grayish-brown, SILT with sand
MW- 4	S-50	415.0 - 416.5	32.7							45	38				12.2	ML	olive-gray, SILT with sand
MW- 4	S-51a	428.0 - 428.5	25.6							NP	NP					SP-SM	gray, poorly graded SAND with silt
MW- 4	S-51b	428.5 - 429.0	34.9											77.1		ML	dk. olive-gray, SILT with sand, thin interbedded organics
MW- 4	S51c	429.0 - 429.5	32.7										6.7	93.3		ML	dk. olive-gray, SILT, thin interbedded organic layers

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW- 4	S-52	432.0 - 433.0	47.5													ML	olive-gray, SILT, thin interbedded organic layers
MW- 4	S-53	435.0 - 435.7	7.7													GP	olive-gray, poorly graded GRAVEL with sand
MW- 4	S-54	436.0 - 437.0	8.2									68.0	31.1	1.0		GW	gray, well graded GRAVEL with sand
MW- 4	S-55	437.5 - 438.3														GW	olive-gray, well graded GRAVEL with sand
MW- 4	S-56	438.0 - 438.5	4.9									85.7	14.3	0.0		GW	gray, well graded GRAVEL with sand
MW- 4	S-57	444.0 - 444.5	5.1									75.3	24.7	0.0		GW	gray, well graded GRAVEL with sand
MW- 4	S-58	444.5 - 445.0	13.3									32.2	67.0	0.8		SW	gray, well graded SAND with gravel
MW- 4	S-59	445.0 - 446.5														GP	olive-gray, poorly graded GRAVEL with silt and sand

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWA GEOSCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 4 of 4

PROJECT NO.: 99153-490

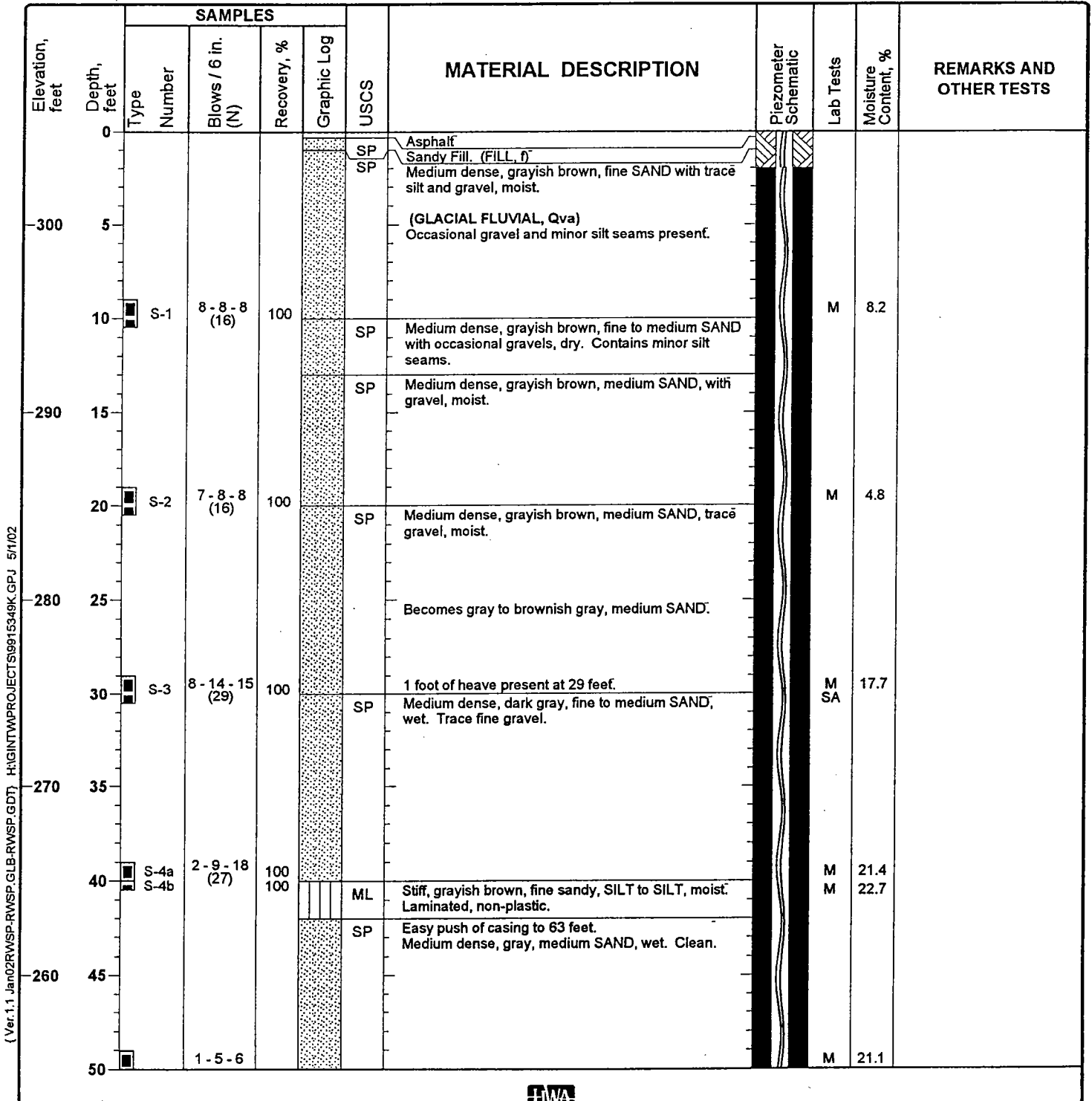
FIGURE: A-5.33

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 1 of 7

Date(s) Drilled	11/27/01 - 12/7/01	Geotechnical Consultant	HWA GeoSciences Inc.	Logged By	SEG, BKH	Checked By	MLR/SEG
Drilling Method/ Rig Type	Becker Hammer/ Truck	Drilling Contractor	Layne Christensen Company	Total Depth of Borehole	352.0 feet		
Drill Bit Size/Type	Dual Wall Reverse Circ.	Hammer Weight/Drop (lbs/in.)	300#, 30"	Ground Surface Elevation/Datum	305 feet / NAVD88		
Location	Lake Ballinger Pump Station	Coordinates	N. 47.77739 W. 122.33416	Elevation Source	Plan		



(Ver. 1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-6.1

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 2 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
50		S-5		(11)	100		1' of heave present at 50 feet.		SA		
250	55										
	60	S-6					5' of heave present at 60 feet. No sample taken.				
240	65					ML	Stiff to hard, dark gray, massive to laminated, low plastic SILT, dry. (GLACIOLACUSTRINE, Qv1c)				
	70	S-7		7 - 10 - 15 (25)	100	ML	Hard, dark gray, slightly fine sandy SILT, moist. Laminated, non to slightly plastic.		M	28.4	
230	75						Becomes drier, massive, more clayey with slightly wavy laminae.				
	80	S-8		5 - 9 - 14 (23)	100	CL	Hard, dark gray, lean CLAY, laminated, moist. low plastic.		M	25.3	
220	85						A few gravels present at 86 feet.				
	90	S-9		7 - 8 - 10 (18)	100	ML	Hard, dark gray, SILT, moist. Laminated and minor slickensides present. Low plasticity.		M SA AL HA	26.1	
210	95										
	100	S-10			100	CL	Stiff to hard, dark gray, massive to laminated, lean CLAY, dry.		M	24.4	
200	105					SM	Very dense, gray, silty SAND with gravel (diamicton) with beds of interlaminated silty SAND and sandy SILT, wet. (GLACIAL TILL, Qpgt)				

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-6.2

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 3 of 7

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTWP\PROJECTS\9915349K.GPJ 5/1/02

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
110		S-11		8 - 20 - 15 (35)	100	SM	Very dense, dark gray, very silty, SAND with gravel, dry.	[Piezometer Schematic]	M SA	12.0	
115							Boulder encountered at 115 feet.				
120		S-12		5 - 18 - 18 (36)	100	SP-SM	Dense, dark gray to brownish gray, interbedded medium SAND with gravel to silty fine SAND, wet. Encountered water at 118 feet. (GLACIAL FLUVIAL, Qpgf)	[Piezometer Schematic]	M SA	13.3	
130		S-13					5 feet of heave present at 129 feet. No sample taken. Cuttings are silty SAND.				
140		S-14		71	100	SM	Very dense, dark gray, silty SAND to sandy SILT with gravel, moist to wet. TILL-LIKE. (GLACIAL TILL, Qpgt)		M SA	15.2	
150		S-15		81	100			[Piezometer Schematic]	M	9.7	
155						GP	Becomes sandy gravel and wet at 152 feet.				
160		S-16		5 - 19 - 30 (49)	44	SM	Dense, dark gray, silty medium SAND to fine sandy SILT interbeds. Mostly massive with silty laminae. 6 feet of heave present at 158 feet.	[Piezometer Schematic]	M SA	22.4	Vibrating Wire Piezometer 3/27/2002 158 ft ▽ 1/30/2002 158.6 ft ▽
165						ML	Hard, olive gray to dark gray, SILT, moist. Light and				Standpipe Piezometer 3/27/2002 165.9 ft ▽

Figure: A-6.3

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 4 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
170	170	S-17		90	83		dark draped laminations present. A few drop stones present. (GLACIOLACUSTRINE, Qpgl)		M	16.5	1/30/2002 166.79 ft
130	175						Becomes more clayey at 175 feet.				
180	180	S-18		6 - 14 - 17 (31)	100	ML	Hard, dark gray, slightly sandy SILT, moist. Faintly laminated to massive. Low plasticity. Sand is fine to medium.		M SA AL HA	26.8	
120	185						Becomes cobbly at 185 feet.				
190	190	S-19		70	100	ML	Hard, dark gray, SILT, with trace fine sand, dry. Faintly laminated to massive.		M	23.8	
110	195					SM	Between 194 and 196 feet, silty sand interbeds present.				
200	200	S-20		10 - 18 - 27 (45)	100	CH	Hard, dark gray, fat CLAY with trace fine sand, dry. Laminations are contorted. Moderate plasticity when wet. Appears carbonaceous.		M SA AL HA	19.2	
100	205										
210	210	S-21			83	SM	Very dense, dark gray, silty, fine to coarse SAND, moist. Some cobbles present. Till-like. (GLACIAL TILL, Qpgt)		M	8.2	
90	215										
220	220	S-22		87	83				M	10.2	
80	225										

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTWAPROJECTS\9915349K.GPJ 5/1/02



HWA GEOSCIENCES INC.

Figure: A-6.4

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 5 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
225											
	230		S-23			SM	At 227.5, soil becomes wet. Cuttings are soupy, dark gray, silty SAND. Blows counts decreased. (GLACIAL FLUVIAL, Qpgf) 7 feet of heave present at 229 feet. No sample taken.				
70	235										
	240		S-24	40/1.5 feet	100		1.5 feet of heave present. Drove sampler 2 feet. Dense, dark gray, slightly silty, fine to medium SAND, wet.		M SA	20.9	
60	245		S-25	50	83	SM	Very dense, dark gray, fine to coarse gravelly, silty, fine SAND, moist. TILL-LIKE. (GLACIAL TILL, Qpgt)		M	9.1	
	250		S-26	25 - 21 - 51 (72)	33	SM CL	Very dense, dark gray, silty, fine SAND, moist. Hard, dark gray, lean CLAY, moist. Fine sand lenses present. Between 250 and 254 feet, shells noted in cuttings.		M M	18.6 18.6	
50	255		S-27	50	83	SM	Very dense, dark gray, fine to coarse, silty fine to medium SAND, moist. Till like.		M	12.4	
	260		S-28	50	83	ML	Very dense, dark gray, fine to coarse gray, silty SAND to sandy SILT, moist. TILL-LIKE.		M SA	12.6	
40	265		S-29	50	83	SM	Very dense, dark gray, fine to coarse gravelly, silty fine to coarse SAND, moist to wet. Between 265 and 269, cuttings are wet.		M	10.5	
	270		S-30	33 - 50/5* (100+)	100	GP-GM	Very dense, dark gray, silty, sandy GRAVEL, wet. Abundant water. (GLACIAL FLUVIAL, Qpgf)		M	13.5	
30	275		S-31	50	83				M SA	3.9	
	280		S-32	50/3* (100+)	0						

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-6.5

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 6 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
-20	285	■	S-33	50	83	SP-SM	Very dense, dark gray, slightly silty to silty, fine to coarse gravelly, fine to coarse SAND, wet. (Drift-like)		M SA	13.2	
	290		S-34				12 feet of heave present at 289 feet. No sample taken.				
-10	295	■	S-35	83	117	SP-SM	Very dense, dark gray, slightly silty, fine to coarse gravelly, fine to medium SAND, wet.		M M	20.2 8.8	
	300	■	S-36	72	83	GP	4 feet of heave present. Driller pulled back casing and heave fell out, therefore, a sample was taken. Very dense, dark gray, fine to coarse sandy, silty, fine to coarse GRAVEL, wet.		M SA	9.1	
0	305	■	S-37	66	83	SW	Very dense, dark gray, slightly silty, fine to medium gravelly, fine to coarse SAND, wet. Ground water at 169.9 feet below ground surface at start of drilling the next day.		M	7.9	
	310	■	S-38	72	100	SW	Very dense, dark gray, slightly silty, fine to coarse gravelly, fine to coarse SAND, wet.		M	7.8	
-10	315	■	S-39	68	83		Very dense, dark gray, silty, fine to coarse sandy, fine to coarse GRAVEL, wet.		M SA	10.3	
	320	■	S-40	50	0		No recovery. Cuttings are silty, sandy gravel and cobbles.				
-20	325		S-41	75		GP	Very dense, dark gray, slightly silty to clean, fine to coarse sandy, fine to coarse GRAVEL with cobbles, wet.		M	7.5	
	330		S-42				22 feet of heave present at 329 feet. No sample taken.				
-30	335		S-43				10 feet of heave present. Cuttings are sandy GRAVEL, wet.				
	340										

(Ver.1.1 Jan02RWSP-FWSP.GLB-RWSP.GDT) H:\GINT\WPROJECTS\9815349K.GPJ 5/1/02

Figure: A-6.6

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 5

Sheet 7 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
-40	345	S-44		83	[Symbol]	ML	Hard, dark gray, SILT, moist. Laminated and plastic. (GLACIOLACUSTRINE, Qpgl)	[Schematic]	M AL	26.3	
	350	S-45			[Symbol]	GM	Cuttings become gravelly and wet. Lenses of silt and gravel.				
					[Symbol]	ML	Silt present at 350'. Very hard to pull casing out.				
-50	355						Bottom of boring at 352'. 2" piezometer installed to 352'. Vibrating wire piezometer installed to 240'.				
-60	365										
-70	375										
-80	385										
-90	395										
	400										

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-6.7

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (pcf)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-5	S-1	9.0 - 10.5	8.2													ML	I. olive-brown, poorly graded sandy SILT
MW-5	S-2	19.0 - 20.5	4.8													SP	I. olive-gray, poorly graded SAND
MW-5	S-3	29.0 - 30.5	17.7									2.9	93.9	3.2	1.6	SP	olive-brown, poorly graded SAND
MW-5	S-4a	39.0 - 40.0	21.4													SP-SM	gray, poorly graded SAND with silt and gravel
MW-5	S-4b	40.0 - 40.5	22.7													SM	olive-gray, silty SAND
MW-5	S-5	49.0 - 50.5	21.1										97.6	2.4	1.2	SP	olive-gray, poorly graded SAND
MW-5	S-6	59.0 - 59.0															
MW-5	S-7	69.0 - 70.5	28.4													ML	gray, SILT with sand
MW-5	S-8	79.0 - 80.5	25.3													CL	gray, lean CLAY
MW-5	S-9	89.0 - 90.5	26.1			699				40	25		0.3	99.7	3.8	CL	gray, lean CLAY
MW-5	S-10	99.0 - 100.5	24.4													CL	gray, lean CLAY
MW-5	S-11	109.0 - 110.5	12.0									20.9	37.0	42.1		SM	gray, silty SAND with gravel
MW-5	S-12	119.0 - 120.5	13.3									25.6	59.3	15.1	1.6	SM	gray, silty SAND with gravel
MW-5	S-13	129.0 - 129.0															
MW-5	S-14	139.0 - 140.5	15.2									4.9	43.7	51.3		ML	gray, sandy SILT
MW-5	S-15	149.0 - 150.5	9.7													SM	gray, silty SAND with gravel
MW-5	S-16	159.0 - 160.5	22.4										53.7	46.3	1.8	SM	gray, silty SAND
MW-5	S-17	169.0 - 170.3	16.5													ML	gray, SILT with gravel
MW-5	S-18	179.0 - 180.5	26.8							40	26		0.0	100.0	4.0	ML	gray, SILT
MW-5	S-19	189.0 - 190.0	23.8													ML	gray, SILT

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW- 5	S-20	199.0 - 200.5	19.2			591				51	23	0.1	2.2	97.7	5.6	CH	gray, fat CLAY
MW- 5	S-21	209.0 - 209.4	8.2													SM	gray, silty SAND with gravel
MW- 5	S-22	219.0 - 219.9	10.2													SM	gray, silty SAND with gravel
MW- 5	S-23	229.0 - 229.0															
MW- 5	S-24	237.5 - 239.0	20.9									0.2	83.8	16.1	1.0	SM	gray, silty SAND
MW- 5	S-25	245.0 - 245.4	9.1													SM	gray, silty SAND with gravel
MW- 5	S-26a	249.0 - 249.5	18.6													SM	gray, silty SAND
MW- 5	S-26b	249.5 - 250.5	18.6													CL	gray, lean CLAY
MW- 5	S-27	254.0 - 254.4	12.4													SM	gray, silty SAND with gravel
MW- 5	S-28	259.0 - 259.5	12.6									4.6	48.6	46.8		SM	gray, silty SAND
MW- 5	S-29	265.0 - 265.3	10.5													SM	gray, silty SAND with gravel
MW- 5	S-30	269.0 - 270.0	13.5													SM	gray, silty SAND with gravel
MW- 5	S-31	275.0 - 275.3	3.9									71.4	23.2	5.4		GP-GM	gray, poorly graded GRAVEL with silt and sand
MW- 5	S-32	277.0 - 301.0															
MW- 5	S-33	285.0 - 285.5	13.2			425						21.4	64.2	14.5	1.2	SM	gray, silty SAND with gravel
MW- 5	S-34	289.0 - 289.0															
MW- 5	S-35a	295.0 - 296.0	20.2													SP	dark gray, poorly graded SAND with gravel
MW- 5	S-35b	296.0 - 296.3	8.8													SM	gray, silty SAND with gravel
MW- 5	S-36	299.0 - 299.9	9.1									35.6	56.3	8.0		SP-SM	gray, poorly graded SAND with silt and gravel
MW- 5	S-37	305.0 - 305.9	7.9													SW	gray, well graded SAND with gravel

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWA GEOSCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 2 of 3

PROJECT NO.: 99153-490

FIGURE: A-6.15

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONFD COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW- 5	S-38	309.0 - 310.0	7.8													SW	gray, well graded SAND with gravel
MW- 5	S-39	315.0 - 315.9	10.3									44.9	48.2	6.9		SP-SM	gray, poorly graded SAND with silt and gravel
MW- 5	S-40	319.0 - 320.0															
MW- 5	S-41	325.0 - 325.0	7.5													SW-SM	gray, well graded SAND with silt and gravel
MW- 5	S-42	329.0 - 329.0															
MW- 5	S-43	335.0 - 335.0															
MW- 5	S-44	345.0 - 346.4	26.3			622				35	26				4.6	ML	dark gray, SILT
MW- 5	S-45	349.0 - 349.0															

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 3 of 3

PROJECT NO.: 99153-490

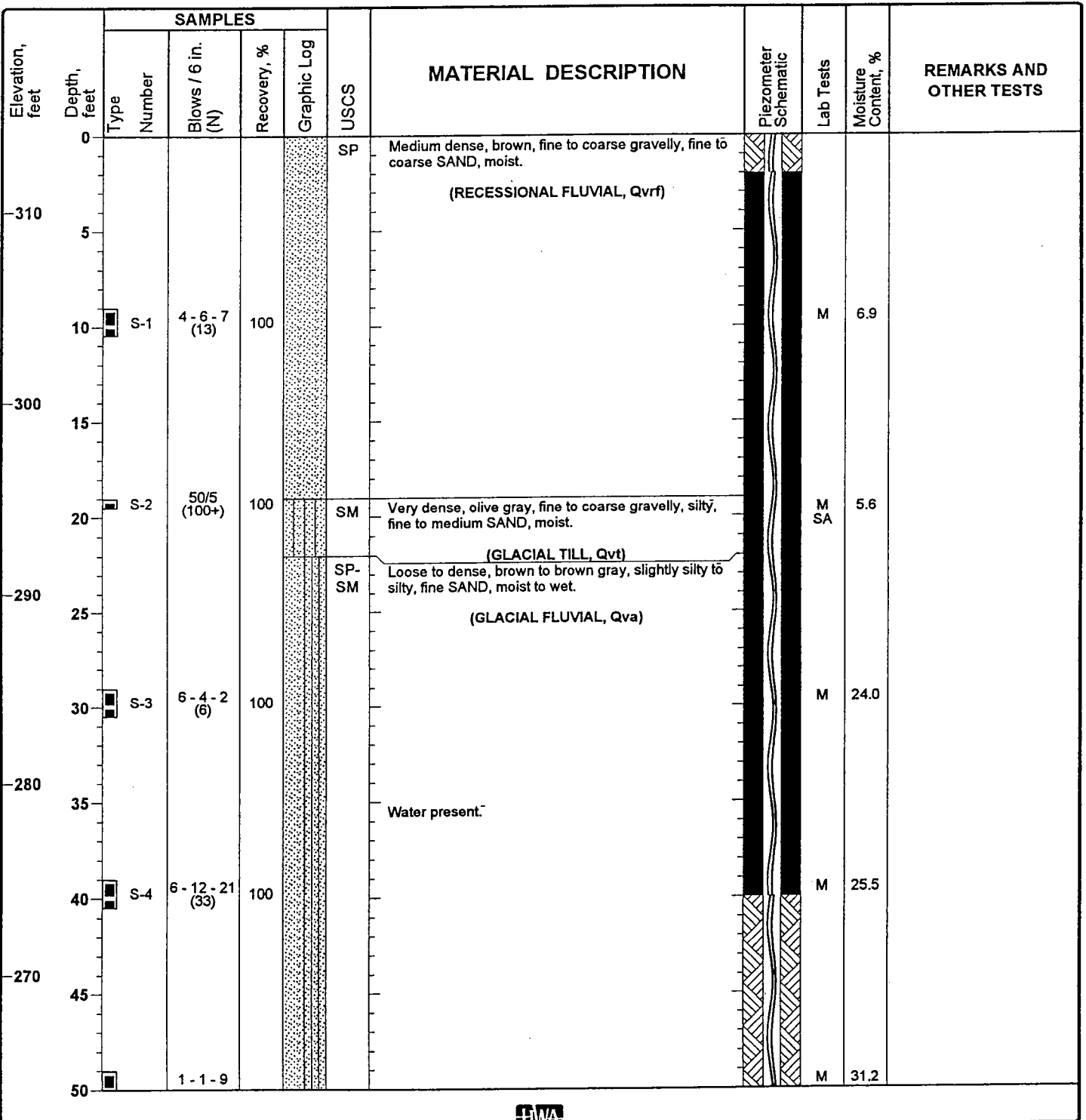
FIGURE: A-6.16

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 6

Sheet 1 of 7

Date(s) Drilled	1/11/02 - 1/23/02	Geotechnical Consultant	HWA GeoSciences Inc.	Logged By	BKH, MBB	Checked By	MLR/SEG
Drilling Method/ Rig Type	Becker Hammer Drill/ Truck	Drilling Contractor	Layne Christensen Company	Total Depth of Borehole	360.5 feet		
Drill Bit Size/Type	Dual Wall Reverse Circ.	Hammer Weight/Drop (lbs/in.)	300#, 30"	Ground Surface Elevation/Datum	314 feet / NAVD88		
Location	North east of 1-5/SR 1-4 I/C	Coordinates	N. 47.77785 W. 122.31449	Elevation Source	Plan		



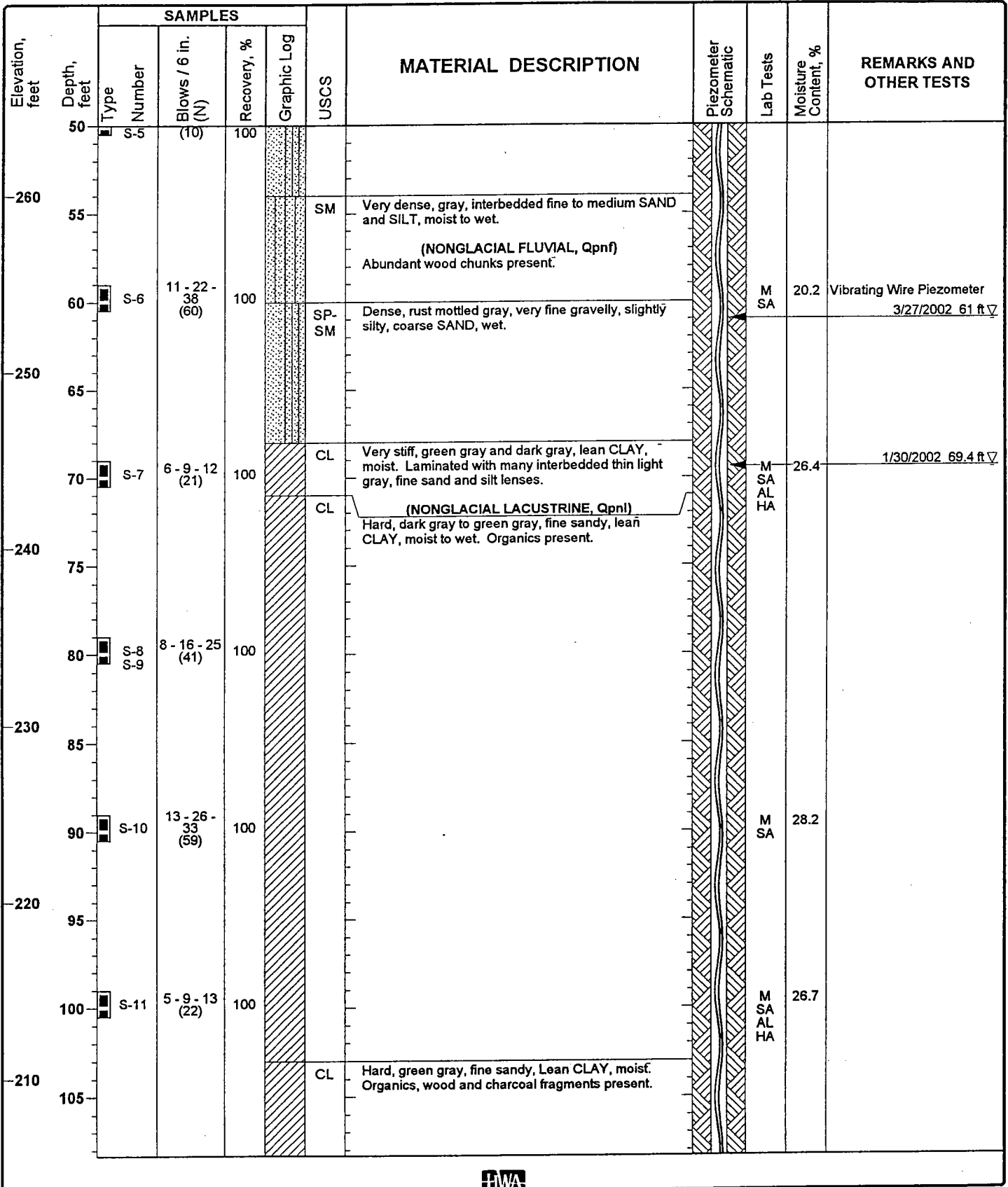
(Ver. 1.1 Jan 02 RWSP-GLB-RWSP.GDT) H:\GINT\PROJECTS\9515349K.GPJ 5/1/02

Figure: A-7.1

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 6

Sheet 2 of 7



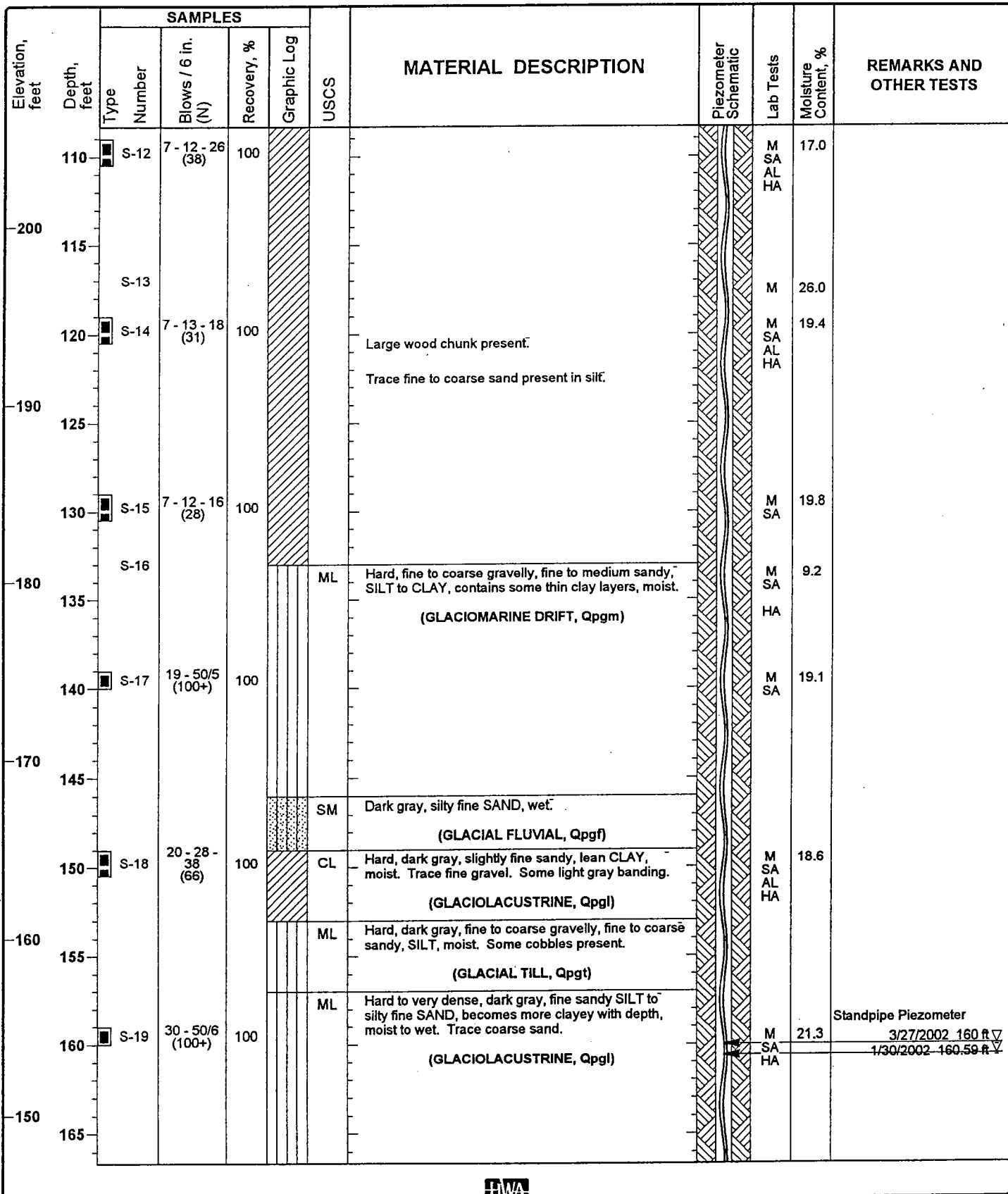
(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-7.2

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-6

Sheet 3 of 7



(Ver. 1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9515349K.GPJ 5/1/02

Figure: A-7.3

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 6
 Sheet 4 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
170	170	S-20		29 - 50/6 (100+)	100			M SA	14.3		
180	180	S-21		38 - 50/3 (100+)	83	CL	Hard, dark gray, lean CLAY, moist.	M SA HA	19.7		
190	190	S-22		7 - 16 - 18 (34)	100			M SA AL HA	20.4		
200	200	S-23a S-23b S-23c		5 - 9 - 13 (22)	100 100 100			M SA AL DD M	26.1 30.3		
210	210	S-24		6 - 8 - 12 (20)	100	CL	Very stiff, dark gray, slightly fine to coarse gravelly, fine to coarse sandy, lean CLAY, moist. (GLACIAL TILL, Qpgt)	M	20.0		
220	220	S-25		8 - 50/5 (100+)	100	SC	Very dense, gray, clayey SAND with gravel and cobbles, moist.	M SA HA	9.9		

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINTW\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-7.4

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 6

Sheet 5 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
225						CL	Hard, dark gray, fine to coarse gravelly, fine to coarse sandy, lean CLAY, moist. Cobbles present. (GLACIOMARINE DRIFT, Qpgm)				
230		S-26		11 - 23 - 39 (62)	100						
235		S-27a S-27b		12 - 25 - 48 (73)	100 100				M SA DD HA M AL	20.7	
240		S-28		10 - 40 - 42 (82)	100				M SA DD HA M AL	16.4	
245		S-29a S-29b S-29c		13 - 18 - 27 (45)	100 100 100				M SA AL HA M SA M SA HA	18.1 17.1	
250		S-30a S-30b		9 - 15 - 22 (37)	100 100	CH	Very stiff, dark gray, fat CLAY, moist. Light gray SILT partings.		M SA AL HA M SA AL M SA HA	19.8 16.4	
255		S-31c S-31a S-31b		12 - 18 - 23 (41)	100 100 100	GP	Gravel present in cuttings.		M SA HA M SA AL DD M SA AL M SA	19.9 18.7	
260		S-32a S-32b		10 - 19 - 25 (44)	100 100				M SA HA M SA AL DD M SA AL M SA	16.3 21.6	
265		S-33a S-33b S-33c		9 - 18 - 21 (39)	100 100 100				M AL M SA M M AL	20.7 21.6	
270		S-34		8 - 12 - 16 (28)	100				M SA M M AL	19.8	
275		S-35a S-35b S-35c		10 - 14 - 14 (28)	100 100 100				M SA HA	24.6	
280		S-36a S-36b S-36c		9 - 12 - 17 (29)	100 100 100	CH	Very stiff, dark gray, fat CLAY, moist. (GLACIOLACUSTRINE, Qpgl)		M SA DD HA M	21.0 21.8	

(Ver.1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9515349K.GPJ 5/1/02

Figure: A-7.5

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW- 6

Sheet 6 of 7

(Ver. 1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT) H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
285		S-37a S-37c S-37b		7 - 12 - 13 (25)	100 100 100				DD AL M SA AL HA M SA M SA M SA AL DD HA M SA AL	26.0 30.4	
290		S-38a S-38b S-38c		7 - 10 - 13 (23)	100 100 100				M SA M SA M SA AL DD HA	25.0 29.1	
295		S-39		5 - 8 - 10 (18)	100				M SA AL	29.2	
300		S-40a S-40b S-40c		5 - 8 - 13 (21)	100 100 100				M SA AL DD HA	30.3	
305		S-41		7 - 9 - 11 (20)	100				M AL SA	27.2	
310		S-42a S-42b S-42c		7 - 10 - 13 (23)	100 100 100				M SA AL DD HA	26.5	
315		S-43		7 - 9 - 13 (22)	100				M SA	28.5	
320		S-44c S-44a S-44b		6 - 9 - 22 (31)	100 100 100				M SA AL HA M SA	17.9	
325		S-45				GP	Dense, dark gray, sandy, fine to coarse gravel, moist. Gravel is sub-rounded to sub-angular.		M SA AL HA M SA	2.0	
325		S-46				SP	(GLACIAL FLUVIAL, Qpgf) Very dense, dark gray, fine to medium SAND, wet. Trace fine sub-rounded gravel and coarse sand.		M SA	18.0	
330		S-47 S-48		3 - 2 - 5 (7)	100		(GLACIAL FLUVIAL, Qpgf)		M SA	30.2	
335		S-49							M SA	18.2	
340											

Figure: A-7.6

Project: CSI Brightwater
 Project Location: King and Snohomish Counties
 Contract Number: E83004E

Log of Boring MW-6

Sheet 7 of 7

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
340		S-50						M SA	30.5		
345		S-51 S-52		3 - 100/5 (100+)	100 100			M SA	21.4		
350		S-53 S-54 S-55				CL	Hard, dark gray to olive gray, lean CLAY to clayey SAND, moist. Upper 1 foot portion is very gravelly. (GLACIOLACUSTRINE, Qpgl)	M SA AL HA M SA	16.4 32.4		
355		S-56 S-57				CL-ML	Very stiff, dark gray, slightly fine sandy SILT, moist.	M SA HA	27.9		
360		S-58		4 - 9 - 19 (28)	100			M SA HA	27.2		
365							Bottom of boring at 360.5 feet. 2" piezometer installed from 340 to 360 feet bgs. Vibrating wire piezometer installed at 180 feet.				
370											
375											
380											
385											
390											
395											
400											

{ Ver. 1.1 Jan02RWSP-RWSP.GLB-RWSP.GDT } H:\GINT\PROJECTS\9915349K.GPJ 5/1/02

Figure: A-7.7

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONFD COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-6	S-1	9.0 - 10.5	6.9													SP	yellowish-brown, poorly graded SAND
MW-6	S-2	19.0 - 19.5	5.6									28.9	48.9	22.3		SM	olive-gray, silty SAND with gravel
MW-6	S-3	29.0 - 30.5	24.0													SP-SM	brown, poorly graded SAND with silt
MW-6	S-4	39.0 - 40.5	25.5													SP-SM	grayish-brown, poorly graded SAND with silt
MW-6	S-5	49.0 - 50.5	31.2													SP-SM	grayish-brown, poorly graded SAND with silt
MW-6	S-6	59.0 - 60.5	20.2									3.2	88.5	8.3		SP-SM	dk. olive-gray, poorly graded SAND with silt
MW-6	S-7	69.0 - 70.5	26.4			576				33	22		0.0	100.0	5.4	CL	olive-gray, lean CLAY
MW-6	S-8	79.0 - 80.5														CL	olive-gray, lean CLAY
MW-6	S-9	80.0 - 80.0														CL	olive-gray, lean CLAY
MW-6	S-10	89.0 - 90.5	28.2											98.5		ML	olive-gray, SILT
MW-6	S-11	99.0 - 100.5	26.7			352				28	22		10.7	89.3	8.2	CL	dk. greenish-gray, lean CLAY
MW-6	S-12	109.0 - 110.5	17.0							33	22		0.1	99.9	4.6	CL	light gray, lean CLAY
MW-6	S-13	117.0 - 117.0	26.0													CL	dark olive-gray, lean CLAY
MW-6	S-14	119.0 - 120.5	19.4			201				36	21		2.0	98.0	5.0	CL	olive-gray, lean CLAY
MW-6	S-15	129.0 - 130.5	19.8											90.4		CL	greenish-gray, lean CLAY
MW-6	S-16	133.0 - 133.0	9.2									11.8	23.1	65.1		ML	lt. yellowish-brown, sandy SILT
MW-6	S-17	139.0 - 140.0	19.1											83.9		CL	gray, lean CLAY
MW-6	S-18	149.0 - 150.5	18.6							42	22		4.2	95.8	3.5	CL	olive-gray, lean CLAY
MW-6	S-19	159.0 - 160.0	21.3										10.4	89.6		ML	olive-gray, SILT
MW-6	S-20	169.0 - 170.0	14.3											87.5		CL	gray, lean CLAY

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWA GEOSCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 1 of 5

PROJECT NO.: 99153-490

FIGURE: A-7.36

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-6	S-21	179.0 - 179.9	19.7									12.3	87.7		ML	gray, SILT	
MW-6	S-22	189.0 - 190.5	20.4							45	22	2.1	97.9		CL	gray, lean CLAY	
MW-6	S-23a	199.0 - 199.5															
MW-6	S-23b	199.5 - 200.0	26.1	122.8	97.4		8.7			45	23		95.8		CL	dark gray, lean CLAY	
MW-6	S-23c	200.0 - 200.5	30.3												CL	olive-gray, lean CLAY	
MW-6	S-24	209.0 - 210.5	20.0												CL	olive-gray, lean CLAY	
MW-6	S-25	219.0 - 220.0	9.9									19.4	36.1	44.5	SC	gray, clayey SAND with gravel	
MW-6	S-26	229.0 - 230.5															
MW-6	S-27a	235.0 - 236.0															
MW-6	S-27b	236.0 - 236.5	20.7	126.7	105.0		6.8					0.6	99.4		CL	mottled gray & dark gray, lean CLAY	
MW-6	S-28	239.0 - 240.5	16.4							44	19				CL	gray, lean CLAY	
MW-6	S-29a	245.0 - 245.5	18.1							47	20	0.9	4.9	94.2	CL	olive-gray, lean CLAY	
MW-6	S-29b	245.5 - 246.0															
MW-6	S-29c	246.0 - 246.5	17.1										93.2		CL	dark gray, lean CLAY	
MW-6	S-30a	249.0 - 250.0	19.8							48	21		95.2		CL	gray, lean CLAY	
MW-6	S-30b	250.0 - 250.5	16.4			848						2.3	9.1	88.6	4.0	CH	gray, fat CLAY
MW-6	S-31c	255.0 - 255.5															
MW-6	S-31a	255.5 - 256.0	19.9			915						2.3	6.0	91.8	3.2	CH	dark gray, fat CLAY
MW-6	S-31b	256.0 - 256.5	18.7	123.9	104.4					44	18		91.0		CL	dark gray, lean CLAY	
MW-6	S-32a	259.0 - 260.0	16.3							38	18		86.7		CL	dark brownish-gray, lean CLAY	

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (pcf)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-6	S-32b	260.0 - 260.5	21.6									1.7	8.0	90.3		CL	gray, lean CLAY
MW-6	S-33a	265.0 - 265.5	20.7							49	24					CL	gray, lean CLAY
MW-6	S-33b	265.5 - 266.0	21.6											97.7		CL	gray, lean CLAY
MW-6	S-33c	266.0 - 266.5				983											
MW-6	S-34	269.0 - 271.0	19.8							49	23					CL	dark gray, lean CLAY
MW-6	S-35a	275.0 - 275.5	24.6										0.0	100.0		CL	gray, lean CLAY
MW-6	S-35b	275.5 - 276.0															
MW-6	S-35c	276.0 - 276.5															
MW-6	S-36a	279.0 - 279.5	21.0	128.7	106.4		9.1						0.0	100.0		CH	gray, lean CLAY
MW-6	S-36b	279.5 - 280.0	21.8	125.5	103.0		5.7			31	20					CL	dark gray, lean CLAY
MW-6	S-36c	280.0 - 280.5															
MW-6	S-37a	285.0 - 285.5	26.0			1180				79	25		0.0	100.0	5.4	CH	gray, fat CLAY
MW-6	S-37c	285.5 - 286.0															
MW-6	S-37b	286.0 - 286.5	30.4											99.9		CH	dark olive-gray, fat CLAY
MW-6	S-38a	289.0 - 289.5	25.0											100.0		CL	dark gray, lean CLAY
MW-6	S-38b	289.5 - 290.0	29.1	84.3	65.3					74	26		0.0	100.0		CH	dark gray, fat CLAY
MW-6	S-38c	290.0 - 290.5															
MW-6	S-39	295.0 - 297.0	29.2							84	27			97.4		CH	dark gray, fat CLAY
MW-6	S-40a	299.0 - 299.5														CH	dark gray, fat CLAY
MW-6	S-40b	299.5 - 300.0	30.3	121.8	93.5	1070	13.2			70	27		0.0	100.0	4.6	CH	dark gray, fat CLAY

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONFD COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-6	S-40c	300.0 - 300.5														CH	dark gray, fat CLAY
MW-6	S-41	305.0 - 306.5	27.2							68	24			100.0	6	CH	dark gray, fat CLAY
MW-6	S-42a	309.0 - 309.5	26.5	124.1	98.1		12.1			53	25		0.1	99.9		CH	dark gray, fat CLAY
MW-6	S-42b	309.5 - 310.0														CH	dark gray, fat CLAY
MW-6	S-42c	310.0 - 310.5														CH	dark gray, fat CLAY
MW-6	S-43	315.0 - 316.5	28.5											100.0		CH	olive-gray, fat CLAY
MW-6	S-44c	319.0 - 319.5															
MW-6	S-44a	319.5 - 320.0	17.9							41	20		4.3	95.7		CL	gray, lean CLAY
MW-6	S-44b	320.0 - 320.5														CH	dark gray, fat CLAY
MW-6	S-45	322.0 - 323.0	2.0									65.9	27.5	6.5		GW-GM	gray, well graded GRAVEL with silt and sand
MW-6	S-46	325.0 - 326.0	18.0									5.3	92.2	2.5		SP	dark gray, poorly graded SAND
MW-6	S-47	329.0 - 330.5															
MW-6	S-48	331.0 - 332.0	30.2											1.2		SP	dark gray, poorly graded SAND
MW-6	S-49	335.0 - 336.0	18.2									14.6	82.1	3.3		SP	dark gray, poorly graded SAND
MW-6	S-50	342.0 - 343.0	30.5									0.5	96.7	2.8		SP	dark gray, poorly graded SAND
MW-6	S-51	345.0 - 346.0															
MW-6	S-52	346.0 - 347.0	21.4									0.1	98.8	1.1		SP	dark gray, poorly graded SAND
MW-6	S-53	349.0 - 350.0	16.4							37	17	42.8	14.1	43.1		GC	dark olive-gray, clayey GRAVEL
MW-6	S-54	350.0 - 351.0	32.4											61.1		CL	olive-gray, gravelly lean CLAY
MW-6	S-55	351.0 - 352.0														SC	olive-gray, clayey SAND with gravel

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.

EXPLORATION DESIGNATION	SAMPLE NUMBER	DEPTH (ft)	WATER CONTENT (%)	WET DENSITY (PCF)	DRY DENSITY (pcf)	SODIUM CONTENT (mg/kg dry)	UNCONF'D COMPR. STRENGTH (ksf)	COHESION (psi)	PHI ANGLE (degrees)	LIQUID LIMIT	PLASTIC LIMIT	% GRAVEL	% SAND	% FINES	ORGANIC CONTENT (%)	ASTM SOIL CLASSIFICATION	SAMPLE CLASSIFICATION
MW-6	S-56	353.0 - 355.0														SC	gray, clayey SAND with gravel
MW-6	S-57	355.0 - 356.0	27.9										3.3	96.7		ML	dark olive-gray, SILT
MW-6	S-58	359.0 - 360.5	27.2										0.4	99.6		ML	dark olive-gray, SILT

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report text, other graphs and tables, and the exploration logs.



HWA GEOSCIENCES INC.

Brightwater Project
King and Snohomish Counties
Washington

SUMMARY OF
MATERIAL PROPERTIES

PAGE: 5 of 5

PROJECT NO.: 99153-490

FIGURE: A-7.40

Project: Brightwater Project	Log of Boring BW-4 Sheet 1 of 4
Project Location: N. King County/S. Snohomish County	
Contract Number:	

Date(s) Drilled: 12/28/01 - 1/11/02	Geotechnical Consultant: SHANNON & WILSON	Logged By: XDH/BMP	Checked By: TWH
Drilling Method/ Rig Type: mud rotary/ Mobile B-59	Drilling Contractor: Geo-Tech Explorations, Inc.	Total Depth of Borehole: 366.4 feet	
Drill Bit Size/Type: 6-inch Tricone	Hammer Weight/Drop (lbs/in.): 300#/30"	Ground Surface Elevation/Datum: 368 feet / NAVD88	
Location: 228th St/Hwy 99	Coordinates: N 292703.0 E 1270572.0	Elevation Source: Topo	

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0						SP-SM	Very dense, gray-brown, slightly silty, fine to medium SAND, trace of gravel; moist to wet; scattered gravelly layers; SP-SM. (Qva)				
360	10	■	1	50/5"	0						
	20	■	2	99/10.5"	89				M	5.7	
340	30	□	3	58/6"	100				M SA	11.9	
	40	■	4	50/5"	74				M	11.8	
320	50	□	5	57/6"	98				M SA	9.6	
	60	□	6	87/6"	100				M	18.1	
300	70	□	7	65/6"	0						
	80	□	8	54/6"	100				M	15.1	
280	90	□	9	85/6"	0						VWP1 2/26/2002 87.37 ft ▼
100											

[Ver. 1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ] [APP\GINT\PROJECTS\21\21-08933.GPJ 5/1/02]

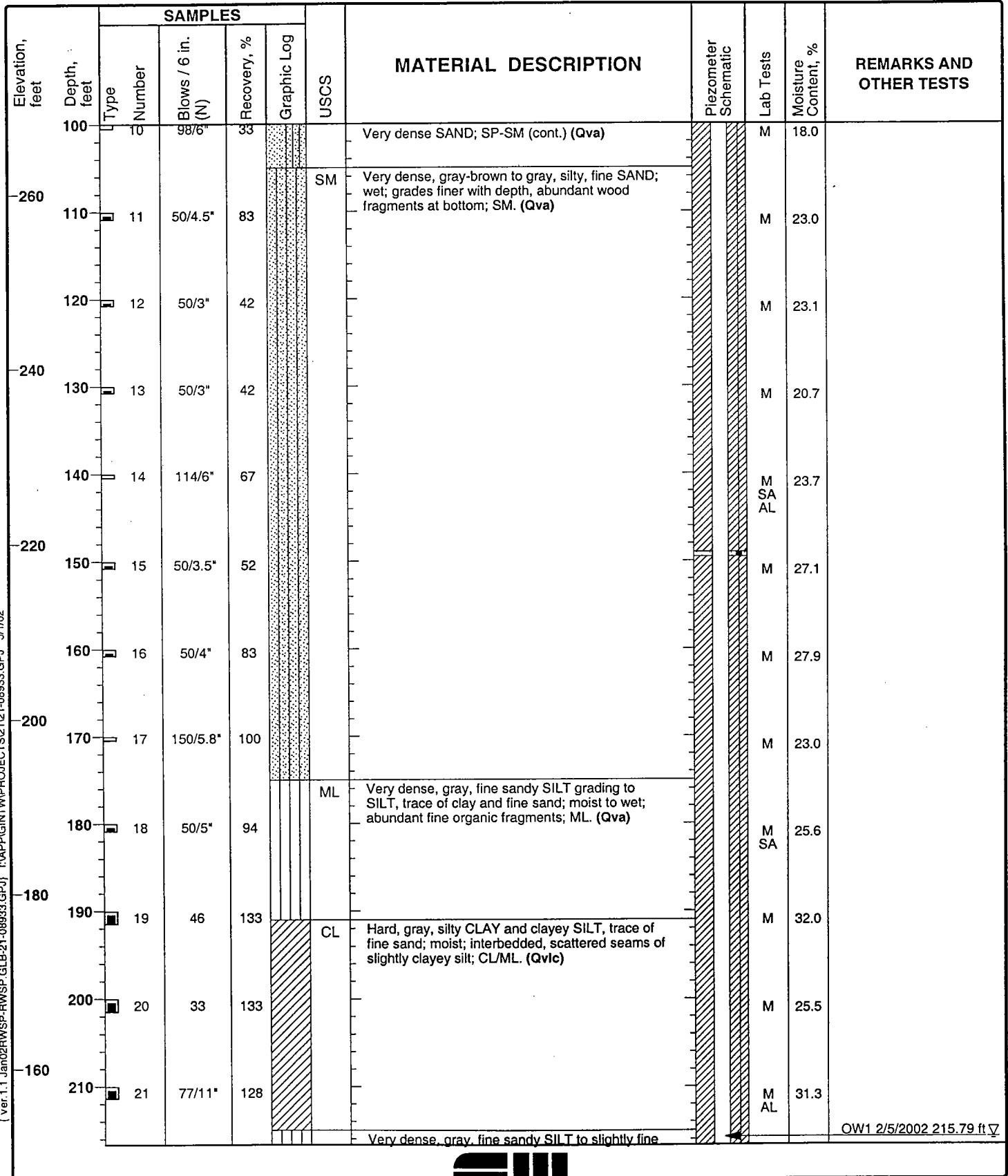


Figure: B-3.1

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-4

Sheet 2 of 4



I:\APP\GINT\PROJECTS\2121-08933.GPJ 5/1/02

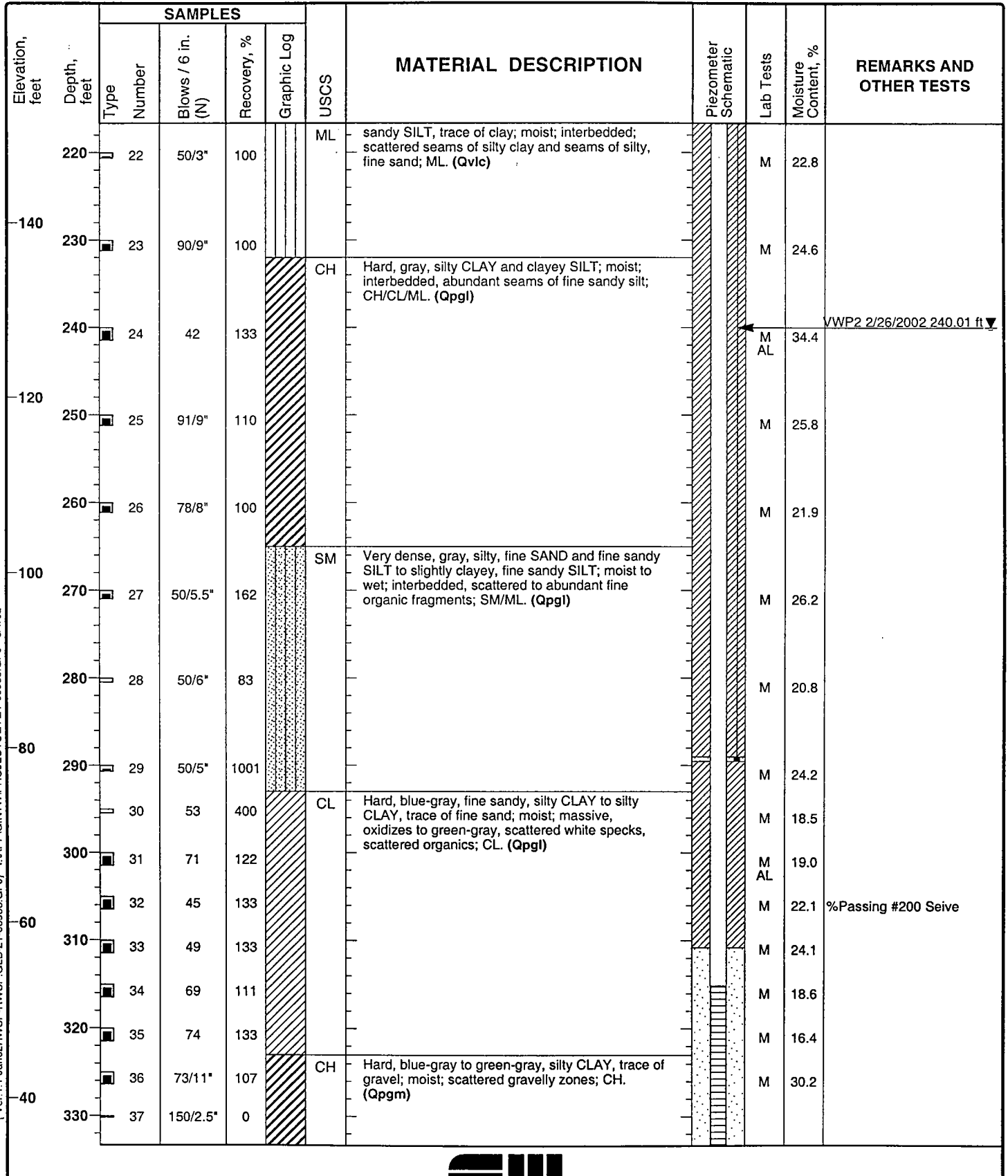


Figure: B-3.2

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-4

Sheet 3 of 4



[Ver.1.1 Jan02RWSP-RWSP.GLB-21-06933.GPJ | \APPGINTWPROJECTS\21121-06933.GPJ 5/1/02



Figure: B-3.3

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-4

Sheet 4 of 4

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS	
		Type	Number	Blows / 6 in. (N)	Recovery, %							Graphic Log
			38	200/2.5"	0		Hard, silty CLAY; CH (cont.) (Qp _{gm})					
	340		39	93/11"	115	CL	Hard, gray to blue-gray, slightly fine sandy, silty CLAY to fine sandy, silty CLAY; moist; abundant sandy seams, trace of gravel locally; CL. (Qp _{gl})		M	21.7		
			40	50/5"	200					M	19.5	
-20	350		41	69/6"	300					SA		
			42			CL	Hard, gray, silty CLAY, trace of sand; moist; massive, trace of gravel locally, scattered streaks of green-gray mottling; CL. (Qp _{gl})		M	19.3		
			43	90	100					M	23.4	
	360		44	87/11"	107					M	21.9	
				87/11"	107				M	25.1		
0	370	BOTTOM OF BORING COMPLETED 1/11/2002										
	380											
-20	390											
	400											
-40	410											
	420											
-60	430											
	440											
-80	450											

I:\ver.1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ I:\APP\GINTWPROJECTS\2121-08933.GPJ 5/1/02



Figure: B-3.4

**FIGURE B-3.7
LABORATORY TESTING SUMMARY FOR BW-4**

SAMPLE DATA				% Gravel	GRAIN-SIZE ANALYSES			ATTERBERG LIMITS ^{d,e}				Interpreted Geologic Unit	Soil Description ^f
Boring No.	Sample No.	Top Depth (feet)	Natural Water Content ^a (%)		% Sand	% Fines ^b	% < 2 mm ^c	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	USCS Symbol		
BW-4	S-3	30.0	11.9	16.4	76.7	6.8					SP-SM	Qva	Gray-brown, slightly silty, fine gravelly, SAND
	S-5	50.0	9.6	1.7	92	6.2					SP-SM	Qva	Gray-brown, slightly silty, fine to medium SAND, trace of fine gravel
	S-14	140.0	23.7			13.7					SM	Qva	Gray, silty, fine SAND
	S-18	180.0	25.6			99.4	4.5				ML	Qva	Gray, slightly clayey, SILT, trace of sand
	S-21	210.0	31.3					42	23	19	CL	Qglc	Gray, silty CLAY
	S-24	240.0	34.4					67	31	36	CH	Qppl	Gray, silty CLAY
	S-31	300.0	19.0					36	23	13	CL	Qppl	Blue-gray, slightly sandy, silty CLAY
	S-32	305.0	22.1			93.7					CL	Qppl	Gray-brown, slightly sandy, silty CLAY
	S-40	345.0	19.5	4.9	30.5	64.6	13.4				CL	Qppl	Gray, sandy, silty CLAY, trace of fine gravel
	S-41	350.0	19.3					29	21	8	CL	Qppl	Blue-gray, fine sandy, silty CLAY

Notes:

- a Natural water content conducted on all samples and appears on boring logs.
- b Particle size smaller than 0.075 mm.
- c 1 mm = 1000 µm
- d The numbers shown have been rounded (LL, PL, and PI)
- e NP = non-plastic
- f Soil descriptions have been abbreviated and simplified. For complete descriptions, see the boring logs in Appendix A.1.

Figure B-3.7

Project: **Brightwater Project**
 Project Location: **N. King County/S. Snohomish County**
 Contract Number:

Log of Boring BW-5

Sheet 1 of 3

Date(s) Drilled	12/6/01 - 12/20/01	Geotechnical Consultant	SHANNON & WILSON		Logged By	BMP	Checked By	TWH
Drilling Method/ Rig Type	mud rotary/ Mobile B-59		Drilling Contractor	Geo-Tech Explorations, Inc.		Total Depth of Borehole	391.2 feet	
Drill Bit Size/Type	6-inch Tricone		Hammer Weight/Drop (lbs/in.)	300#/30"		Ground Surface Elevation/Datum	400 feet / NAVD88	
Location	End of 227th Street SW, SE corner		Coordinates	N 292560.0 E 1275712.0		Elevation Source	Topo	

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
400	0					SP-SM	Very dense, gray-brown, slightly silty, fine to medium SAND; moist to wet; massive, scattered gravelly layers inferred from drill action; SP-SM. (Qva)				
	10	■	1	60	83				M	13.0	
380	20	■	2	65	81				M	9.0	
	30	▬	3	50/6"	100		- gravelly sand between 3 and 41 feet		M SA	11.3	VWP1 2/26/2002 35.8 ft ▼
360	40	▬	4	50/5"	111		- light gray below 45 feet		M	13.6	VWP2 1/9/2002 43.79 ft ▼
	50	▬	5	50/4"	93				M	18.9	
340	60	▬	6	50/4.5"	79				M	21.4	OW1 2/26/2002 60.3 ft ▼
	70	■	7	44	122	CL	Hard, gray-brown to gray, silty CLAY to clayey SILT, trace of fine sand; moist to wet; massive, scattered seams of fine sandy silt, abundant organics; CL/ML. (Qpgl)		M M	23.3 23.7	
320	80	■	8	80	128	CH	Hard, green-gray to brown and gray, silty CLAY to clayey SILT; moist; massive to bedded, trace of sand locally, weathered at top; CH/MH/CL. (Qpgl)		M AL M	23.5 23.8	
	90	■	9	33	133				M AL	33.2	
300	100	■	10	37	133				M	34.2	
	110	■	11	52	113	ML	Hard, light brown, silty CLAY grading to clayey SILT; moist; massive to bedded, abundant seams of silty, fine sand to fine sandy SILT, scattered gravel inferred from drill action; ML/CL. (Qpgl)		M	36.8	
280	120	■	12	73/11.8"	122				M AL	34.2	
	130	▬	13	50/3"	119						

I:\Ver.1.1\Jan02RWSP-RWSP_GLB-21-08933.GPJ I:\APP\GINT\PROJECTS\21\21-08933.GPJ 5/1/02



Figure: B-4.1

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-5

Sheet 2 of 3

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
							Hard, silty CLAY to clayey SILT; ML/CL (cont.) (Qpgl)		M	30.2	
-260	140		14	82/9"	100				M	20.7	
	150		15	50/6"	67	CL	Hard, light gray-brown, sandy, gravelly, silty CLAY to very dense, silty, sandy GRAVEL, trace of clay; massive; scattered cobbles inferred from drill action; CL/GM. (Qpgm)		M	29.7	
-240	160		16	100/4"	97				M	17.8	
	170		17	100/5"	98	SM	Very dense, brown, silty, fine to medium SAND, trace of gravel; moist to wet; massive, scattered gravelly layers inferred from drill action; SM. (Qpgf)		M	13.3	
-220	180		18	60/6"	60				M	17.6	
	190		19	100/4"	33	SM	Very dense, brown to gray-brown, silty, gravelly SAND; moist to wet; scattered slightly clayey layers, scattered layers of silty, fine sand; SM. (Qpgf)		SA	20.9	
-200	200		20	200/4.5"	31				M	15.9	
	210		21	200/4"	101				M	8.8	
-180	220		22	125/6"	50				M	11.2	
	230		23	110/6"	25				SA	11.0	
-160	240		24	150/4"	78				M	14.4	
	250		25	150/6"	92				M	11.5	
-140	260		26	150/4"	33				M	17.4	
	270		27	150/4.5"	125				M	16.7	
-120	280		28	150/3"	100	SC	Very dense, gray, fine gravelly, silty, clayey SAND and hard, fine sandy, silty CLAY to clayey SILT;		SA	7.3	
									M	12.4	

I:\APP\GINT\PROJECTS\21-08933.GPJ 5/1/02



Figure: B-4.2

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-5

Sheet 3 of 3

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
290	29		150/5"	52		SC/CL. (Qp gm)		M AL	17.6		
100	300		150/3"	73				M	11.5		
310	31		150/5"	99				M	18.3		
80	320		50/6"	125		CH	Hard, gray, silty CLAY, trace of sand and gravel to sandy, gravelly, silty CLAY; moist; massive; faintly sheared texture at top; gravelly, silty, clayey sand locally; CH/CL/SC. (Qp gm)	M	19.4		
330	33		50/5.5"	133				M	16.5		
	34		50/6"	142				M	21.0		
60	340		50/6"	117				M AL	12.7		
	36		50/5"	137				M	13.3		
350	37		50/5"	148				M	16.5		
	38		150/3.5"	194							
40	360		65/6"	133				M	9.0		
	40		85/6"	133		CH	Hard, gray, silty CLAY; massive to bedded, trace of sand above 375 feet, scattered silt pockets and partings; CH/CL. (Qpgl)	SA M	15.5		
370	41		96/6"	133				M AL	15.7		
	42		65/6"	125				M	16.4		
20	380		50/4.5"	116				M	24.7		
	44		88/8"	152				M	29.4		
390	45		50/5"	130				M	30.3		
						BOTTOM OF BORING COMPLETED 12/20/2001					
0	400										
	410										
-20	420										
	430										

[Ver. 1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ] I:\APP\GINT\PROJECTS\21-08933.GPJ 5/1/02



Figure: B-4.3

**FIGURE B-4.6
LABORATORY TESTING SUMMARY FOR BW-5**

SAMPLE DATA				% Gravel	GRAIN-SIZE ANALYSES				ATTERBERG LIMITS ^{d,e}				Interpreted Geologic Unit	Soil Description ^f
Boring No.	Sample No.	Top Depth (feet)	Natural Water Content ^a (%)		% Sand	% Fines ^b	% < 2 mm ^c	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	USCS Symbol			
BW-5	S-3	30.0	11.3	0.1	89.6	10.2					SP-SM	Qva	Gray-brown, slightly silty, fine to medium SAND	
	S-7B	71.0	23.7					32	23	9	CL	Qpgl	Gray, silty CLAY, trace of fine sand	
	S-8	81.0	23.8					45	26	19	CL	Qpgl	Gray, silty CLAY	
	S-9	90.0	33.2					54	31	23	MH	Qpgl	Light brown, clayey SILT	
	S-12	120.0	34.2					39	28	11	ML	Qpgl	Gray, clayey SILT	
	S-18	180.0	20.9	2.2	83.6	14					SM	Qpgf	Gray-brown, silty, fine to medium SAND, trace of gravel	
	S-22	220.0	11.0	15.7	58.5	25.7					SM	Qpgf	Gray-brown, fine gravelly, silty SAND, trace of clay	
	S-28	280.0	12.4	13.7	41.8	44.4					SC	Qpgm	Gray, fine gravelly, silty, clayey SAND	
	S-29	290.0	17.6					34	22	12	CL	Qpgm	Gray, silty CLAY, trace of sand	
	S-34	335.0	21.0					63	26	37	CH	Qpgm	Gray, silty CLAY, trace of sand and gravel	
	S-35	340.0	12.7	5.5	31.1	63.3					CL	Qpgm	Gray, slightly gravelly, silty, sandy CLAY	
	S-39	360.0	9.0	16.6	36.8	46.5					SC	Qpgm	Gray, fine gravelly, silty, clayey SAND	
	S-41	370.0	15.7					43	21	22	CL	Qpgl	Gray, silty CLAY, trace of sand	

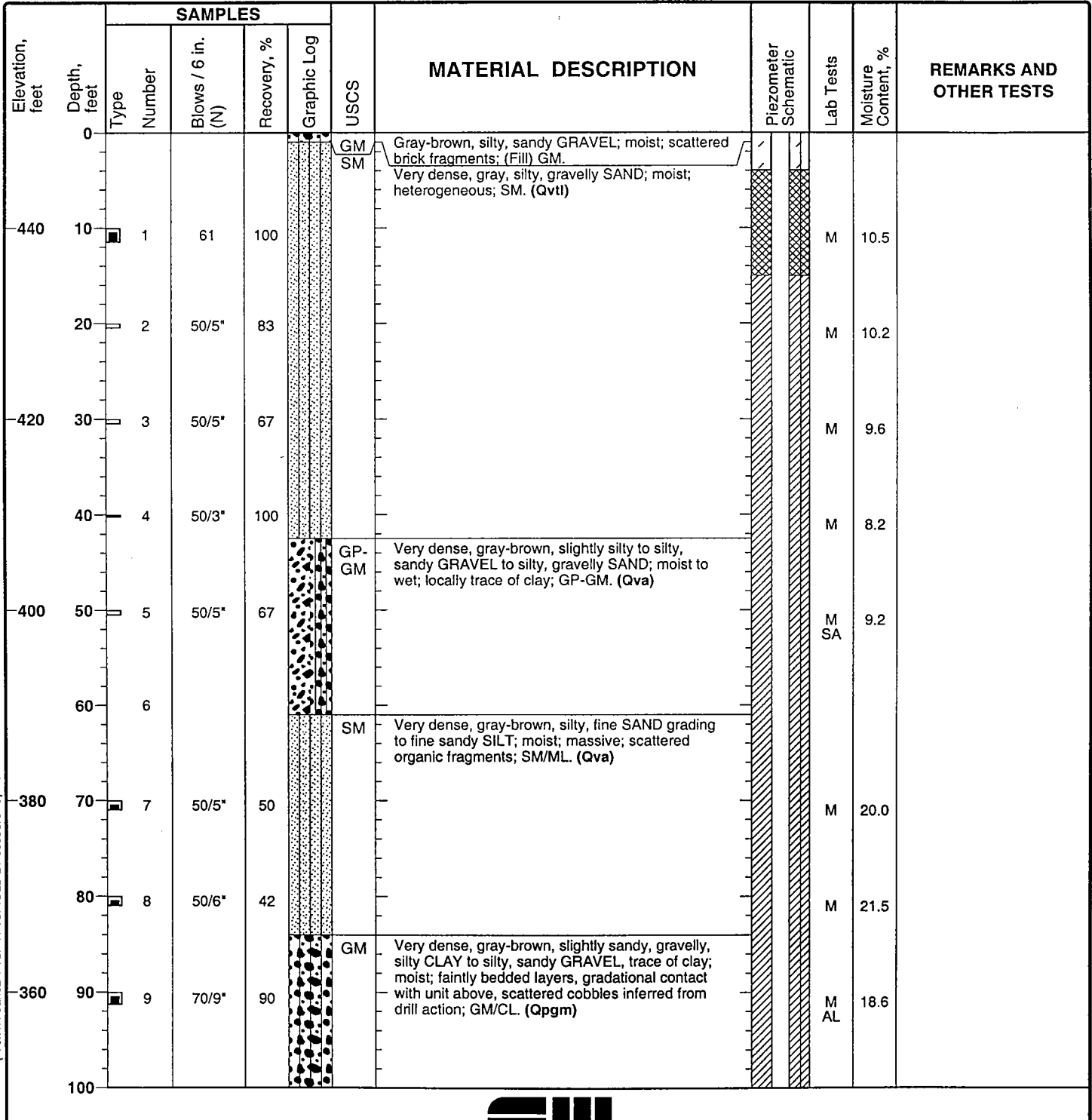
- Notes:
- a Natural water content conducted on all samples and appears on boring logs.
 - b Particle size smaller than 0.075 mm.
 - c 1 mm = 1000 μm
 - d The numbers shown have been rounded (LL, PL, and PI)
 - e NP = non-plastic
 - f Soil descriptions have been abbreviated and simplified. For complete descriptions, see the boring logs in Appendix A.1.

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-6

Sheet 1 of 4

Date(s) Drilled	12/13/01 - 12/28/01	Geotechnical Consultant	SHANNON & WILSON	Logged By	KGW	Checked By	TWH
Drilling Method/ Rig Type	mud rotary/ CME-85		Drilling Contractor	Gregory Drilling		Total Depth of Borehole	430.5 feet
Drill Bit Size/Type	6-inch Tricone		Hammer Weight/Drop (lbs/in.)	300#/30"		Ground Surface Elevation/Datum	450 feet / NAVD88
Location	228th and 48th Ave. W.		Coordinates	N 292329.0 E 1280286.0		Elevation Source	Topo



[Ver.1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ I:\APPGINT\PROJECTS\21\21-08933.GPJ 5/1/02

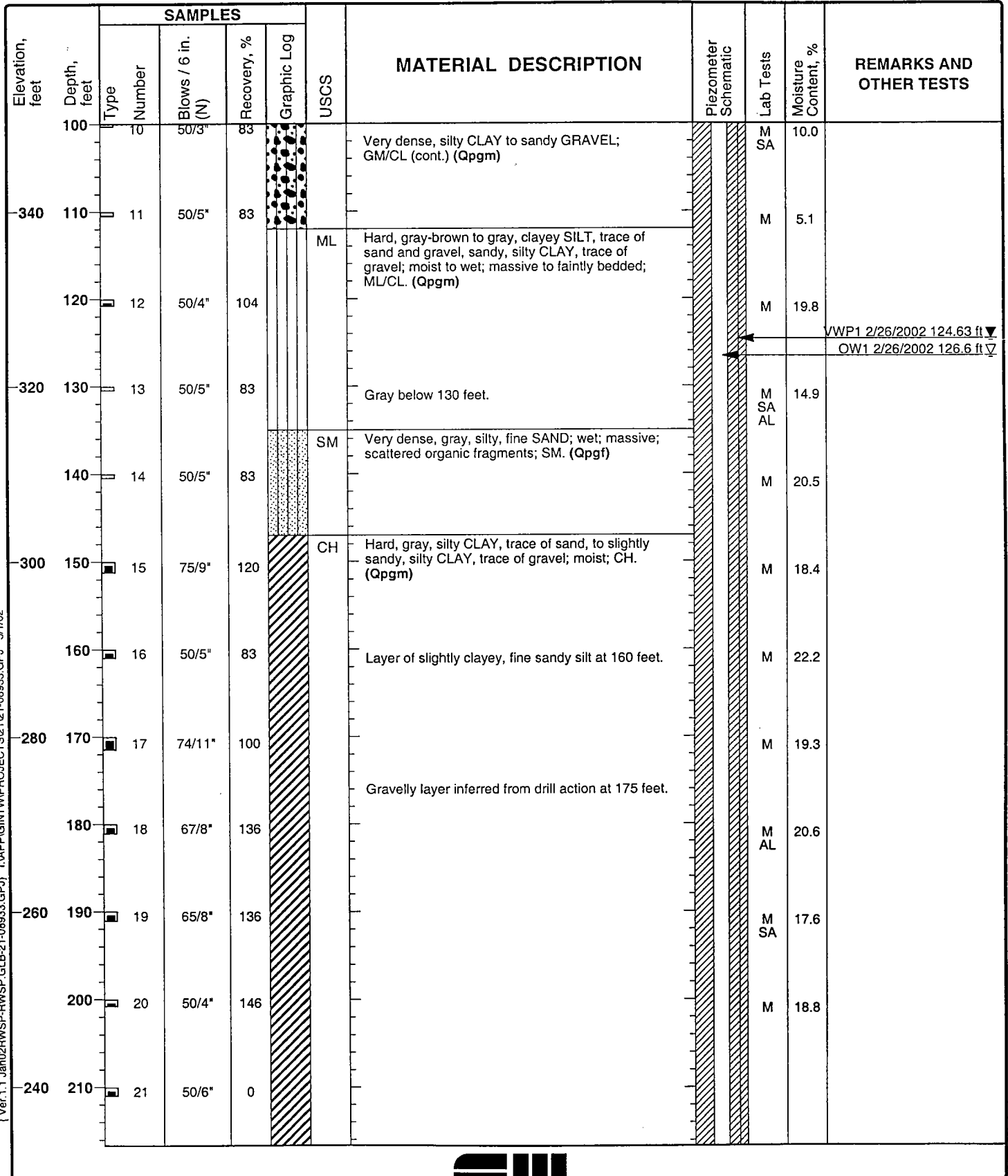


Figure: B-5.1

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-6

Sheet 2 of 4



[Ver. 1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ I:\APPGINTW\PROJECTS\21\21-08933.GPJ 5/1/02



Figure: B-5.2

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-6

Sheet 3 of 4

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
220	22		70/3"	100		SM	Very dense, gray, clayey, silty, gravelly SAND; moist; SM. (Qpgm)		M	9.7	
220	230		67/4"	101		SP-SM	Very dense, gray, slightly silty, fine to medium SAND, trace of gravel, to fine to medium SAND, trace of silt; wet; scattered seams and layers of silty, fine sand to fine sandy, clayey silt; SP-SM/SP. (Qpgf)		M	16.0	
240	24		65/4"	69					M	17.9	
200	250	■	70	100					M	18.5	
260	26	■	50/5"	100					M	19.2	
							Gravelly layer inferred from drill action at 265 feet.				
180	270	■	50/5"	83					M	17.2	
						GW-GM	Very dense, gray, slightly silty, sandy GRAVEL; wet; scattered to abundant cobbles inferred from drill action; GW-GM. (Qpgf)				
280	28		116/3"	83					M	10.4	
							Gravelly sand inferred from drill action below 282 feet.				
160	290	■	50/4"	100		SM	Very dense, gray SILT, trace of fine sand and clay, to silty, fine SAND; moist; scattered to abundant organic and wood fragments, scattered slightly silty layers; SM/ML. (Qpnl)		M	28.6	
300	30		69/5"	83					MSA	21.7	
140	310	■	71/11"	95		CL	Hard, gray, clayey SILT to silty CLAY, trace of sand; moist; bedded, abundant organic to peaty seams and wood fragments; CL/ML. (Qpnl)		MAL	36.0	
320	32	■	41	100					M	35.6	
120	330	■	57/3"	100		SW-	Very dense, gray, slightly silty SAND, trace of gravel and slightly silty, fine SAND; wet; scattered		M	27.8 24.5	

[Ver. 1.1 Jan02RWSP-RWSP.GLB-21-08933.GPJ I:\APPGINT\PROJECTS\21\21-08933.GPJ 5/11/02



Figure: B-5.3

Project: Brightwater Project
 Project Location: N. King County/S. Snohomish County
 Contract Number:

Log of Boring BW-6

Sheet 4 of 4

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Moisture Content, %	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
						SM	organic fragments; SW-SM/SP. (Qpnf)				
	340		34	60/5.5"	67				M SA	16.9	
-100	350		35	40/6"	100				M	22.6	
	360		36	58/4"	101				M	11.0	
-80	370		37	50/5.5"	83				M SA	15.8	
			38	63/5"	21				M	10.4	
	380		39	65/8"	97	ML	Hard, dark gray, clayey SILT to silty CLAY, trace of fine sand; moist; massive to laminated; scattered peaty partings; scattered to abundant organic fragments; ML/CL. (Qpntf)		M	31.7	
			40	50/3"	95				M AL	23.3	
-60	390		41	50/3"	100				M	26.8	
			42	65/10"	100				M	23.0	
	400		43	86	94				M	24.3	
			44	50/4"	83	ML	Very dense, dark gray, slightly clayey, fine sandy SILT to fine sandy, clayey SILT; moist; interbedded, abundant organics and shell fragments; ML/CL-ML. (Qpntf)		M	20.0	
-40	410		45	74	89				M AL	23.2	
			46	50/5.5"	92				M	22.3	
	420		47	85/5.5	83	SP	Very dense, gray, fine to medium SAND, trace of silt, to slightly silty, fine to medium SAND; wet; SP/SP-SM. (Qpnf)		M	17.3	
			48	100/6"	100				M SA	18.9	
-20	430		49	92/6"					M	15.6	
							BOTTOM OF BORING COMPLETED 12/28/2001				
	440										
0	450										

I:\APP\GINT\PROJECTS\21\21-08933.GPJ 5/1/02



Figure: B-5.4

Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-105

Sheet 1 of 16

Date(s) Drilled	3/3/03 - 4/3/03	Geotechnical Consultant	Camp Dresser & McKee Inc.	Logged By	TCB	Checked By	VJP 02-03-04
Drilling Method/Rig Type	Roto-Sonic/	Drilling Contractor	Cascade/Boart-Longyear	Total Depth of Borehole	535.0 feet		
Casing Size/Type	8"/6"/4"/Telescoping Casing	Hammer Weight/Drop (lbs/in.)	N/A	Ground Surface Elevation/Datum	549.0 feet / Metro		
Location	5th Ave SW, Shoreline	Coordinates	N 287349 E 1264077	Elevation Source	Survey		

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											0 to 6 feet excavated with vacuum truck, not sampled
545	5										Starting with 9-inch casing
540	10		1	93		SM	Medium dense, red brown, wet, slightly gravelly SILTY SAND (SM), fine to coarse sand, fine to coarse gravel, subround to round, numerous organics, wood debris, homogeneous (af)				
535	15					SW	Dense, brown-olive-gray, moist, gravelly SAND (SW), well-graded, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
530	20		2	97			Grades brown, slightly gravelly, trace silt				
525	25					SP	Dense, brown-light gray, moist, gravelly SAND (SP), poorly-graded, trace silt, trace fine sand, medium to coarse sand, fine gravel, subround to round, homogeneous (Qva)				

Groundwater Observation Data:
 OW (FT BGS):
 VWP1 (FT BGS):
 VWP2 (FT BGS):

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
 Recovery values > 100 indicate sample expansion during sampling.

Rev. 3 [Ver. 1.1 Jan 02] BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT: O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
25			3		100	SW	Dense, brown to light gray, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine gravel, subrounded to rounded, homogeneous (Qva)				
520	30					SP-SM SP	Dense, light gray, moist, slightly silty gravelly SAND (SP-SM), poorly-graded, trace coarse sand, fine to medium sand, subround to round (Qva) Dense, red brown, moist to wet, slightly gravelly SAND (SP), poorly-graded, trace coarse sand, fine to medium sand, fine to coarse gravel, subround to round, homogeneous, trace silt (Qva)				
515	35		4		91						
510	40		5		100	SW	Dense, red brown, moist, gravelly SAND (SW), well-graded, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
						SP	Dense, red brown, moist, slightly gravelly SAND (SP), poorly-graded, trace silt, trace coarse sand, fine to medium sand, subround to round, homogeneous (Qva)				
						SW	As above at 40.0 feet below ground surface (bgs)				
505	45		6		100	SP	Dense, red brown, moist, slightly gravelly SAND (SP), poorly-graded, trace silt, trace coarse sand, fine to medium sand, subround to round, homogeneous (Qva) Dense, red brown, moist, slightly gravelly SAND (SP), poorly-graded, trace silt, trace coarse sand, fine to medium sand, subrounded to rounded, homogeneous (Qva)				
500	50		7		100	SP-SM GM	Dense, gray, wet, slightly silty, gravelly SAND (SP-SM), poorly-graded, trace coarse sand, fine to medium sand, fine to coarse gravel, subround to round, scattered weathered silty nodules, brown yellow, homogeneous (Qva)				
						SM	Dense, gray, wet, silty sandy GRAVEL (GM), fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva) Dense, brown/gray, wet, slightly silty gravelly SAND (SM), trace coarse sand, fine to medium sand, subround to round, homogeneous, trace Fe(II) staining, weathered yellow brown silt clasts-scattered (Qva)				
495	55		8		88		Becomes very dense, trace fine sand, medium to coarse sand Decreasing silt with depth				
490	60										

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 3 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
60						SW	Dense, red brown, wet, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
						GM	Very dense, light gray, wet, silty GRAVEL (GM), trace medium to coarse sand, fine sand, fine to coarse gravel (2-3 in. diameter), subround to round (Qva)				
485	65		9	100		SP-SM	Dense to very dense, red brown, slightly silty, gravelly SAND (SP-SM), poorly-graded, layers of fine to coarse gravel, subrounded to rounded, homogeneous, fine to coarse sand (Qva)				
480	70					SP	Very dense, red brown, wet, gravelly SAND (SP), poorly-graded, trace silt, trace fine sand, medium to coarse sand, subround to round, homogeneous, scattered oxidation (Qva)				
475	75		10	100		GW	Very dense, red brown, wet, sandy GRAVEL (GW), well-graded, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous, trace silt (Qva) Scattered, laminated clasts of dark brown/light brown silt				Photo labeled as E-115
470	80					SP	Dense, brown, wet SAND (SP), poorly-graded, trace silt, trace medium to coarse sand, trace fine to coarse gravel, fine sand, subround to round, homogeneous (Qva)				
	85		11	100			Increasing medium to coarse sand				
465	85						Grades slightly gravelly, fine sand, fine to coarse gravel, subrounded to rounded				
460	90		12	100			Grades trace coarse sand, fine to medium sand				
455	95										

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95			13		100						
						SW	Dense, brown, wet, slightly gravelly SAND (SW), well-graded, fine to coarse sand, fine to coarse gravel, rounded to subrounded, homogeneous (Qva)				
						SP	Dense, brown, wet, slightly gravelly SAND (SP), poorly-graded, trace coarse sand, fine to medium sand, fine to coarse gravel, subrounded to rounded, homogeneous (Qva)				
450						SW	Dense, brown, moist, slightly gravelly SAND (SW), well-graded, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
100			14		100	SP	Dense, brown/light gray SAND (SP), poorly-graded, trace silt, trace medium to coarse sand, trace fine gravel, fine sand, subround to round, homogeneous (Qva)				
						SW	Very dense, brown, wet, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round (Qva)				
445						SP	Dense, brown, wet SAND (SP), poorly-graded, trace silt, trace medium to coarse sand, trace fine to coarse gravel, fine sand, subrounded to rounded (Qva)				
105			15		100	GW	Becomes very dense, gravelly SAND (SP), trace coarse sand, fine to medium sand, homogeneous (Qva)				
						SP	Very dense, brown, moist, sandy GRAVEL (GW), well-graded, fine to coarse sand, fine to coarse gravel, subrounded to rounded, homogeneous (Qva)				
440							Dense, brown, moist, slightly gravelly SAND (SP), poorly-graded, trace medium to coarse sand, trace silt, fine sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
110			16		100		Scattered clasts of gray sandy SILT				
435							Grades with increased coarse sand				
115			17		100						
430						SW	Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
120						SP	Dense, brown, moist SAND (SP), poorly-graded, trace fine gravel, trace medium to coarse sand, fine sand, subrounded to rounded, homogeneous (Qva)				
425			18		96	SW	Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
125						SP	Dense, brown, moist SAND (SP), poorly-graded, trace fine gravel, trace medium to coarse sand, fine sand, subrounded to rounded, homogeneous (Qva)				
						SW	Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
420						SP	Dense, brown, moist SAND (SP), poorly-graded, trace fine gravel, trace medium to coarse sand, fine sand, subrounded to rounded, homogeneous (Qva)				
						SW	Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subround to round, homogeneous (Qva)				
130											

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 5 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
130			19		100	SP	Dense, brown, moist, slightly gravelly SAND (SP), poorly-graded, trace silt, trace coarse sand, fine to medium sand, fine to coarse gravel, subrounded to rounded, homogeneous, numerous silt/sand clasts (Qva)				
415						SW SP	Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subrund to round, homogeneous (Qva)				
135			20		100		Dense, brown, moist SAND (SP), poorly-graded, trace silt, trace fine gravel, trace coarse sand, fine to medium sand, subrounded to rounded, homogeneous (Qva)				
410			21		100		Dense, brown, moist, gravelly SAND (SW), well-graded, trace silt, fine to coarse sand, fine to coarse gravel, subrund to round, homogeneous (Qva)				
140			22		100		Dense, brown, moist SAND (SP), poorly-graded, trace fine gravel, trace medium to coarse sand, fine sand, subrounded to rounded, homogeneous (Qva)				
			23		100		Layer of well-graded fine to coarse sand Grades slightly gravelly, fine to coarse gravel, subrund to round, homogeneous				
405			24		100		Laminated silt and sand Numerous 1/2-inch to 3/4-inch laminated sand/silt clasts Slightly gravelly from 145 to 146 feet bgs				
145			25		100						
400			26		100		Sand grades fine, trace fine gravel Scattered laminated clasts of sand/silt				
150			27		100						
395			28		100		Grades to slightly gravelly, fine to coarse gravel 12-inch-thick, well-graded slightly gravelly SAND layer				
155											
390											
160											
385											
165											

With numerous +2-inch rip-up clast of sandy SILT

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT; O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
165			29		100		+6 inch cobble				
							Scattered laminated clasts of sand/silt				
380			30		23						
170											
			31		100						
375											
175											
370			32		89	SW	Very dense, brown gray, wet, slightly silty, gravelly SAND (SW), well-graded, fine to coarse sand, fine to coarse gravel, subrounded to rounded, homogeneous (Qva) 3-inch interbed of fine to medium sand				
180											
365						SP	Dense, brown, moist SAND (SP), poorly-graded, trace coarse sand, trace fine to coarse gravel, trace silt, fine to medium sand, subrounded to rounded, homogeneous (Qva) Grades slightly gravelly SAND (SP)				
185											
360			33		93		Grades trace fine to coarse gravel 187 to 190 feet bgs				
190											
355											
195											
350			34		91		Becomes wet				
200											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 7 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
200											
345	205		35	125							
340	210		36	100		Becomes very dense					
335	215		37	100		Becomes dense, brown gray, moist to wet					
330	220		38	100		Layers of very dense, fine sand (SP)					
325	225		39	100							
320	230		40	83		Grades dark gray, slightly silty					
315											
235											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235			41		94						
310	240						Grades moist, fine sand, trace medium sand, fine sand				
305	245		42		93						
300	250										
295	255					ML	Hard, dark gray, moist, clayey SILT (ML), trace fine sand, subrounded to rounded, medium plasticity, medium strength, massive (Qvlc)		MP		
290	260		43		88	SP-SM	Very dense, dark gray, moist, slightly silty SAND (SP-SM), poorly-graded, trace medium sand, fine sand, homogeneous, micaceous, occasional organic fragments (Qpfnf)				
285	265		44		100						
280	270					SM	Dense, dark gray brown, wet, silty SAND (SM), fine sand, subrounded to rounded, homogeneous, rapid dilatancy, occasional organic fragments (Qpfnf)				
							Rapid dilatancy				Perched zone

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 9 of 16

Elevation, feet	Depth, feet	SAMPLES			USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)						
270			45		100					
275										
275										
270										
280			46		100					
265										
285										
260			47		100	Grades to sandy SILT				
290										
290						Grades to silty SAND				
255			48		100					
295										
295						ML Very stiff, dark gray, wet, clayey SILT (ML), trace fine sand, low plasticity, rapid dilatancy, massive, trace organics (Qpfnl)				295-305 foot sample was drilled on 3/11/03 end of day. Sample was lost while pulling rods. Redrilled and left rods in hole overnight to attempt retrieval
250			49		99					
245										
305										

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
305							Increased plasticity, slow dilatancy				
			50		100						
240											
						CL	Hard, gray, moist, very silty CLAY (CL), low to moderate plasticity, medium dry strength, massive, slow dilatancy, occasional organics (Qpfnl)				Termination of 8-inch casing
310											
235											
			51		95						
315											
									MP		Petroleum hydrocarbon odor and apparent oil-like coating on surface of sample probably due to decaying organics Sample loss occurred approximately 320 to 322 feet. Driller required 250 to 300 psi to extrude sample
230							Laminated with light gray silt and fine sand partings/seams, slow to rapid dilatancy				
320											
225							Grades moderate to highly plastic				
			52		75				M AL		Slickensides
325											
220							3-foot stratum grades very stiff, wet, slightly sandy to trace sand, fine sand, subround to round, low plasticity, massive to scattered inderbeds of fine sand				
330											
215											
			53		90						
335											
210						SP-SM	Dense, dark gray, moist, slightly silty SAND (SP-SM), poorly-graded, fine to medium sand, subrounded to rounded, homogeneous, scattered (ML) layers (Qpfnf)				Begin with 6-inch casing and 4-inch core
340											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 11 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
340			54		100		ML Hard, dark gray, moist, clayey SILT (ML), low plasticity, non-dilatant, massive (Qpfnl)				
205							SP-SM Dense, gray green, moist, slightly silty SAND (SP-SM), poorly-graded, fine to medium, trace organics (Qpfnl)				
345							ML Very stiff to hard, gray, moist, clayey SILT (ML), low plasticity, laminated, rapid dilatancy, organic odor (Qpfnl) Rapid dilatancy, low strength Slow dilatancy, medium strength				
200			55		142		Hard, non dilatant, medium strength				
350							Hard, non dilatant, medium strength, laminated/interbedded light gray, fine sand seams		M AL MP		
195							Hard, gray, low to moderate plasticity, medium strength, scattered laminated fine sand seams, slow dilatancy Approximate 2.5-inch brown organic layer				354 to 360 foot sample fell out of core barrel during extraction Conventional radiocarbon date 28,600 +/- 250 years B.P.
355											
190			56		98		Trace medium to coarse sand				Occasional brown organic nodules
360											
185							SM Dense, dark gray, moist, silty SAND (SM), fine sand, subround to round, poorly-graded, homogeneous, trace organics, scattered brown organic nodules (Qpfnl)				
365											
180			57		104		ML Stiff, dark gray, moist, sandy SILT (ML), trace medium sand, fine sand, subround to round, nonplastic, slow to rapid dilatancy, homogeneous (Qpfnl)				
370											
175							ML Hard, dark gray, wet, sandy SILT (ML), fine sand, subrounded to rounded, nonplastic to low plasticity, low strength (Qpfnl)				
375			58		90		CL Hard, dark gray, silty CLAY (CL), trace fine gravel, low plasticity, medium strength, massive, occasional partings of light gray fine sand (Qpfnl)				
										4	

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
375						ML	Stiff, dark gray, wet, slightly sandy, clayey SILT (ML), fine sand, subround to round, low plasticity, low strength, massive, slow dilatancy (Qpfnl)				
		59		100		SM	Dense, dark gray, wet, silty SAND (SM), sand fine, rapid dilatancy (Qpfnf)		M SA		
170						CH	Hard, dark gray, moist, silty CLAY (CH), trace fine sand and fine gravel, moderate to high plasticity, medium strength, non-dilatant, massive, scattered partings of fine sand, light gray (Qpfnl)				
380		60		83					M AL		
165							Numerous partings/nodules of light gray fine sand/silt				
385		61		87							
160							Stiff, wet, slightly sandy, fine sand, subround to round, nonplastic to low plasticity, massive, scattered brown organic nodules				
390		62		100					SA HA		
155							18-Inch thick silty fine SAND stratum, numerous organics, rootlets/woody debris Hard, moist, slightly silty, trace fine sand, low plasticity, medium strength, numerous partings of light gray silt.				
395		63		98							
150							Increasing silt content		M		
400		64		100		SM	Dense, dark gray, moist, silty SAND (SM), fine sand, subrounded to rounded, layers of silty sand with clay (CL) pockets, trace organics (Qpfnf)				
145											
405											
140											
410											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 13 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
410							Dense, dark gray, moist, silty, clayey SAND (SC), fine sand, low to nonplastic (Qpfnf)				Run 65 drilled/sampled 3 times before brought up, TCB starts logging
					ML	Very stiff, dark gray, moist, sandy SILT (ML) (Qpfnl)					
135						Grades slightly clayey, trace fine sand, nonplastic, slow to rapid dilatancy, massive		M SA			
415	65		80								
							Dense, dark gray, moist, very silty SAND (SM), to very sandy SILT (ML), fine sand, trace organics (Qpfnf)				Sample discovered to have slid out at 1615. 1 foot run lost. Drilling done for 3/15 at 1630.
					SM	Grades silty, trace clay, low toughness, nonplastic, fine sand					
130						Decreasing silt					
420						Increasing silt		M SA MP			
							Grades silty SAND (SM) to sandy SILT (ML)				
125							Stiff, dark gray, moist, slightly sandy to sandy SILT (ML), trace medium sand, fine sand, subround to round, nonplastic, slow to rapid dilatancy, homogeneous (Qpfnl)				
425	66		79		ML						
							Dense, dark gray, wet, silty SAND (SM), fine to medium sand, subrounded to rounded, homogeneous (Qpfnl)				
120											
430											
115											
435	67		100		SM						
110											
440											
105											
445	68		93								

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
445											
100	450		69	75		ML	Hard, dark gray, moist, slightly clayey SILT (ML), nonplastic, medium strength, numerous organics (dark brown), black and white organic nodules, massive (Qpfnl)				
	455					SM	Dense, dark gray, wet, silty to slightly silty SAND (SM), fine to medium sand, subround to round, homogeneous, scattered silt clasts (Qpfnf)				Recovery of Run 70 attempted approximately 6 times
95	455										
90	460		70	92							459 to 465 feet sample appears to be intact and not reworked/drilled
85	465					ML	Hard, gray, moist, slightly clayey SILT (ML), low plasticity, low strength, massive, numerous black organic nodules (Qpfnl)				
	465		71	100		OL	Hard, dark brown, moist, organic SILT (OL), nonplastic to low plasticity, medium strength, numerous brown-black organics (Qpfnw)				
80	470					ML	Hard, brown gray to gray, moist, clayey SILT (ML), low plasticity, medium strength, massive, numerous organic nodules (Qpfnl) Becomes sandy Trace fine sand, low plasticity, low strength, slow dilatancy Layer of rapid dilatancy at				
75	475		72	92		OL	Hard, dark brown, moist, organic SILT (OL), nonplastic to low plasticity, medium strength, numerous brown-black organics (Qpfnw)				
70	480					ML	Stiff, gray, wet, slightly clayey SILT (ML), trace fine sand, low plasticity, low strength, massive, scattered organics, slow dilatancy (Qpfnl)				

M
SA

Rev. 3 (Ver. 1.1, Jan 02) BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT; O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-105

Sheet 15 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
480						SP-SM	Dense, dark gray, wet, slightly silty SAND (SP-SM), fine to medium sand, subround to round, poorly-graded, homogeneous (Qpfnf)				
							Grades silty, occasional organics				
65						ML	Grades to very stiff, brown gray, mottled, silt (ML), trace fine sand, nonplastic to low plasticity, medium strength, numerous organics, slow to rapid dilatancy				
485	73		94			SM	Layer rapid dilatancy at 484 ft bgs (Qpfnf) Dense, dark gray, wet, silty SAND (SM), fine to medium sand, subrounded to rounded, homogeneous, occasional organics (Qpfnf) Grades trace fine sand, nonplastic				
60						ML	Medium stiff, gray, wet, sandy SILT (ML), fine sand, nonplastic, homogeneous, rapid dilatancy (Qpfnl) Grades very stiff, moist, slightly clayey, trace fine sand, numerous organic nodules				
490						PT	Very stiff, dark brown, wet PEAT (PT), numerous partings of ash, fibrous, wood debris, 2 to 4-inch layers of gray SILT (ML) (Qpfnw)				
55			94			ML	Medium stiff, gray, wet, slightly sandy SILT (ML), fine sand, nonplastic, scattered organics, low strength, rapid dilatancy (Qpfnl)				
495	74										
50						SM	Dense, dark gray, moist, silty SAND (SM), fine sand, subround to round, homogeneous (Qpfnf)				
500											
45			100				Grades fine to medium sand, numerous silt clasts, trace fine gravel		M SA		
505	75										
40							Grades gravelly, fine to coarse gravel, subround to round				
510											
35							Grades slightly gravelly				
515											

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
515		76			100	SP-SM	Dense, dark gray, moist, slightly silty SAND (SP-SM), fine to medium sand, subround to round poorly-graded, homogeneous, numerous silt clasts (Qpfnf)				
30	520					ML	Interbedded layers of stiff, dark gray/green gray, slightly clayey, slightly sandy SILT (ML), fine sand, nonplastic, low strength, trace brown organic nodules (Qpfnl)				
25	525					SP	Dense, dark gray, moist, slightly silty SAND (SP), poorly-graded, fine to medium, trace gravel (Qpfnl) Grades fine to coarse gravel, subround to round		MP		
20	530	77			100	SM	Dense, dark gray, moist, silty SAND (SM), fine sand, trace medium sand, homogeneous (Qpfnf)				
15	535						Terminated boring at 535 feet below ground surface				
10	540										
5	545										
0	550										

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-106

Sheet 1 of 17

Date(s) Drilled 4/11/03 - 4/22/03	Geotechnical Consultant Camp Dresser & McKee Inc.	Logged By RW	Checked By VJP 02-03-04
Drilling Method/Rig Type Wireline/ T3	Drilling Contractor Cascade Drilling, Inc.	Total Depth of Borehole 566.0 feet	
Casing Size/Type PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.) 300# / 30"	Ground Surface Elevation/Datum 581.9 feet / Metro	
Location 20357 Greenwood Ave	Coordinates N 287281 E 1266085	Elevation Source Survey	

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											0 to 6 ft bgs excavated with vacuum truck, not sampled
580											Drive 6-inch-diameter casing to 18 feet bgs
575						GW-GM	Dense, yellow-brown, moist, slightly silty, sandy GRAVEL (GW-GM), trace cobbles, well-graded, fine to coarse subangular gravel (Qvtm)				Soil description inferred from drill action and cuttings
570											Driller reports hard drilling in cobbles and boulders
565		■	1	46 - 50/2" (100+)	0						No recovery, stone in sampling shoe
560											Drillers use tri cone bit for drilling through formation from 21 to 37 feet bgs
25											

Groundwater Observation Data:
 OW (FT BGS): 390.0 (Low) 388.9 (High)
 VWP1 (FT BGS):
 VWP2 (FT BGS):

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
 Recovery values > 100 indicate sample expansion during sampling.

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
25											
555											
30											
550											
35											
545		2	19 - 50/5" (100+)	9	GW	Very dense, brown, moist, sandy GRAVEL (GW), well-graded, fine to coarse subangular gravel (Qva)					Hard drilling
40											
540					SW-SM	Very dense, brown, moist, slightly silty, gravelly SAND (SW-SM), well-graded, fine to coarse sand, fine to coarse subangular gravel (Qva)					Drilling smoother Soil description inferred from drill action and cuttings
45											
535											
50											
530											
55											
525		3	55/6" (100+)	100							
60											

Rev. 3 { Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 3 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
60											
520											
65											
515											
70											
510											
505		4	50/5" (100+)	40	GW	Very dense, gray, moist to wet, sandy GRAVEL (GW), trace silt, well-graded, fine to coarse subangular to subrounded gravel (Qva)					Drill action suggests cobbles
75											
80											
500											
85											
495											
90											
490											
95											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95											
485		5		50/4" (100+)	101	SP	Very dense, wet, olive gray, slightly gravelly to gravelly SAND (SP), poorly-graded, fine to coarse sand, fine to coarse subangular to subrounded gravel (Qva)				Cobbles noticed by drillers
100											Soil description inferred from drill action and cuttings
480											
475											
105											
470											Drill action suggests some gravel
465		6		50/4" (100+)	0						
120											
460											
125											
455											
130											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 5 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
130											
450											
135											
445		7	50/4" (100+)	0							Drill action suggests gravelly sand
140											
440											
145											
435											
150											
430											
155						Grades brown					
425		8	50/5" (100+)	99							Soil description inferred from drill action and cuttings
160											
420											
165											

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 6 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
165											
415											
170											
410											
175											
405		9	50/3" (100+)	0							Gravel in shoe, drillers assume sandy gravel to gravelly sand with cobbles
180											
400											
185											
395											
190											
390											
195											
385		10	50/5" (100+)	99		Grades gravelly					Soil description inferred from drill action and cuttings
200											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 7 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
200											
380											
205											
375											
210											
370											
215											
365		■	11	50/6" (100+)	100	SP-SM	Very dense, brown, wet, slightly gravelly, slightly silty SAND (S-SM), poorly-graded, fine to coarse sand, fine subrounded gravel (Qva)				Soil description inferred from drill action and cuttings
220											
360											
225											
355											
230											
350											
235											

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 8 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235											
345		■	12	50/6" (100+)	100		Grades olive gray, sand, trace silt, trace gravel				Soil description inferred from drill action and cuttings
240											
340											
245											
335											
250											
330											
255											
325		■	13	50/4" (100+)	76		Grades moist, fine to medium sand				
260											
320											
265											
315											
270											

Rev. 3 { Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 9 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
270											
310											
275						GM	Silty sandy GRAVEL (GM) (Qpfnf)				Gravel inferred by drill action and cuttings Soil description inferred from drill action and cuttings
305											Switch to wire-line Slough, outwashed sand
280	14			40							
300											
285						ML	Stiff, olive gray, moist, slightly clayey, sandy SILT (ML), medium plasticity, scattered organics (Qpfnl)				
295	15			67			Grades very stiff, no clay				
290											Organic odor
290						SP	Dense, olive gray, moist, SAND (SP), trace silt, poorly-graded fine to medium sand, occasional organics (Qpfnf)				
295											
285	16			100							
300											
280											
305							Scattered organics				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
305											
275			17		100						
310							Wood fragment				
						ML	Stiff to very stiff, gray, moist SILT (ML), low plasticity (Qpfn)				
270							Grades slightly sandy, hard, nonplastic				
315							Occasional organic fragments				
265			18		100		Grades sandy				
							Trace sand, low plasticity				
320							Trace clay				
260											
325											
255			19		100		Grades slightly clayey				
330											
250							Grades clayey silt, slickensided, homogeneous				
335							1-foot fine sand layer				
245			20		100						
340											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
340											
240						Trace sand					
						Frequent seams of lighter-colored silt					Organic odor
345						1-foot silty fine sand layer					
						Grades silt and clay					
235		21		84							
350					SP	Dense, dark gray, moist, SAND (SM), trace silt, poorly-graded, fine to medium sand, scattered organics (Qpfnf)					Organic odor
						Medium dense, olive gray					Organic odor
230											
355					ML	Very stiff, dark gray, moist, slightly clayey SILT (ML), fine sand seam at 355 ft bgs, low plasticity, homogeneous, slickensides (Qpfnl)					
225		22		90							
						1-foot fine sand layer					
360					CH	Hard, dark gray, moist silty CLAY (CH), high plasticity (Qpfnl)					Organic odor
220											Slickensides
					SP	Dense, dark gray, moist, SAND (SP), trace silt, poorly-graded, fine to medium sand (Qpfnf)					Organic odor
365											
215		23		79	CH	Very stiff, dark gry, moist, silty CLAY (CH), high plasticity, slickensides (Qpfnl)					
						Sand layers					
370											
210						Grades stiff					
375											

Rev. 3 (Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER, GLOB-BRIGHTWATER, GDT); O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
375							Grades, hard, sandy clay, scattered organics				
205		24		100		SP	Very dense, dark gray, moist, SAND (SP), trace silt, poorly-graded fine sand, occasional organics (Qpfnf)				Drill action suggests some gravel
380							Grades fine to medium				
200							Becoming gray green				
385											
195		25		100		CL	Hard, dark gray, moist, silty CLAY (CL), medium plasticity, slickensided, homogeneous, fine sand layers (Qpfnl)				
390											
190						MH	Hard, dark gray, moist, clayey SILT (MH), medium to high plasticity (Qpfnl)		M AL		Interbedded fine sand has strong organic odor
395											
185		26		98					M		
400											
180						CH	Hard, dark gray, moist, silty CLAY (CH), medium to high plasticity (Qpfnl)		M AL	4	
405											
175		27		100		SP	Dense, dark gray, moist SAND (SP), trace silty, poorly-graded sand, scattered organics (Qpfnf)				Strong organic odor in fine sand
410											

Rev. 3 (Ver. 1.1, Jan 02) BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 13 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
410											
-170					CL	Hard, dark gray, moist, silty CLAY (CL), medium plasticity, slickensided, homogeneous, fine sand layers (Qpfnl)		M SA			
					ML	Stiff, dark gray to gray green, moist, SILT (ML), trace sand, trace clay, low plasticity, scattered organics (Qpfnl)					
415								M SA		Organic odor	
-165		28		59							
					SP	Very dense, dark gray, moist, SAND (SP), trace silt, poorly-graded fine sand, scattered organics (Qpfnf)					Organic odor
420											
-160						Grades dense, fine to medium sand					
425								M SA			
-155		29		95							
					ML	Hard, dark gray, moist, slightly sandy SILT (ML), scattered organics, nonplastic (Qpfnl)					
430											
-150					SP	Very dense, dark gray, moist, SAND (SP), trace silt, poorly-graded fine sand, scattered organics (Qpfnf)					Outwashed gravel
435											
-145		30		100		Grades fine to medium sand					
						Wood pieces					
440											
-140					PT	Brown PEAT (Qpfnw)					
					CL	Very stiff, dark gray, moist, silty CLAY (CL), medium plasticity (Qpfnl)					
					ML	Hard, gray olive, slightly clayey, sandy SILT (ML), low plasticity, occasional organics (Qpfnl)					
445						Very dense, gray green, moist, SAND (SP), trace					

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
445			32		82	SP	silt, poorly-graded fine sand (Qpfnf) Grades dense, dark gray, fine to coarse sand, trace fine gravel, poorly-graded sand				Hard drilling
135											
450			33		85	ML	Hard, gray, moist, sandy SILT (ML), low plasticity, scattered organics (Qpfnl) 6-inch laminated brown organic silt and sand		M		
130											
455			34		100	SP	Very dense, light gray, moist, SAND (SP), poorly-graded fine sand, homogeneous, occasional organics (Qpfnf)				
125											Organic odor
460			35		100	OL	Hard, gray brown, moist, organic SILT (OL), trace sand, trace clay (Qpfnw)		M		Hard drilling
120											
465											
460											Organic odor
115											
470											
110											
475											
475			37		43	SP	Dense, dark gray, moist, SAND (SP), trace silt, poorly-graded fine to coarse sand, occasional organics (Qponf)				Hard drilling
105											
480											
480			38		0		Grades fine sand, scattered organics				Outwashed sand
											No recovery, outwashed sand

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 15 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
480							Wood pieces				
100							Grades very stiff, sandy silt, low plasticity				
485			39		80						Hard drilling
95			40		0						Circulation mud: Fine to coarse sand, no recovery, outwashed sand
490						GW-GM	Slightly silty, gravelly SAND (SW-SM) to slightly silty sandy GRAVEL (GW-GM) (Qpfnb)				Soil description inferred from drill action and cuttings Circulation mud: Sandy gravel to gravelly sand shell fragments
90											No recovery
495			41		0						Gravelly drilling
85											
500											Gravelly drilling
80											No recovery
505			42		0						
75											
510											Gravelly drilling
70											
515			43		0						

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ_2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
515											
65			44	0							Coarse subrounded gravel inferred by drill action and cuttings
520											
60											
525			45	17		SW	Very dense, dark gray, moist, SAND (SW), trace silt, trace fine gravel, well-graded fine to coarse sand, occasional organics (Qpfnf)				
55						CL	Very stiff, dark gray, moist, sandy, silty CLAY (CL), medium plasticity (Qpfnl)				
530						GM	Very dense, slightly silty, sandy GRAVEL (GM), fine to coarse sand, subrounded gravel (Qpfnf)				
50			46	53		CL	Stiff, dark gray, moist, sandy, silty CLAY (CL) medium plasticity (Qpfnl)				
535						SP	Dense, dark gray, wet SAND (SP), trace silt, poorly-graded fine to coarse sand (Qpfnf)				
45						GW-GM	Slightly silty, gravelly SAND (SW-SM) to sandy GRAVEL (GW-GM) (Qpfnf)				Soil description inferred from drill action and cuttings Gravelly drilling
540			47	0							
40											
545			48	70		SM	Very dense, dark gray, moist, very silty, slightly gravelly, SAND (SM), fine to coarse sand, fine to coarse subrounded gravel, scattered organics (Qpogl)				
35			49	70					M		
550											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER G.L.B.-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-106

Sheet 17 of 17

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
550											
30											
555			50		61						
25											
560			51		91						
20											
565			52		100				SA HA		
15							Terminated boring at 566 feet below ground surface				
570											
10											
575											
5											
580											
0											
585											

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-107

Sheet 1 of 16

Date(s) Drilled	3/19/03 - 3/24/03	Geotechnical Consultant	Camp Dresser & McKee Inc.	Logged By	RW	Checked By	VJP 02-03-04
Drilling Method/Rig Type	D&M/Wireline/ T3	Drilling Contractor	Cascade Drilling, Inc.	Total Depth of Borehole	548.0 feet		
Casing Size/Type	PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.)	300# / 30"	Ground Surface Elevation/Datum	549.5 feet / Metro		
Location	782 N. 204th SE	Coordinates	N 287288 E 1268071	Elevation Source	Survey		

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											0-8 feet excavated with vacuum truck, not sampled
545	5										
540	10					GP	Very dense, brown, wet, sandy GRAVEL (GP), trace silt, poorly-graded fine to coarse sand, poorly-graded fine subangular gravel (Qva)				Drilling Mud Rotary 8 to 180 feet below ground surface (bgs) Soil description inferred from drill action and cuttings Irregular drilling resistance in gravel
535	15										
530	20	■	1	50/3" (100+)	33						
525	25										

Groundwater Observation Data:

OW (FT BGS):

VWP1 (FT BGS) 223.8 (Low) 221.3 (High)

VWP2 (FT BGS) 223.9 (Low) 220.8 (High)

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
Recovery values > 100 indicate sample expansion during sampling.

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 2 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
25											
520	30										
515	35										
510	40	■ 2	50/5" (100+)	50	GW	Very dense, brown, wet, sandy GRAVEL (GW), well-graded to sandy GRAVEL (GP), poorly-graded, fine to coarse gravel, fine to coarse sand, subangular gravel (Qva)					Soil description partially inferred from drill action and cuttings
505	45										
500	50										
495	55										
490	60										

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 3 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
60	60	■	3	50/4" (100+)	0	[Graphic Log: Sand and gravel in circulation mud]	[Piezometer Schematic: Dashed lines]	[Lab Tests: Empty]	[Pocket Penetrometer: Empty]	Sand and gravel in circulation mud	
485	65										
480	70										
475	75										
470	80	■	4	60/5" (100+)	0					Sand and gravel in circulation mud	
465	85										
460	90										
455	95										

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 4 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95											
450	100	■	5	100/3" (100+)	0						Sample collected from cuttings of circulation mud (crushed gravel)
445	105										Hard drilling in gravel and cobbles
440	110					SP-SM	Very dense, brown yellow, wet, slightly silty, gravelly SAND (SP-SM), poorly-graded fine to medium sand, fine subangular gravel (Qva)				Drilling smooth
435	115										Soil description inferred from drill action and cuttings
430	120	■	6	50/5" (100+)	50						
425	125										
420	130										


Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 5 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
130											
415	135										
410	140	8	50/4" (100+)	0		GW Sandy GRAVEL (GW), well-graded to sandy GRAVEL (GP), poorly-graded, fine to coarse gravel, fine to coarse sand, subangular gravel (Qva)				Soil description inferred from drill action and cuttings	
405	145										
400	150										
395	155										
390	160	9	50/5" (100+)	0							
385	165										

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 6 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
165											
380	170										
375	175										
370	180	■	10	50 - 50/1" (100+)	67	SP-SM	Very dense, brown yellow, wet, slightly silty gravelly SAND (SP-SM), poorly-graded fine to coarse sand, fine subangular gravel (Qva)				Soil description inferred from drill action and cuttings
365	185										
360	190										
355	195										
350	200										Switch from mud rotary to wire-line at 180 feet; no core recovery. Switch to Mud Rotary from 185 to 229 feet bgs

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
200		■	11	50/4" (100+)	67		Transitions fine to medium sand, dropstones of coarse gravel		BG		
345	205										
340	210										
335	215										
330	220	■	12	50/4" (100+)	50	SP	Very dense, olive gray, moist, SAND (SP), trace silt, trace gravel, poorly-graded fine to medium sand, homogeneous (Qva)				
325	225	■	13	75/5" (100+)	50		Transitions - no gravel				
320	230	■	14	20 - 50/2" (100+)	67						
			15		100						
315	235	□	16		94						Switch to wire-line drilling method

Rev. 3 (Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 8 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235						ML	Very stiff, olive gray to gray blue, moist, slightly sandy, slightly clayey SILT (ML), medium plasticity, homogeneous (Qpfn) Occasional organics				
	235	17	108								
	310						Transitions hard, gray, clayey silt, low plasticity		BG	4	
	240	18	50								
	305						Transitions stiff, wet, trace clay		MP	2.6	
	245	19	108								
	300					SM	Dense, gray to gray green, wet, silty SAND (SM), fine sand, homogeneous, occasional organics (Qpfn) Slightly silty				
	250	20	90								
	300										
	255	21	98								
	295										
	255	22	96								
	290										
	260	23	70			ML	Very stiff to hard, gray, moist SILT (ML), low plasticity, homogeneous (Qpfn)				
	285					CL	Very stiff, gray, moist, silty CLAY (CL), medium plasticity, homogeneous (Qpfn)				
	265	24	70								
	280										
	270	25	120			ML	Very stiff, gray, moist, clayey SILT (ML), medium plasticity (Qpfn)		MP		
	280										
	270	26	76								

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 9 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
270						SP	Very dense, gray green, moist, SAND (SP), trace silt, poorly-graded fine sand, homogeneous, occasional organics (Qpfnf)				
							3.5-inch very stiff, sandy, silty clay layer				
275	27			78			Wood fragments				
270	28			111			18-inch stratum wet, scattered organics and wood fragments		BG		
280							Transitions fine to medium sand				
265	29			100							
260	30			104							
255	31			100		SM	Dense, gray, wet, slightly silty SAND (SM) (Qpfnf) 12-inch silty clay layer				
250	32			80			18-inch silty clay stratum			4.5	Slickensides
245	33			91		SP	Very dense, gray, wet, SAND (SP), trace silt, poorly-graded fine to medium sand, occasional organics, homogeneous (Qpfnf)				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 10 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
305						SM	Very dense, gray, wet, silty SAND (SM), fine sand, occasional organics (Qpfnf)				
	310	34		104		CL	Very stiff, olive gray, moist, silty CLAY (CL), medium plasticity, sandy layers, massive (Qpfnl)			4.2	Slickensides
240											
	315	35		60							
235											
	320	36		100		SM	Very dense, olive gray, moist, silty SAND (SM), trace clay, fine to medium sand, homogeneous, occasional organics (Qpfnf)				
230											
	325	37		100			Sandy silty clay layer		BG		
225						CH	Very stiff, olive gray, moist, silty CLAY (CH), medium to high plasticity (Qpfnl)				Slickensides
	330	38		80							
220											
	335	39		44			12-inch sandy silt layer		BG MP		
215							Transitions slightly silty clay				
	340	40		64			Coarse gravel, subangular		M AL		
210						GC	Very dense, gray, wet, slightly sandy, silty, clayey GRAVEL (GC), fine to coarse subangular gravel				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
340						(Qpfnf)					Gravel-size granite and sandstone
	205		41	2							
	345		42	67		SP-SM	Very dense, gray green, wet, slightly silty SAND (SP-SM), poorly-graded fine to medium sand, homogeneous (Qpfnf)				344 to 345.5 feet bgs, sample appears to be slough
			43	108			6-inch gravelly, fine to coarse sand layer				348 to 350.5 feet - organic odor
200	350										
			44	88		CL	Hard, gray green, moist, sandy CLAY (CL), low plasticity, oxidized, scattered organics, dropstones (fine gravel) (Qpfnf)		M AL MP BG		Slickensides dip approximately 20 degrees from horizontal
195	355						Transitions gray, occasional organics				
			45	30							
190	360						Grades to clayey silt				
			46	82		GC	Very dense, gray, wet, clayey GRAVEL (GC), subrounded, in clayey matrix (Qpfnf)				Soil description inferred from drill action and cuttings
185	365						Silty sand layer 0.5 foot thick		M SA		
			47	0							Drill action infers gravel
180	370										
			48	0							
175	375										

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 12 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
375											
	49		20		SP-SM	Loose, gray green, wet, slightly silty SAND (SP-SM), trace gravel, poorly-graded fine sand, fine subrounded gravel, occasional organics, homogeneous (Qpfnf)					
170	380				GW	Dense, gray, wet GRAVEL (GW), well graded, fine to coarse gravel, subrounded (Qpfnf) Transitions wet, fine to medium gravel		M SA			
	50		43					M SA			
165	385				ML	Stiff, gray green, wet, sandy SILT (ML), trace clay, low plasticity, homogeneous (Qpfnl)				3	
	51		78					M			
160	390				SP-SM	Loose, gray green, wet, slightly silty SAND (SP-SM), poorly-graded fine to coarse sand, occasional organics, homogeneous (Qpfnf)					
	52		100								
155	395				GW	Loose, gray green, slightly silty, sandy GRAVEL (GW), well-graded fine to coarse sand, well-graded fine to coarse gravel (Qpfnf)					
	53		60					M SA			
150	400				SP	Medium dense, gray green, wet SAND (SP), trace silt, poorly-graded fine to medium, occasional organics, homogeneous (Qpfnf)					
	54		10								
145	405				SM	Dense, gray olive, moist, silty SAND (SM), fine to medium sand, homogeneous (Qpfnf)					
	55		100								
140	410										

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
410						SP	Medium dense, gray green, wet SAND (SP), trace silt, poorly-graded fine to medium sand, occasional organics, homogeneous (Qpfnf)				
		56		100							
135	415						Gray olive, silty sand layer		BG		Organic odor
		57		100							
130	420						Silty sand layer				
		58		58							
125	425					GW	Dense, gray olive, wet, sandy GRAVEL (GW), trace silt, well-graded fine to coarse sand and well-graded fine to coarse subrounded gravel (Qpfnf)				Hard drilling
		59		30							
120	430										Soil description inferred from drill action and cuttings
		58		0							
115	435					SP-SM	Gray olive, wet, slightly silty SAND (SP-SM), poorly-graded fine to medium sand, occasional organics, homogeneous (Qpfnf)		M SA		Organic odor
		60		82							
110	440								BG		Organic odor
		61		100							
105	445						Sandy silt layer				
		62		96							

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
445											
							Transitions gray green color				
							Occasional organics				
100	450		63	96			Silty gravelly sand layer, occasional organics				
						ML	Hard, gray, moist, slightly clayey, sandy SILT (ML), trace fine gravel, low plasticity, occasional organics (Qpfn)		MP		
95	455		64	22			Transitions gravelly silt				
						SP	Dense, gray green, wet SAND (SP), trace silt, poorly-graded fine to medium sand (Qpfn)				
90	460		65	62			12-inch gravel layer (Qpfn)				
							12-inch gravel layer				
85	465		66	46			4-foot slightly gravelly stratum				
80	470		67	100							Organic odor
75	475		68	100							Organic odor
70	480		69	100		SP-SM	Medium dense, gray green, wet, slightly silty SAND (SP), poorly-graded fine to medium sand (Qpfn)		M SA		

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
480											
			70	70							Organic odor
65	485										
			71	100							
60	490						Transitions gray olive, occasional organics				
			71	100			2-foot slightly silty stratum				
55	495										
			73	100							Organic odor when cutting core from 495 to 520 feet bgs
50	500										
			74	100							
45	505										
			75	100			Silty sand layer		M SA		
40	510										
			76	100							
35	515										

Rev. 3. (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-107

Sheet 16 of 16

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
515											
	30		77		100						
	520										
	25		78		90	ML	Very stiff, gray, moist, sandy SILT (ML), occasional organics (Qpfnl)				
	525										
	20		79		90						
	530										
	15		80		60						Loss of mud circulation
	535		81		70	SP	Dense, gray, wet SAND (SP), poorly-graded fine to medium sand (Qpfnf)		MP BG		
	10		82		56						
	540						Wood fragments		M SA		
	5		83		34						Organic odor
	545										
	0		84		100						
	550						Terminated boring at 548 feet below ground surface				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER GLB-BRIGHTWATER GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-108

Sheet 1 of 11

Date(s) Drilled 7/7/03 - 7/7/03	Geotechnical Consultant Camp Dresser & McKee Inc.	Logged By SHE	Checked By RWS 2/03/04
Drilling Method/Rig Type Wireline/ Porta-drill	Drilling Contractor Gregory Drilling, Inc.	Total Depth of Borehole 346.0 feet	
Casing Size/Type PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.)	Ground Surface Elevation/Datum 453.1 feet / Metro	
Location 1621 N. 205th St	Coordinates N 287249 E 1270073	Elevation Source Survey	

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											0 to 8 ft bgs excavated with vacuum truck, not sampled
450											
5											
445						SP	Wet, brown, gravelly SAND (SP), poorly graded (af)				70% to 80% quartz, gravel 60% to 70% dark gray volcanics Soil description inferred from drill action and cuttings
10											
440											
15											
435											
20											
430		1		0							
25											

Groundwater Observation Data:
 OW (FT BGS): 139.9 (Low) 137.8 (High)
 VWP1 (FT BGS):
 VWP2 (FT BGS):

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
 Recovery values > 100 indicate sample expansion during sampling.
 Pocket Penetrometer shown as 4.6 indicates unconfined compressive strength > 4.5 tsf (penetrometer upper limit).

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 2 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
25											
425											Drill on something metallic at 27 ft bgs
30						CL	Soft, yellow brown, moist CLAY (CL), occasional fine organics, wire and other debris (af)				Soil description inferred from drill action and cuttings
420											
35											Drilled out to 36 ft bgs
415											Driller reports soft material consistent with soft clay
40		2		0							
410						SP-SM	Medium dense, dark brown, moist, slightly silty to silty, SAND (SP-SM), trace fine gravel, poorly-graded sand (Qal/Qvrf)				Soil description inferred from drill action and cuttings
45											Driller report: sand beds
405											
50		3		0							
400											
55											
395											
60		4		0							Driller report: "loose" sand and gravel

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 3 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
60											
390											
65											
385			5		0						
70											
380											
75											
375											
80			6		0						
370											
85											
365											
90			7		0						
360											
95											

Intermittant heavy rattling (gravel or dense sand)
 Soil sample fell out of core barrel

Rev. 3 { Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 4 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95											
355	100		8		0						Driller report: "loose" sand
350											
105											
345	110		9		74	SP	Dense, dark brown, moist to wet SAND (SP), trace silt, poorly-graded sand, homogeneous to laminated medium to coarse sand layers, scattered red brown bands and laminae (Qva)				Change in drill action infers increase in soil density
340											
115							Grades gray, wet, fine to medium sand				
335	120		10		94						
330											
125											
325											
130			11		70	ML	Very stiff, olive gray, moist, clayey SILT (ML), medium plasticity, slow dilatancy, laminated to				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
130							layered, sand layers at contact (Qpfnl)				
320						CL	Very stiff, olive gray, moist, silty CLAY (CL), medium plasticity, slow to no dilatancy, laminated (Qpfnl)		MP		Scattered fractures with off-sets
135											
315											
140		12		72							
310											
145						SM	Olive gray, wet, silty SAND (SM), laminated, occasional organics, micaceous (Qpfnf)				
305											
150		13		78							
300						ML	Stiff, olive gray, wet, clayey SILT (ML), homogeneous (Qpfnl)				
155							Grades hard and moist			4.5	
295						CL	Hard, olive gray, moist, silty CLAY (CL) (Qpfnl)				
160		14		86			Frequent seams of lighter colored silt				
290							6-inch silty sand layer				
165											

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 6 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
165							6-inch wet, silty sand layer				
285			15	73		SM	Dense, dark gray, moist, silty SAND (SM), occasional organics, homogeneous, scattered mica (Qpfnf)		MP		
170							18-inch silty clay stratum				
280							Laminated, silty clay and silt layers 177 to 178 ft bgs and 178.5 to 179 ft bgs				
175			16	70							
275											
180											
270						CL	Hard, gray olive, moist, silty CLAY (CL), trace fine sand, frequent fine sand laminations (Qpfnl)				Sheared zones 184 to 185 ft bgs
185											
265			17	50					MP		
190											
260											
195							1-foot yellow brown, silty gravel layer				Palesol
255			18	10							
200											

Rev. 3 (Ver. 1.1, Jan02BRIGHTWATER-BRIGHTWATER, CLB-BRIGHTWATER, GDT), C:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
200											
250							Grades yellow brown and gray mottled, gravel layers, occasional organics			2.5	Blocky, fractured, slickensides 203 to 212 ft bgs
205											
245											
210		19		25							
240						SM	Dense, gray olive, moist to wet silty SAND (SM), fine sand, scattered gravel layers, clay coatings on gravel, occasional organics (Qp _{fmw})			2.5	Disturbed bedding, brecciated texture
215							6-inch clayey silt layer				
235											
220		20		27			Grades wet, slightly silty				
230											
225											
225						CL	Hard, mottled and streaked yellow red to gray green, dry to moist, silty CLAY (CL), trace gravel, coarse sand (Qp _{fmw})			1.5	Lithologies granitic (diorite) and volcanic
225						ML	Hard, dark olive gray, moist, clayey SILT (ML), medium plasticity, slow dilatancy, gravelly layers (Qp _{fmw})				
230		21		50							
220											
235							Mottled brown gray, gavelly		MA		Brecciated texture 234 to 237 ft bgs

Rev. 3 (Ver. 1.1, Jan02BRIGHTWATER-BR-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 8 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235											
215	22			61		SM	Dense, dark gray, wet, silty SAND (SM), homogeneous, occasional organics (Qpfnf)				Slight organic odor
240							Grades sandy silt layers		M AL	4.6	
210										0.25	
245	23			88							
205											
250							Grades fine to medium sand, numerous organics,				
200						SP-SM	Dense, dark gray, wet, slightly silty SAND (SP-SM), poorly-graded fine sand, homogeneous (Qpogf)		M SA HA AL M		
255											
195	24			66		CL	Hard, silty, gravelly CLAY (CL) (Qpogd)		MP		
260						SP	Dense, dark gray, wet, silty SAND (SP) poorly-graded fine to medium sand, homogeneous, occasional organics (Qpogf)		M SA		
190											
265						SP	Dense, dark gray, wet SAND (SP), trace silt, poorly graded fine sand (Qpogf)				
185	25			73					MP		
270											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
270											
180							Occasional organics, stems				
275						CL	Hard, dark olive gray, moist, gravelly, silty CLAY (CL), trace fine sand (Qpogd)			4.5	Fractures, slickensides
175		26		50							
280											
170											
285		27		56			18-inch dense, wet, fine to coarse sand layer, occasional organics				
165											
290						CH	Hard, dark gray, moist, slightly silty CLAY (CH), high plasticity, scattered gravel and cobbles (Qpogm)			3	Sheared zones observed several layers, with slickensides, scattered planar fractures with slickensides
160		28		84			3-inch brown, organic silt layer		MP	4	
295											
155							Slightly silty, clay from 296 to 300 ft bgs Scattered fine gravel dropstones				Cobble Brecciated texture 296 to 304 ft bgs
300		29		100							
150											
305											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER-GLB-BRIGHTWATER-GDT) O:\GINT\PROJECTS\BRIGHTWATER P-19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
305							Grades trace fine gravel				Hydrogen sulfide odor
145						Abundant clam shells					
310		30		100		SP Dense, dark gray, wet, SAND (SP), poorly-graded fine sand, homogeneous, shell fragments (Qpogm)		MP			
140					ML	Hard, olive gray, moist, very gravelly, sandy SILT (ML), trace clay, low to medium plasticity, matrix supported (Qpogd)					
315						Grading silt, trace fine sand, low plasticity, slow dilatancy					
135					ML	Hard, olive gray, moist clayey SILT (ML) (QpogI)					
320		31		77		Gravelly bed					
130											
325					SP	Dense, olive gray, wet SAND (SP), trace silt, poorly-graded fine to medium sand, homogeneous (Qpogf)					
125					CL-ML	Very stiff, olive gray, moist, clayey SILT (ML) homogeneous (QpogI)			4		
330		32		83		Sand layer					
120											
335											
115											
340							2-foot sand stratum				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-108

Sheet 11 of 11

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
340			33		66		Grading trace sand and fine gravel		MP		
110											
345							Terminated boring at 346 feet below ground surface				
105											
350											
100											
355											
95											
360											
90											
365											
85											
370											
80											
375											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-109

Sheet 1 of 8

Date(s) Drilled	7/29/03 - 7/30/03	Geotechnical Consultant	Camp Dresser & McKee Inc.	Logged By	RW	Checked By	VJP 02-03-04
Drilling Method/Rig Type	Wireline/ T3	Drilling Contractor	Cascade Drilling, Inc.	Total Depth of Borehole	260.0 feet		
Casing Size/Type	PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.)	N/A	Ground Surface Elevation/Datum	395.0 feet / Metro		
Location	NE 205th St and 1st Ave	Coordinates	N 287168 E 1272288	Elevation Source	Survey		

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
395	0										0 to 6 feet excavated with vacuum truck, not sampled
390	5										
385	10		1	38		GP-GM	Medium dense, brown, moist to wet, slightly silty, sandy GRAVEL (GP-GM), poorly-graded fine to coarse sand, fine to coarse subangular to subrounded gravel (af)				
380	15		2	15		GM	Loose, gray, wet, silty, sandy GRAVEL (GM), fine to coarse sand, fine to coarse subangular to subrounded gravel (af)				
375	20					SM	Hard, brown yellow to yellow red, gravelly, silty SAND (SM), scattered cobbles (Qpfnf)				
						SW	Loose, brown, wet, gravelly, SAND (SW), well-graded fine to coarse sand, fine to coarse subangular gravel (Qpfnf)				
	25										

Groundwater Observation Data:
OW (FT BGS): 130.6 (Low) 115.0 (High)
VWP1 (FT BGS):
VWP2 (FT BGS):

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
Recovery values > 100 indicate sample expansion during sampling.
Pocket Penetrometer shown as 4.6 indicates unconfined compressive strength > 4.5 tsf (penetrometer upper limit).

Rev. 3 (Ver. 1.1, Jan02) BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
370	25		3		25						
365	30					ML	Hard, brown yellow, moist, sandy SILT (ML), low plasticity (Qpfnl) Grades dark gray, clayey, low to medium plasticity, laminated				
360	35		4		85						
355	40						Soft to medium stiff, wet				
350	45		5		45						
345	50					SM	Medium dense, dark gray, wet, silty to slightly silty, SAND (SM), fine sand, occasional organics (Qpfnf)				
340	55		6		45		Dark brown, numerous organics				
						ML	Hard, dark brown to olive gray, moist, sandy SILT (ML), 0.5-foot layer of peat (Qpfnl)				Paleosol
60							Transitions gray, wet, rapid dilatancy, no organics				

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-109

Sheet 3 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
335	60										
330	65		7		20						
325	70					ML	Hard, gray, moist, sandy SILT (ML), fine sand, rapid dilatancy (Qpogl)				Blocky structure, scattered slickensides
320	75		8		20						
315	80										
310	85		9		60		Grades slightly gravelly, trace sand			4	Scattered gravel dropstones
305	90									4.6	
95						GM	Medium dense to dense, dark gray, moist to wet, slightly sandy, silty GRAVEL (GM), fine to coarse subrounded gravel (Qpogd)				

Rev. 3 (Ver.1.1, Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
300	95		10		15						
295	100		11		0	SW	Medium dense, dark gray, wet, gravelly SAND (SW), well graded sand (Qpogf)				
290	105		12		0						
285	110					GM	Dense, gray, wet, sandy, silty GRAVEL (GM), fine to coarse subround gravel (Qpogf)				
280	115		13		24	SM	Medium dense, dark gray, wet, silty SAND (SM), fine sand (Qpogf)				
275	120					GM	Grades dense, moist, slightly gravelly Dense, dark gray, moist, silty, sandy GRAVEL (GM) (Qpogd)				
270	125		14		30						
130											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
265	130										
260	135		15		15					2.5	
255	140					ML	Hard, dark gray, moist, sandy SILT (ML), nonplastic to low plasticity, fequent shell fragments (Qpogm)			4	Scattered gravel dropstones
250	145		16		35						
245	150									4.6	
240	155		17		25						
235	160									4.6	
165							Grades wet, slow to rapid dilatancy				

Rev. 3 (Ver.1.1.Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER.P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
230	165		18		53		Grades slightly gravel				High angle fracture at 70 degrees
						CL	Hard, dark gray, moist, silty CLAY (CL), low plasticity (QpogI)				Scattered slickensides 30 to 40 degrees
225	170						Grades trace fine to coarse sand and fine gravel				
220	175		19		50						
						ML	Hard, dark gray, moist, sandy SILT (ML), low plasticity (QpogI)				
215	180					GM	Dense, dark gray, moist, sandy, silty GRAVEL (GM), fine to coarse subround gravel (Qpogt)				
						ML	Hard, dark gray, moist, slightly clayey SILT (ML), trace sand, low plasticity (QpogI)				
210	185		20		30				MSA		
205	190					CH	Hard, dark gray, moist, silty CLAY (CH), medium to high plasticity (QpogI)				
									MAL	4.6	
200	195		21		78						Dropstones 195.5 to 196 ft bgs, vertical planar fracture
										4.6	
200											

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT; O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-109

Sheet 7 of 8

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
195	200										
190	205		22	15			Grades soft to medium stiff, wet, sandy, clayey silt, medium plasticity		M AL		
185	210					GM	Dense, dark gray, moist, silty, sandy GRAVEL (GM) (Qpogd) Grades fine to coarse subrounded gravel, occasional cobbles				
180	215		23	10							
175	220										
170	225		24	4							
165	230		25	50							
235											

Rev. 3 { Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT; O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
160	235	26		5							
155	240										
150	245	27		35		SP-SM	Dense, dark gray, moist to wet, slightly silty SAND (SP-SM), poorly-graded (Qpogf)				
145	250					CL	Hard, dark gray, moist, silty CLAY (CL), trace sand, medium plasticity, homogeneous (Qpogl)				Steel liners in core barrel. Scattered fume gravel dropstones
140	255	28		100							253 to 260 ft bgs, high angle fractures 50 to 70 degrees
135	260						Terminated boring at 260 feet below ground surface				
130	265										
270											

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-110

Sheet 1 of 13

Date(s) Drilled 3/28/03 - 4/4/03	Geotechnical Consultant Camp Dresser & McKee Inc.	Logged By RW	Checked By VJP 02-03-04
Drilling Method/Rig Type Wireline/ T3	Drilling Contractor Cascade Drilling, Inc.	Total Depth of Borehole 438.0 feet	
Casing Size/Type PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.) 300# / 30"	Ground Surface Elevation/Datum 444.7 feet / Metro	
Location 603 NE 204th St	Coordinates N 286964 E 1274158	Elevation Source Survey	

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											Vacuum out to 5.9 ft bgs, not sampled. Drive 7-inch casing to 8 feet.
440	5					SM	Very dense, brown, moist, silty, gravelly SAND (SM), poorly-graded coarse sand, poorly-graded fine to coarse angular gravel (Qvt)				Soil description partially inferred from drill action and cuttings
435	10										
430	15										
425	20	■	1	23 - 50/3" (100+)	100						
420	25										

Groundwater Observation Data:

OW (FT BGS):	109.4 (Low)	98.4 (High)
VWP1 (FT BGS):	144.8 (Low)	143.4 (High)
VWP2 (FT BGS):	170.1 (Low)	167.6 (High)

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
Recovery values > 100 indicate sample expansion during sampling.
Pocket Penetrometer shown as 4.6 indicates unconfined compressive strength > 4.5 tsf (penetrometer upper limit).

Rev. 3 (Ver. 1.1, Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
25											
415	30										
410	35					GW-GM	Very dense, brown, moist, slightly silty, sandy GRAVEL (GW-GM), well-graded fine to coarse sand, well-graded fine to coarse angular gravel (Qvtm)				Soil description partially inferred from drill action and cuttings Drillers note hard drilling
405	40	■	2	28 - 50/2" (100+)	63	SM	Silty, gravelly SAND (SM), fine to coarse sand, fine angular gravel (Qvtm)				Soil description partially inferred from drill action and cuttings
400	45										
395	50										
390	55										Irregular drilling resistance suggest gravel and cobbles
385	60	■	3	48 - 50/5" (100+)	44	SP	Very dense, brown, moist, SAND (SP), trace silt, poorly-graded fine to medium sand (Qva)		BG		

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-110

Sheet 3 of 13

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
60											
380	65										
375	70										
370	75					SP	Very dense, gray green, moist SAND (SP), trace silt, poorly-graded fine to medium sand, occasional organics (Qpfnf)				Drillers note interbedded gravel and cobbles
365	80	■	4	50/5" (100+)	79				BG		
360	85										
355	90										
350	95					CL	Very stiff to hard, gray green, moist, sandy silty CLAY (CL), trace fine gravel, low to medium plasticity, occasional organics (Qpfnl)				

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95											0 to 98' mud-rotary, switch to wire-line at 98', drilling from 95'
			6	26 - 50/5" (100+)	91		Transitions to gravelly, gravel and cobbles in clayey silt matrix, subrounded				
345	100		5		35						
						SM	Very dense, dark gray, moist, silty to very silty SAND (SM), trace gravel, scattered organics (Qpfnf)				
340	105					ML	Very stiff, dark gray SILT (ML), nonplastic, numerous organics (Qpfnl)				
						OL	Hard, brown, moist Organic SILT (OL) (Qpfnw)		AD MP		Conventional radiocarbon date >47,930 yrs B.P.
335	110		7		76	ML	Very stiff, gray olive, moist SILT (ML), trace sand, low plasticity, scattered organics (Qpfnl)				
						ML	Very stiff, olive gray, wet SILT (ML), rapid dilatancy, occasional organics, homogeneous (Qpfnl)				
330	115										
							Medium stiff SILT at 118 ft bgs				
325	120		8		94						
							Transitions to stiff, moist to wet				
320	125										
315	130										

Rev. 3 (Ver.1.1, Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
130					77						
						CL	Stiff, gray green, moist, silty CLAY (CL), medium plasticity, slickensides (Qpog1)		MP		
310	135						Transitions to gray				
305	140	10			100		Transitions to very stiff, gray green to green, silty clay				
300	145						Transitions to gray, sandy, silty clay				
295	150	11			70						
						SM	Very dense, gray green to gray olive, moist, silty, gravelly SAND (SM), fine to coarse sand, fine to coarse subrounded gravel, occasional organics (Qpogtm)				
290	155						Transitions to slightly silty and gravelly sand				
285	160	12			30						
						GW	Very dense, gray green, moist GRAVEL (GW), well-graded fine to coarse, subrounded gravel, layers of gravelly sand (Qpogf)				
280	165					GM	Very dense, gray green, moist, silty, sandy GRAVEL (GM), fine to coarse sand, fine to				

Rev. 3 (Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER, G.L.B.-BRIGHTWATER, G.D.T.) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
165							coarse gravel, subrounded (Qpogtm)				
275	170		13		50	SM	Very dense, gray green to olive gray, moist, slightly silty, gravelly SAND (SM), fine to coarse sand, fine to coarse subrounded gravel, strong cementation (Qpogtm)				
270	175					GW	Dense, gray green, moist GRAVEL (GW), well-graded fine to coarse, subrounded gravel (Qpogf)				
265	180		14		33		Hard, olive gray, moist, gravelly, sandy SILT (ML), nonplastic stratum				
260	185						Stratum of very stiff to hard, gray, moist, slightly clayey, slightly sandy silt, low plasticity, occasional organics, homogeneous, slow to rapid dilitancy				
255	190						Transitions to gravel and cobbles, subrounded				
250	195		16		6						
245	200										

Rev. 3 { Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT } O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
200						ML	Hard, dark gray, dry to moist, gravelly, sandy SILT (ML), medium plasticity, homogeneous (QpogI)				
		17		67			Transitions to very stiff, clayey silt, trace fine sand and fine gravel as dropstones, homogeneous				Sand-filled high angle fractures
240	205										
		18		111			Increasing clay, decreasing sand and gravel				209 to 216.5 ft bgs, slickensides
235	210										
		19		93							
230	215					SM	Dense, dark gray, wet, silty SAND (SM), rapid dilatancy (QpogI)		M AL		
		20		93					M SA		
225	220					ML	Hard, dark gray, dry to moist, slightly clayey, sandy SILT (ML), low plasticity, homogeneous (QpogI)				Slickensides at 45 degree angle
		21		100		CL	Very stiff, dark gray, moist, slightly silty CLAY (CL), low to medium plasticity, dropstones, slickensided, homogeneous (QpogI)				225 to 230 ft bgs, scattered slickensides
220	225					CH	Very stiff, dark gray, moist, slightly sandy CLAY (CH), high plasticity (QpogI)				
215	230								M AL		230 to 240 ft bgs, slickensides, shearing, fracturing
210	235										

Rev. 3 (Ver.1.1, Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235											
	240	22	115		CL	Very stiff, dark gray, moist slightly sandy CLAY (CL), medium plasticity (Qpog1)		M AL			
205						Layer of sand		M M AL SA HA M	4.6	240 to 259 ft bgs, scattered slickensides, high angle silt-filled fractures	
200	245	23	100			Decrease in clay content		M			
195	250							M			
190	255	24	106			Transitions to sandy and gravelly		M			
185	260				SM	Very dense, gray, dry to moist, very gravelly, silty, clayey SAND (SM), fine to coarse sand, fine subrounded gravel (Qpogd)		M DD UC M SA M SA		Sand-filled fracture at 20 degree angle	
180	265	25	83								
	270	26	100		CL	Hard, dark gray, moist, silty CLAY (CL), low to medium plasticity, scattered partings of fine sand				Scattered slickensides, high angle 70 degrees, block structure	

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-110

Sheet 9 of 13

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
270			27		83						
170	275		28		90	SM	Very dense, gray, moist, very gravelly, very silty SAND (SM), trace gravel, subrounded to rounded (Qpogd)				
165	280		15		1	CL	Hard, dark gray, moist, slightly sandy, silty CLAY (CL), low to medium plasticity, slickensided, homogeneous (Qpogm)				Scattered slickensides 20 to 40 degrees
160	285		29		102						
155	290		30		100				M AL		Bedding dip from horizontal approximately 25 degrees
150	295		31		100						
145	300										
140	305										

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
305											
135	310		32		100						Horizontal breaks along fine sand seam
130	315							M			
125	320						Interbeds of olive gray and gray green, fine sand from 317 to 320 ft bgs, occasional organics	MP			Paleosol
120	325		33		100		Shell fragments at 322 ft bgs	PA			Strong HCL reaction at 322 ft bgs
115	330										Organic odor
110	335		34		89		1-foot layer of sandy clay occasional organics and shell fragments	MP			
105	340					SP-SM	Dropstones (fine gravel)				Strong HCL reaction
							Very dense, dark gray, wet, slightly silty SAND (SP-SM), poorly-graded, occasional organics (Qpogf)				Strong organic odor
											Sheared sand layer; sand injection

Rev. 3 (Ver. 1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) 0:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-110

Sheet 11 of 13

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
340											
			35		92		Transitions to moist, silty SAND				
-100	345					ML	Hard, dark gray, moist, clayey, sandy SILT (ML), low plasticity, scattered dropstones, slickensided (Qpogf)				Blocky structure, numerous slickensides, high angle 50 to 70 degrees (oer consolidated) by movement?
							Layers of fine sand			3	
-95	350										
			36		70					2.5	Sheared sand seam at 50 degrees, sand injection
-90	355					SM	Very dense, dark gray, moist, silty, SAND (SM), fine sand, scattered organics (Qpogf)				
						ML	Hard, dark gray, moist to dry, clayey, slightly sandy SILT (ML), low to medium plasticity, dropstones, slickensided (Qpogf)				358 to 359 ft bgs, numerous slickensides, high angle 60 to 70 degrees
-85	360										
			37		79		Transitions to moist, fine sandy silt, medium plasticity, homogeneous				
-80	365					CL	Hard, dark gray, moist, sandy, silty CLAY (CL), trace gravel, medium plasticity, slickensided, homogeneous, occasional organics (Qpogf)				
-75	370										
			38		98						
						ML	Hard, dark gray, moist, sandy SILT (ML), fine sand (Qpogf)				
-70	375								BG		

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-110

Sheet 12 of 13

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
375						CL	Hard, dark gray, sandy, silty CLAY (CL), trace gravel, medium plasticity, slickensided, homogeneous, occasional organics (Qpfnl)				
65	380		39		100		Very fine sand layer				
60	385										
55	390		40		95		Layer of silty fine sand, trace gravel, occasional organics				
50	395										
45	400		41								
40	405						Sandy gravel layer				
35	410									4	High angle slickensides, near vertical

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-110

Sheet 13 of 13

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
410			42		50		Sandy gravel with partings and sand layers				
30	415										
25	420		43		95						
20	425						Transitions to hard sandy silty clay				
15	430		44		100		Sand partings				
10	435		45		100				MP		Piece of wood, occasional mussel Shells and shell fragments
5	440						Terminated boring at 438 feet bgs				
0	445										

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER.P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
Project Location: King & Snohomish Counties, Washington
Contract Number: E23007E

Log of Boring E-211

Sheet 1 of 9

Date(s) Drilled	6/9/03 - 6/12/03	Geotechnical Consultant	Camp Dresser & McKee Inc.	Logged By	SHE	Checked By	RWS 2/02/04
Drilling Method/Rig Type	Wireline/ Porta-drill	Drilling Contractor	Gregory Drilling, Inc.	Total Depth of Borehole	280.0 feet		
Casing Size/Type	PQ (7"O.D.)	Hammer Weight/Drop (lbs/in.)	300# / 30"	Ground Surface Elevation/Datum	413.4 feet / Metro		
Location	20059 Ballinger Way NE	Coordinates	N 286325 E 1277100	Elevation Source	Survey		

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
0											0 to 6 feet excavated with vacuum truck, not sampled
410	5					SM	Medium dense, brown, silty, slightly gravelly to gravelly SAND (SM), fine to medium (Qvrf)				Inferred from drill action and cuttings
405	10										
400	15										Some grinding and bucking while drilling (gravel, possible cobbles)
395	20		1	2 - 30 - 50/5" (100+)	41	SM	Very dense, yellow red, moist, silty, gravelly SAND (SM), non-plastic, fine to medium, gravel fine to coarse, subrounded (Qvt)				
390	25										

Groundwater Observation Data:

OW (FT BGS):	49.2 (Low)	30.0 (High)
VWP1 (FT BGS):	66.1 (Low)	60.5 (High)
VWP2 (FT BGS):		

Remarks: Negative Groundwater Data indicates measurements above Ground Surface
Recovery values > 100 indicate sample expansion during sampling.
Pocket Penetrometer shown as 4.6 indicates unconfined compressive strength > 4.5 tsf (penetrometer upper limit).

Rev. 3 [Ver. 1.1, Jan 02] BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT] O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
60		■	3	18 - 28 - 40 (68)	89	SP	Very dense, gray olive and gray blue, moist SAND (SP), trace silt, gravel, yellow orange laminae, organic material along scattered bedding planes (Qpfnf)				2 feet slough, washing hole; slough cleaned, smooth, even sample, bedding inclined 5 to 10 degrees
350	65										
345	70					CL-ML	Hard, dark brown, moist, clayey SILT CL-ML), numerous disseminated organics, trace fine sand in scattered layers, slightly plastic, slow dilatancy, mottled brown and green (Qpfnl)				Drilling quiet at 67 ft bgs Driller reports interbedded silty CLAY and SAND, 3-foot beds
340	75										
335	80	■	4	9 - 19 - 17 (36)	83						
330	85										
325	90										
320	95										

Rev. 3 (Ver. 1.1 Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-211

Sheet 4 of 9

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
95											
315											
100		■ ■ ■ ■ ■	5	6 - 9 - 13 (22)	100	CH	Very stiff, olive gray, moist, silty CLAY (CL) high plasticity, homogeneous, scattered bedding plane partings (Qpfnl)			4.5	Smooth, even drive Sheared zone, top of core
310											
105											
305											
110											
300											
115											
295											
120		■ ■ ■ ■ ■	6	7 - 13 - 19 (32)	89	ML	Hard, olive gray, wet, clayey SILT (ML), trace fine gravel, medium plasticity, slow dilatancy, occasional fine organic fragments (Qpfnl)				Smooth, even drive
290											
125											
285											
130											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-211

Sheet 5 of 9

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
130											
280											
135											
275											
140			7	10 - 14 - 22 (36)	100						Even drive
270											
145											
265											
150						SM	Very dense, olive gray, very silty, gravelly SAND (SM), cobbles, gravel rounded to subangular, sand fine grained, poorly graded, clayey slough (Qpogt)				Driller reported change at 150 ft bgs, drill chattering and bucking
260											
155											
255											
160			8		13						
250											
165											Drilled out - button bit, 164 to 170 ft bgs

Rev. 3 (Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER, GLB-BRIGHTWATER, GDT); O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
165											
245											
170			9		17						
240											
175							Scattered gravel layers, approximately 6 inches to 1 foot thick				Pressure meter test 175 to 180 ft bgs Slow drilling
235							6-inch gravelly beds, with 6-inch sand/silt layers				
180			10		17						
230							Silt content decreases				
185											
225			11		54				M SA	4	
190											
220						CL	Hard, olive gray, moist, silty CLAY (CL), trace gravel and sand dropstones, medium plasticity, no dilatancy, laminae of light gray silt (QpogI)		M SA		Frequent slickensides and fractures, becciated texture
195											
215			12		91		6-inch silt layer		M AL	4.6	
200											

Rev. 3 (Ver.1.1 Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P.19.GPJ 2/4/04



Elevation, feet	Depth, feet	SAMPLES			USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)						
200										
					High plasticity		SA HA			
210										
205										
		13		41						
205										
					Laminated silt layer from 210 to 211 ft bgs		M AL			
210										
200										
215										
		14		70						
195										
					Gravelly from 219 to 220 ft bgs					
220										
		15		33						
190										
225					Scattered shiny black and small white grains (glass and ash?), volcanics and quartz					Rotary 220.5 to 225 ft bgs Drilled out to clean borehole for in situ test, drilling rocky at top, less gravel and cobbles toward 225 ft bgs
185										
										Difficult drilling - formation choking off circulation - mainly clay and silt, few gravel Pressure meter test 225 to 230 ft bgs
230										
180										
					Gravelly from 232 to 234 ft bgs					
235										

Rev. 3 {Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER.GLB, BRIGHTWATER.GDT, O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ, 2/4/04}



Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type	Number	Blows / 6 in. (N)	Recovery, %						
235	16				33						
175	240		17		62		Slightly gravelly				
170	245						Cobbles				
165	250						Scattered slickensides, scattered light gray partings				Bedding inclined to 5 degrees
160	255		18		22						
155	260										Sheared and brecciated
150	265		19		72						
145							Cobbles				Pressure meter test 269 to 280 ft bgs
270											

Rev. 3 (Ver. 1.1, Jan 02 BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Project: King County WTD / Brightwater Conveyance System
 Project Location: King & Snohomish Counties, Washington
 Contract Number: E23007E

Log of Boring E-211

Sheet 9 of 9

Elevation, feet	Depth, feet	SAMPLES				USCS	MATERIAL DESCRIPTION	Piezometer Schematic	Lab Tests	Pocket Penetrometer (tsf)	REMARKS AND OTHER TESTS
		Type Number	Blows / 6 in. (N)	Recovery, %	Graphic Log						
270					[Hatched Box]						
140											
275											
135											
280							Terminated boring at 280 feet below ground surface				
130											
285											
125											
290											
120											
295											
115											
300											
110											
305											

Rev. 3 (Ver.1.1, Jan02BRIGHTWATER-BRIGHTWATER.GLB-BRIGHTWATER.GDT) O:\GINT\PROJECTS\BRIGHTWATER P19.GPJ 2/4/04



Drilled	Start 7/7/2014	End	Total Depth (ft)	80.4	Logged By Checked By	DTM CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger	
Surface Elevation (ft) Vertical Datum	329 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig		
Easting (X) Northing (Y)	1375549.288 386456.8713			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)	
Notes: Autohammer efficiency = 87% (measured 11/1/2013)											

Elevation (feet)	FIELD DATA						Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level					
0							SM	Brown silty fine to medium sand with gravel and occasional cobbles and debris (loose, moist) (fill) (ESU 1A)			Water knife/vector to 5 feet. Soil description based on visual observation.
5	12	17		1 MC			SM	Brown silty fine to medium sand (medium dense, moist) (glacial till) (ESU 5A)	12		
10	13	50		2 SA			SM	Brownish gray silty fine to medium sand with gravel and occasional cobbles and boulders (very dense, moist) (ESU 5B)	9	29	
15	0	50/2"									No recovery
20	0	50/2"									Switched to mud rotary No recovery
25	12	78		3 SA			SM	Gray silty fine to medium sand with gravel (very dense, moist)	9	28	
30	12	66		4 %F					10	25	
35	10	78		5 SA					10	25	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B06



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Figure A-54
 Sheet 1 of 3

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082026\GINT\4082026\GINT\4082026_01-C10.GPJ DBT\template\lib\template.GEOTECH_STANDARD

Elevation (feet)	FIELD DATA					Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing							
35		10	59		6 MC				10			
290							SM	Gray silty fine to medium sand with gravel (dense, moist)				
40		13	48		7 SA				10	21		
285							SM	Brown silty fine to medium sand with occasional gravel (dense, moist)				
45		11	35		8 MC				10			
280							SP-SM	Brownish gray with oxidation staining fine to medium sand with silt (dense, wet) (advance outwash) (ESU 6B)				
50		14	37		9 SA				22	12	Groundwater observed at 50 feet during drilling	
275							SM	Brownish gray silty fine to medium sand (very dense, wet)				
55		17	61		10 %F				22	13		
270							SP-SM	Brownish gray fine to medium sand with silt (very dense, wet)				
60		18	64		11 %F				22	10		
265												
65		10	50/6"		12 MC				22		Rough drilling at 66 feet	
260							SM	Brownish gray silty fine sand (very dense, wet)				
70		16	69		13 SA				24	13		
255							SP-SM	Gray fine sand with silt (very dense, wet)				
75		10	50/6"		14 %F				24	12		

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\26\202_B01-C10.GPJ DBTemplate\lib\template.GEOENGINEERS.GDT\GEIR_GEO TECH_STANDARD

Log of Boring LLE-B06 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBTTemplate\LibTemplate.GEOENGINEERS.GDT\GEIR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
80	5	50/5"		15 MC			SM	Gray silty fine sand (very dense, wet)	21	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B06 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Drilled	Start 7/11/2014	End 7/11/2014	Total Depth (ft)	81.5	Logged By Checked By	ERH CEW	Driller	Holocene	Drilling Method	Hollow-Stem Auger
Surface Elevation (ft) Vertical Datum	317 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Diedrich D-120 Truck Rig	
Easting (X) Northing (Y)	1375800.355 387658.5549			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)
Notes: Autohammer efficiency = 70% (measured 9/30/2013)										

Elevation (feet)	FIELD DATA						Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing								
315	0							SOD	3 inches sod			Water knife/vector to 5 feet. Soil description based on visual observation.	
	5	14	28		1 SA			SM	Brown silty fine to medium sand with gravel, occasional cobbles and boulders (medium dense, moist) (fill) (ESU 1B)				
310								SP-SM	Gray fine to coarse sand with silt and gravel (dense, moist)	8	20		
	10	15	35		2 MC			SM	Gray silty fine to medium sand with occasional gravel (dense to very dense, moist) (glacial till) (ESU 5B)	5		Rough drilling	
305								SM	Gray silty fine to medium sand with occasional gravel (dense to very dense, moist) (glacial till) (ESU 5B)	8	26		
300	15	18	36		3 %F			SM	Grades to with gravel	8	25		
295								SM	Grades to brown	8			
290	20	18	46		4 SA			SM	Grayish brown silty fine to medium sand (medium dense, moist)	7	27	Rough drilling	
	25	18	57		5 MC			SM	Gray silty fine to medium sand with occasional gravel and trace organics (dense, wet)				
285	30	18	27		6 SA			SM					
	35												

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B08



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-55
 Sheet 1 of 3

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBTemplate\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
80	18	68		15 AL			ML	Gray silt with sand (hard, wet)	25	AL (non-plastic)

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B08 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Elevation (feet)	FIELD DATA						Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing								
295	35	4	50/4"		5 SA					14	11		
290	40	17	85/11"		6 SA				Grades to with occasional gravel	22	9		
285	45	12	50/6"		7 %F			SP-SM	Brown fine to medium sand with silt (very dense, wet)	22	9		
280	50	12	50/6"		8 SA			SM	Brown silty fine to medium sand with occasional gravel and silt interbeds (very dense, wet)	19	17		
275	55	12	50/6"		9 %F					20	20		
270	60	6	50/6"		10 SA					12	28		
265	65	6	50/6"		11 MC					11			
260	70	0	50/3"									No recovery	
255	75	18	40		12 AL			CH	Gray sandy fat clay (hard, moist) (transitional beds) (ESU 7)	29		AL (LL = 63%; PI = 34%)	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B09 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Figure A-56
 Sheet 2 of 3

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\26\B01-C10.GPJ_DBT\template\lib\template\GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Redmond: Date: 9/23/14 Path: \\RED\PROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBT\template\lib\template.GEOENGINEERS.GDT\GEBR_GEO TECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
80	18	83/11"		13 MC			ML	Gray sandy silt (hard, wet)	26	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B09 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Drilled	Start 6/19/2014	End 6/19/2014	Total Depth (ft)	81.5	Logged By Checked By	DTM CRW	Driller	Holocene	Drilling Method	Mud Rotary
Surface Elevation (ft) Vertical Datum	354 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig	
Easting (X) Northing (Y)	1376055.124 388777.7713			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013) 3 (in) solid well installed at 80 (ft), decommissioned after seismic testing.										

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS										
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level					Graphic Log	Group Classification								
0							AC			3 inches asphalt concrete										
							SM			Grayish brown silty fine to medium sand with occasional gravel and cobbles (dense to very dense, moist) (glacial till) (ESU 5B)										Water knife/vector to 5 feet. Soil description based on visual observation.
350																				
	5	8	50/2"		1 SA							13	28							
345																				
	10	1	50/3"		2 MC							9								
340																				
	15	1	50/3"		3 MC					Grades to moist to wet		8								
335																				
	20	8	50/2"		4 SA		SM			Grayish brown silty fine to medium sand with occasional gravel (very dense, moist)		12	43							
330																				
	25	3	50/3"		5 MC					Grades to moist to wet		9								
325																				
	30	1	50/4"		6															Poor recovery
320																				
	35						SP-SM			Grayish brown silty fine to medium sand with gravel (very dense, moist to wet) (advance outwash) (ESU 6B)										

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B10S



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-57
 Sheet 1 of 3

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\44082026\GINT\4082026\B01-C10.GPJ_DBT\template\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA					Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing							
35	6	50/4"	7	%F					8	7		
315							SM	Grayish brown silty fine to medium sand with gravel (very dense, moist to wet)				
40	2	50/5"	8	MC					11			
310												
45	2	50/3"	9									
305												
50	4	50/4"	10	%F					14	36		
300							CL	Grayish brown lean clay with sand (hard, moist)				
55	9	50/3"	11	AL					22		AL (LL = 27%; PI = 8%)	
295							SM	Brown silty fine to medium sand (very dense, moist)				
60	12	88/9"	12	SA					16	22		
290												
65	0	50/5"									No recovery	
285							SM	Gray silty fine to medium sand with silt interbeds (very dense, wet)				
70	16	82	13	SA					18	12	Groundwater observed at 70 feet during drilling	
280												
75	18	83	14	%F					24	19		

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B10S (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-57
 Sheet 2 of 3

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBTemplate\lib\template.GEOENGINEERS.GDT\GEIR_GEO TECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS		
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level					Graphic Log	Group Classification
75	80	17	93/11"		15 SA			SM	Gray silty fine sand (very dense, wet)	21	24	

Note: See Figure A-0 for explanation of symbols.

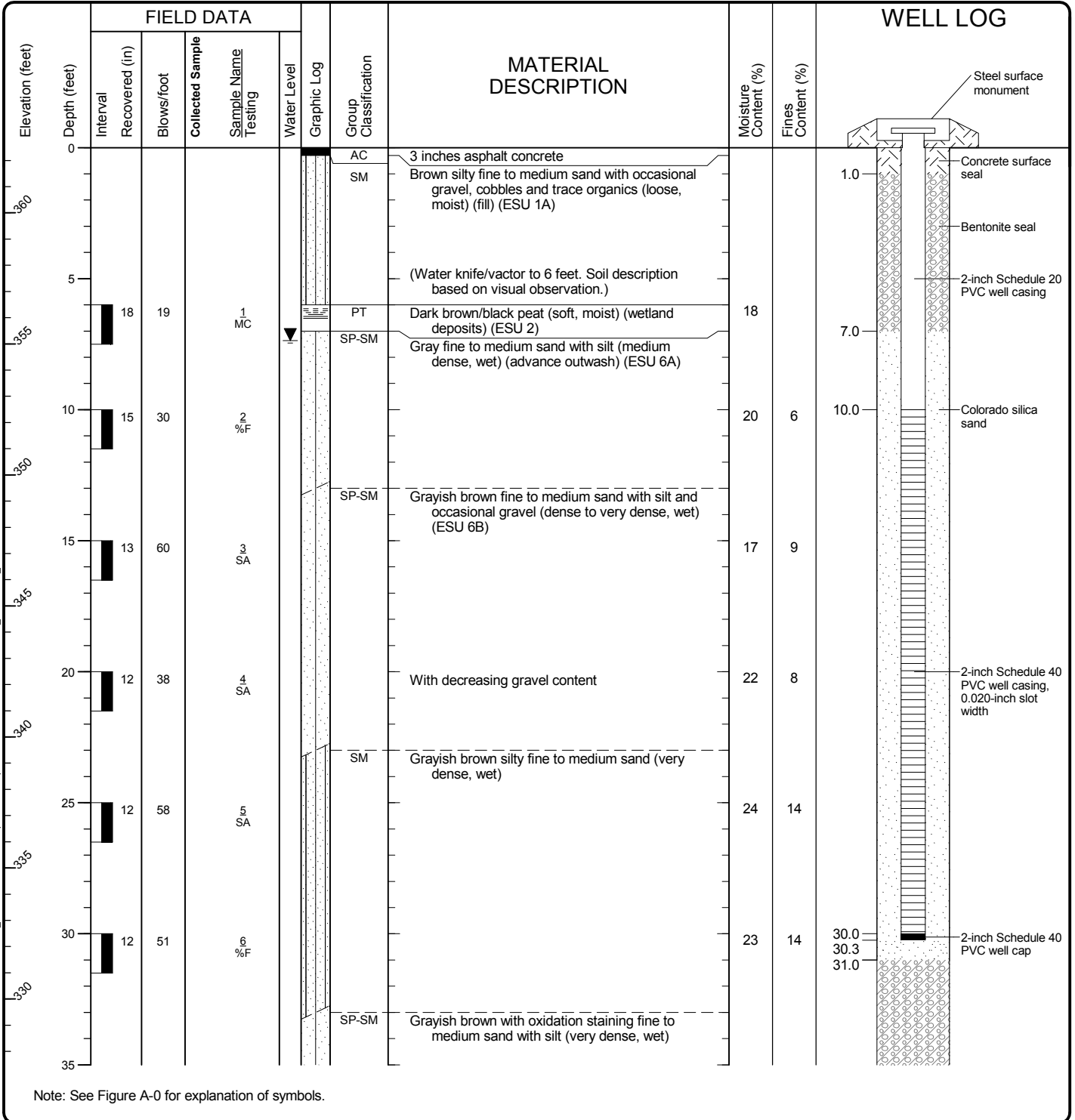
Log of Boring LLE-B10S (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-57
 Sheet 3 of 3

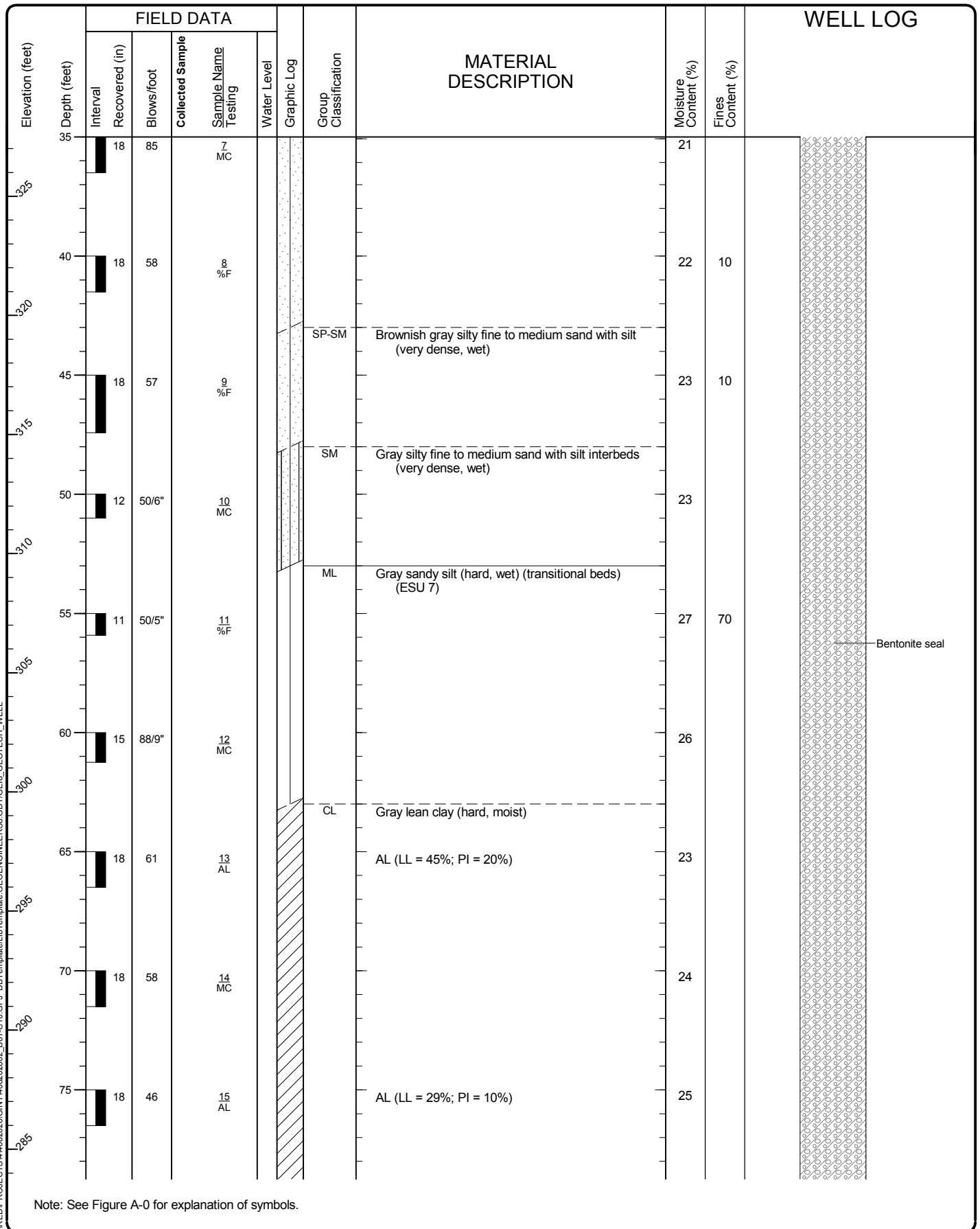
Drilled	Start 6/20/2014	End 6/20/2014	Total Depth (ft)	81.5	Logged By Checked By	DTM CRW	Driller	Holocene	Drilling Method	Mud Rotary
Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment			Mobile B-61 Truck Rig		DOE Well I.D.: BIP344 A 2 (in) well was installed on 6/20/2014 to a depth of 30.25 (ft).	
Surface Elevation (ft) Vertical Datum		362.49 Project			Top of Casing Elevation (ft)		362.26			
Easting (X) Northing (Y)		1376334.3 389321.79			Horizontal Datum		Project			
		Groundwater Date Measured		8/12/2014		Depth to Water (ft)		7.4		Elevation (ft) 354.9
Notes: Autohammer efficiency = 87% (measured 11/1/2013)										



Log of Boring LLE-B11P



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02



Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B11P (continued)





Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Figure A-58
Sheet 2 of 3

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\26\202_B01-C10.GPJ_DBT\template\lib\template.GEOENGINEERS.GDT\GEIR_GEOTECH_WELL

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\GINT\4082\26\B01-C\10.GPJ DBTTemplate\lib\template.GEOENGINEERS.GDT\GEIR_GEOTECH_WELL

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	WELL LOG
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
80	15	59		16 MC				27		

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B11P (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Drilled	Start 7/21/2014	End 7/21/2014	Total Depth (ft)	81	Logged By Checked By	ERH CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger
Surface Elevation (ft) Vertical Datum	394 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig	
Easting (X) Northing (Y)	1376263.904 389682.3318			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013)								7/21/2014	38.2	355.85

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
0						AC	7 inches asphalt concrete			Water knife/vector to 5.5 feet. Soil description based on visual observation.
						CR	2 inches base course			
390						SP-SM	Brown fine sand with silt, gravel and occasional cobbles (medium dense, moist) (fill) (ESU 1B)			
5	15	10		1 SA		SP-SM	Brown with oxidation staining fine to medium sand with silt (medium dense, moist)	11	12	
385						SP-SM	Brown fine to medium sand with silt and occasional gravel (medium dense, moist)			
10	13	30		2 %F				6	8	Hard drilling
380										No recovery
15	0	48								
375						SP-SM	Gray fine to medium sand with silt and occasional gravel (medium dense, moist)			
20	18	21		3 MC				9		
370						SM	Gray silty fine to medium sand with occasional gravel and lenses of wood/peat and silt (dense, moist) (advance outwash) (ESU 6A)			
25	13	33		4						
365										
30	18	31		5 SA				8	13	
360						SM	Brown with oxidation staining silty fine to medium sand with occasional gravel and silt interbeds (dense, moist) (ESU 6B)			
35										

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: \\REDIP\PROJECTS\44082026\GINT\4082026\B01-C10.GPJ_DBT\template\lib\template\GEOENGINEERS.GDT\GEBR_GEO TECH_STANDARD

Log of Boring LLE-B12



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Elevation (feet)	FIELD DATA						Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing								
35		18	35		6 %F					17	14		
355								SP	Brown fine to medium sand with trace silt and occasional gravel (dense, wet)			Groundwater observed at 38 feet during drilling	
40		18	42		7 SA					18	4	Driller added mud to control heave	
350								SP-SM	Brown fine to medium sand with silt (dense to very dense, wet)				
45		16	55		8 %F					20	8		
345													
50		17	47		9 SA					19	10		
340													
55		14	75		10 %F					21	7		
335								SP-SM	Brown with oxidation staining fine to medium sand with silt and silt interbeds (very dense, wet)				
60		18	53		11 SA					23	9		
330													
65		1	64		12							Poor recovery	
325													
70		12	74		13 MC					18			
320								SP-SM	Brown with oxidation staining fine to medium sand with silt (very dense, wet)				
75		18	67		14 SA					21	10		

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\26\202_B01-C10.GPJ DBT\template\lib\template.GEOENGINEERS.GDT\GEIR_GEO TECH_STANDARD

Log of Boring LLE-B12 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Redmond: Date: 9/23/14 Path: \\RED\PROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ_DBT\template\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
37.5	80	11	50/5"		15 %F					

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B12 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Elevation (feet)	FIELD DATA						Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing								
35		18	48		7 %F					13	6	Driller added mud to auger to prevent heave	
355		18	44		8 %F					17	5		
350		18	40		9 SA			SP-SM	Gray fine to medium sand with silt (dense to very dense, wet)	20	8		
345		18	55		10 MC					22			
340		18	48		11 %F					22	6		
335		18	64		12 SA			SM	Gray silty fine to medium sand (very dense, wet)	23	12		
330		18	72		13 %F					26	15		
325		18	67		14 %F			SP-SM	Brown with oxidation staining fine to medium sand with silt (very dense, wet)	23	8		
320		18	80		15 MC					23			

Note: See Figure A-0 for explanation of symbols.


Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBT\template\lib\template.GEOENGINEERS.GDT\GEIR_GEO TECH_STANDARD

Log of Boring LLE-B13 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Redmond: Date: 9/23/14 Path: \\RED\PROJECTS\4082\26\GINT\4082\02\02_B01-C\10.GPJ DBTemplate\lib\template.GEOENGINEERS.GDT\GEIR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS		
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level					Graphic Log	Group Classification
315	80	18	77		16 %F			SM	Gray silty fine sand with sand interbeds (very dense, wet)	26	18	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B13 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-60
 Sheet 3 of 3

Drilled	Start 6/28/2014	End 6/28/2014	Total Depth (ft)	41	Logged By Checked By	DTM CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger
Surface Elevation (ft) Vertical Datum	398 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig	
Easting (X) Northing (Y)	1376477.14 389951.8931			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013)										

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
0						AC	4 inches asphalt concrete			Water knife/vector to 5.5 feet. Soil description based on visual observation.
						CR	8 inches base course			
						CC	3 inches concrete			
395						SP-SM	Brown fine to coarse sand with gravel (medium dense, moist) (fill) (ESU 1A)			
5										
390		12	30			SP-SM	Brownish gray with oxidation staining fine to medium sand with silt and occasional gravel (medium dense to dense, moist) (advance outwash) (ESU 6A)	9		
10		12	20	1	MC			9	10	
385										
15		18	41	3	SA			7	7	
380										
20		18	54	4	SA			8	12	
375										
25		18	45	5						
370										
30		18	47	6	%F			5	7	
365										
35										

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B14



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-61
 Sheet 1 of 2

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\026\B01-C10.GPJ DBT\template\lib\template\GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBTemplate\LibTemplate.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS			
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level					Graphic Log	Group Classification	
35		18	71		Z %F					Grades to wet	21	6	Groundwater observed at 35 feet during drilling
40		12	50/6"		SA				SP	Brownish gray fine to medium sand with trace silt (very dense, wet)	30	3	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B14 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Drilled	Start 6/28/2014	End 6/28/2014	Total Depth (ft)	41.5	Logged By Checked By	DTM CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger
Surface Elevation (ft) Vertical Datum	400 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig	
Easting (X) Northing (Y)	1376476.393 390195.1061			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013)										

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
0							AC			Water knife/vector to 5.5 feet. Soil description based on visual observation.
							CR			
							SP-SM			
5										
							SP-SM			
10		18	56		1 MC			7		
15		18	29		2 %F			7	8	
							(ESU 6B)			
20		18	61		3 SA			4	9	
							SP			
25		12	50/6"		4 %F			3	4	
							SW-SM			
30		18	65		5 SA			4	8	
							SW-SM			
35		18	64		6					
							SW-SM			

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B15



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Redmond: Date: 11/01/14 Path: \\RED\PROJECTS\4402026\GINT\4082026\B01-C10.GPJ DBT\template\Lt\template\GEOENGINEERS8.GDT\GEB_GEOTECH_STANDARD

Ref: 11/01/14 Path: \\RED\PROJECTS\4482026\GINT\4082026_01-C10.GPJ DBT\template\Lb\template\GEOENGINEERS8.GDT\GEB_GEO TECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Interval	Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
35	18	76		SA			12	7	Groundwater observed at 35 feet during drilling	
360										
40	14	81/8"		%F		SP	16	4	Brownish gray fine to medium sand with occasional gravel and trace silt (very dense, wet)	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B15 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Elevation (feet)	FIELD DATA					Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing							
35		18	47		7 %F					15	8	
44.5							SP	Brown fine to medium sand (very dense, wet)				
40		15	70/9"		8 SA					16	1	
44.0							SP-SM	Gray fine to medium sand with silt (very dense, wet)				
45		14	52		9 SA					21	5	
43.5							SP-SM	Brown fine to medium sand with silt (dense to very dense, wet)				
50		14	53		10 MC					23		
43.0												
55		18	48		11 %F					23	11	
42.5							SP-SM	Brown fine sand with silt (dense to very dense, wet)				
60		14	40		12 SA					23	9	
42.0												
65		16	58		13							
41.5							SM	Brown silty fine sand (very dense, wet)				
70		18	85		14							
41.0							CL	Brown with oxidation staining lean clay (hard, moist to wet)				
							SP-SM	Gray fine sand with silt (very dense, moist)				
75		11	50/5"		15 %F					24	9	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B17 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Figure A-63
 Sheet 2 of 3

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
405	80	18	74/11"		16 MC		ML	Gray sandy silt (hard, moist) (transitional beds) (ESU 7)	27	
400	85	18	45		17 %F		ML	Gray silt with sand (hard, moist)	25	89
395	90	18	60		18 AL		CL	Gray lean clay (hard, moist)	32	
390	95	18	47		19 MC				33	
385	100	18	25		20 AL		CH	Tan fat clay (very stiff, moist)	46	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B17 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Shoreline, Washington
 Project Number: 4082-026-02

Figure A-63
 Sheet 3 of 3

Drilled	Start 4/14/2014	End 4/14/2014	Total Depth (ft)	61	Logged By Checked By	CRW CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger	
Surface Elevation (ft) Vertical Datum	390 Project			Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop			Drilling Equipment	Mobile B-61 Truck Rig		
Easting (X) Northing (Y)	1376060.17 390957.3037			System Datum	Project			Groundwater Date Measured	Depth to Water (ft)	Elevation (ft)	
Notes: Autohammer efficiency = 87% (measured 11/1/2013)											

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
0						AC	3 inches asphalt			Water knife/vector to 5.5 feet. Soil description based on visual observation.
						SM	Reddish brown silty sand with gravel and cobbles (medium dense, moist) (fill) (ESU 1A)			
385	5	10	17	1		SP-SM	Brown fine to coarse sand with silt and occasional gravel (medium dense, moist) (advance outwash) (ESU 6A)	7		
380	10	18	25	2			2 inch silt lens	13		
375	15	18	38	3		SP-SM	Gray fine to medium sand with silt (dense to very dense, moist) (ESU 6B)	9	10	
370	20	18	70	4			With occasional gravel	7	9	
365	25	18	68	5				8	10	
360	30	12	50/6"	6				5		
355	35					SP-SM	Grayish brown fine sand with silt (very dense, moist)			

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B18



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-64
 Sheet 1 of 2

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082\26\GINT\4082\026\B01-C10.GPJ_DBT\template\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
35		15	85/11"		Z SA			8	10	
350		18	85		8 MC			4		With occasional gravel
345		10	50/6"		9 SA			7	9	Gravel zone at 45 feet
340		16	82		10 %F			4	8	
335		12	50/6"		11		SP-SM			Gray fine sand with silt (micaceous) (very dense, moist)
330		10	50/6"		12 SA			6	9	

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B18 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Refmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082026\GINT\408202602_B01-C\10.GPJ DBTTemplate\libTemplate.GEOENGINEERS.GDT\GEBR_GEO TECH_STANDARD

Start Drilled 7/23/2014	End 7/24/2014	Total Depth (ft) 101.5	Logged By Checked By CRW CRW	Driller Holocene	Drilling Method Hollow-Stem Auger
Surface Elevation (ft) Vertical Datum	392 Project	Hammer Data	Autohammer 140 (lbs) / 30 (in) Drop	Drilling Equipment	Mobile B-61 Truck Rig
Easting (X) Northing (Y)	1375821.77 391597.49	System Datum	Project	Groundwater Date Measured	Depth to Water (ft) Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013)					

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
0						SOD	4 inches sod			Water knife/vector to 5 feet. Soil description based on visual observation.
5	18	22		1 %F		SP-SM	Brown fine to medium sand with silt and occasional gravel and cobbles and trace organics (loose, moist) (fill) (ESU 1A)	8	7	
10	18	33		2 SA		SP-SM	Brown fine to medium sand with silt (medium dense to very dense, moist) (advance outwash) (ESU 6A)	7	9	
15	18	52		3 %F			Grades to with occasional gravel	6	9	
20	18	71		4 %F			With decreasing gravel content, grades to very dense (ESU 6B)	6	11	
25	18	56		5 %F			Grades to wet	9	7	
30	16	75/10"		6 SA		SP-SM	Brown fine to medium sand with silt and occasional gravel (very dense, wet)	15	6	6 inches of heave
35						SW-SM	Brown fine to medium sand with silt and gravel (very dense, wet)			

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: I:\REDIP\PROJECTS\4082-026\GINT\40820202_B01-C10.GPJ DBT Template\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Log of Boring LLE-B19



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-65
 Sheet 1 of 3

Elevation (feet)	FIELD DATA					Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing							
35		15	76		Z SA				13	8	Driller added mud to auger to control heave	
35.5							SP-SM	Brown fine to medium sand with silt (dense, wet)				
40		18	43		8 %F				21	7		
350												
45		18	43		9 SA				25	11		
345												
50		18	63		10 MC			Grades to grayish brown	21			
340												
55		18	77		11 %F		SM	Grayish brown silty fine to medium sand (very dense, wet)	20	12		
335												
60		12	50/6"		12 %F		SP-SM	Brownish gray fine to medium sand with silt (very dense, wet)	23	9		
330												
65		6	50/6"		13 SA				22	8		
325												
70		17	77/5"		14 %F		SP-SM	Gray fine to medium sand with silt (very dense, wet)	21	7		
320												
75		18	55		15 AL		CH	Gray fat clay (hard, moist) (transitional beds) (ESU 7)	26		AL (LL = 53%; PI = 28%)	

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\20202_B01-C\10.GPJ_DBT\template\lib\template.GEOENGINEERS.GDT\GEIR_GEO TECH_STANDARD

Log of Boring LLE-B19 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
310	80	18	31		16 MC			24		
305	85	18	56		17 MC		CL	22		Gray with oxidation staining lean clay with sand interbeds (hard, moist)
300	90	18	34		18 AL		MH	40		Brown with oxidation staining elastic silt (hard, moist)
295	95	18	29		19 MC		CH	32		Brownish gray fat clay (hard, moist)
	100	18	37		20 AL			32		AL (LL = 59%; PI = 30%)

Note: See Figure A-0 for explanation of symbols.

Redmond: Date: 9/23/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\26\B01-C\10.GPJ DBTTemplate\lib\template.GEOENGINEERS.GDT\GEBR_GEO TECH_STANDARD

Log of Boring LLE-B19 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Drilled	Start 4/18/2014	End 4/18/2014	Total Depth (ft)	41.5	Logged By Checked By	ERH CRW	Driller	Holocene	Drilling Method	Hollow-Stem Auger	
Surface Elevation (ft) Vertical Datum			432 Project		Hammer Data		Autohammer 140 (lbs) / 30 (in) Drop		Drilling Equipment		Mobile B-61 Truck Rig
Easting (X) Northing (Y)			1375779.837 392394.9864		System Datum		Project		Groundwater Date Measured		Depth to Water (ft) Elevation (ft)
Notes: Autohammer efficiency = 87% (measured 11/1/2013)											

Elevation (feet)	FIELD DATA						Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing								
0								AC	6 inches asphalt concrete			Water knife/vactor to 5.5 feet. Soil description based on visual observation.	
								GP	3 inches gravel base course				
430								SM	Brown silty sand with gravel and occasional boulders (medium dense, moist) (fill) (ESU 1B)				
5		15	28		1 MC			SP-SM	Brown fine to medium sand with silt and gravel (medium dense, moist)	6			
425													
10		18	25		2 MC			ML	Gray silt with occasional sand and gravel (very stiff, moist)	9			
420													
15		18	30		3 SA			SM	Brown silty fine to medium sand with gravel (medium dense to dense, moist)	7	19		
415													
20		14	25		4 %F			SM	Brown silty fine to medium sand with gravel (medium dense to dense, moist)	8	33		
410													
25		18	31		5 SA					10	36		
405													
30		18	23		6 SA			SP-SM	Gray fine to medium sand with silt and occasional gravel (medium dense, moist)	9	28		
400													
35													

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B20



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Figure A-66
 Sheet 1 of 2

Redmond: Date: 9/25/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\026\B01-C10.GPJ_DBTTemplate\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Redmond: Date: 9/25/14 Path: I:\REDIPROJECTS\4082\26\GINT\4082\2602_B01-C\10.GPJ DBTemplate\lib\template.GEOENGINEERS.GDT\GEBR_GEOTECH_STANDARD

Elevation (feet)	FIELD DATA						MATERIAL DESCRIPTION	Moisture Content (%)	Fines Content (%)	REMARKS
	Interval	Recovered (in)	Blows/foot	Collected Sample	Sample Name Testing	Water Level				
35	10	11		Z MC		WOOD	2 to 3 inch wood layer	11		
						SP-SM	Brown with oxidation staining fine to medium sand with silt and gravel (medium dense, moist) (advance outwash) (ESU 6A)			
						SM	Brown silty fine to medium sand (dense, moist)			
40	18	34		8						

Note: See Figure A-0 for explanation of symbols.

Log of Boring LLE-B20 (continued)



Project: Sound Transit - Lynnwood Link Extension
 Project Location: Mountlake Terrace, Washington
 Project Number: 4082-026-02

Appendix C

Borehole Data Summary Sheet

Appendix C, Borehole Data Summary Sheet. Expansions of the abbreviations listed in this table are as follows – Drilling Methods: Hollow Stem Auger (HAS), Mud Rotary (MR), and Becher Hammer (BH); Samplers: Standard Penetration Test (SPT), and Dames and Moore (D&M); Methods of Water Measurement: Vibrating Wire Piezometer (VWP), Observations while Drilling (drilling obs.), and electric tape (e-tape).

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines	
B06	MR	SPT					279	drilling obs. 7/07/2014					
			50	279	37	dense			22.0	0.0	88.5	11.5	
			55	274	61	very dense			22.0			13.0	
			60	269	64	very dense			22.0			10.0	
			65	264	100	very dense			22.0				
			70	259	69	very dense			24.0	0.0	86.6	13.4	
			75	254	100	very dense			24.0			12.0	
			80	249	100	very dense			21.0				
B08	HSA	SPT					281	drilling obs. 7/11/2014					
			40	277	50	very dense			15.0	14.8	81.0	4.3	
			45	272	56	very dense			21.0				
			50	267	60	very dense			23.0			15.0	
			55	262	100	very dense			23.0	3.8	82.0	14.1	
B09	HSA	SPT					312	drilling obs. 7/17/2014					
			25	306	100	very dense			16.0	11.7	95.6	2.7	
			35	296	100	very dense			14.0	22.4	67.0	10.7	
			40	291	100	very dense			22.0	1.1	89.8	9.1	
			45	286	100	very dense			22.0			9.0	
			50	281	100	very dense			19.0	11.7	71.6	16.7	
			55	276	100	very dense			20.0			20.0	
			60	271	100	very dense			12.0			28.0	
			65	266	100	very dense			11.0				
			70	261	100	very dense							
B10S	MR	SPT					284	drilling obs. 6/19/2014					
			35	319	100	very dense			8.0			7.0	
			36	318	100	very dense			11.0				
			37	317	100	very dense							
			40	315	100	very dense			14.0			36.0	

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
B11P	MR	SPT					354.9	e-tape 8/12/2014				
			10	352.5	30	dense			20.0			6.0
			15	347.5	60	very dense			17.0			9.0
			20	342.5	38	dense			22.0			8.0
			25	337.5	58	very dense			24.0			14.0
			30	332.5	51	very dense			23.0			14.0
			35	327.5	85	very dense			21.0			
			40	322.5	58	very dense			22.0			10.0
			45	317.5	57	very dense			23.0			10.0
			50	312.5	100	very dense			23.0			
B12	HSA	SPT					355.85	e-tape 7/12/2014				
			25	369	33	dense						
			30	364	31	dense			8.0	5.4	81.8	12.9
			35	359	35	dense			17.0			14.0
			40	354	42	dense			18.0	6.4	89.7	3.9
			45	349	55	very dense			20.0			8.0
			50	344	47	dense			19.0	3.6	86.2	10.1
			55	339	75	very dense			21.0			7.0
			60	334	53	very dense			23.0	0.8	90.6	8.6
			65	329	64	very dense						
			70	324	74	very dense			18.0			
			75	319	67	very dense			21.0	0.0	90.1	9.9
			80	314	100	very dense			22.0			12.0
B13	HSA	SPT					363	drilling obs. 6/21/2014				
			25	370	22	med dense			7.0	1.5	94.5	4.0
			30	365	23	med dense			16.0			8.0
			35	360	48	dense			13.0	18.5	75.4	6.1
			40	355	44	dense			17.0			5.0
			45	350	40	dense			20.0	0.2	91.4	8.4
			50	345	55	very dense			22.0			
			55	340	48	dense			22.0			6.0
			60	335	64	very dense			23.0	3.3	84.5	12.2
			65	330	72	very dense			26.0			15.0
			70	325	67	very dense			23.0			8.0
			75	320	80	very dense			23.0			
			80	315	77	very dense			26.0			18.0

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
B14	HSA	SPT					363	drilling obs. 6/28/2014				
			12.5	391.5	30	dense			9.0			
			10	388	20	med dense			9.0			10.0
			15	383	41	dense			7.0	0.7	92.0	7.3
			20	378	54	very dense			8.0	1.0	87.3	11.7
			25	373	45	dense						
			30	368	47	dense			5.0			7.0
			35	363	71	very dense			21.0			6.0
			40	358	100	very dense			30.0	4.4	92.3	3.3
B15	HSA	SPT					365	drilling obs. 6/28/2014				
			7.5	392.5	56	very dense			7.0			
			10	390	29	med dense			7.0			8.0
			15	385	61	very dense			4.0	0.1	91.1	8.8
			20	380	100	very dense			3.0			4.0
			25	375	65	very dense			4.0	3.3	89.1	7.6
			30	370	64	very dense						
			35	365	76	very dense			12.0	9.6	83.1	7.3
			40	360	100	very dense			16.0			4.0
B17	HSA	SPT					458	drilling obs. 6/18/2014				
			10	380	14	med dense			4.0			3.0
			15	375	25	med dense			5.0	6.8	83.9	9.3
			20	370	25	med dense			4.0			7.0
			25	365	40	dense			17.0	2.2	90.5	7.3
			30	360	100	very dense			13.0	11.1	82.6	6.3
			35	355	47	dense			15.0			8.0
			40	350	100	very dense			16.0	2.7	96.0	1.3
			45	345	52	very dense			21.0	0.0	94.8	5.2
			50	340	53	very dense			23.0			
			55	335	48	dense			23.0			11.0
			60	330	40	dense			23.0	0.0	91.1	8.9
			65	325	58	very dense						
			70	320	85	very dense						
			75	315	100	very dense			24.0			9.0

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
B18	HSA	SPT					N/A	N/A				
			5	385	17	med dense			7.0			
			10	380	25	med dense			13.0			
			15	375	38	dense			9.0	1.3	88.7	10.0
			20	370	70	very dense			7.0			9.0
			25	365	68	very dense			8.0			10.0
			30	360	100	very dense			5.0			
			35	355	100	very dense			8.0	0.0	90.5	9.5
			40	350	85	very dense			4.0			
			45	345	100	very dense			7.0	3.8	87.3	8.9
			50	340	82	very dense			4.0			8.0
			55	335	100	very dense						
			60	330	100	very dense			6.0	0.3	90.8	8.9
B19	HSA	SPT					367	drilling obs. 7/23/2014				
			5	387	22	med dense			8.0		93.0	7.0
			10	382	33	dense			7.0	1.9	88.9	9.2
			15	377	52	very dense			6.0			9.0
			20	372	71	very dense			6.0			11.0
			25	367	56	very dense			9.0			7.0
			30	362	100	very dense			15.0	3.4	91.0	5.6
			35	357	76	very dense			13.0	19.6	72.6	7.8
			40	352	43	dense			21.0			7.0
			45	347	43	dense			25.0	0.0	88.9	11.1
			50	342	63	very dense			21.0			
			55	337	77	very dense			20.0			12.0
			60	332	100	very dense			23.0			9.0
			65	327	100	very dense			22.0	0.0	92.0	8.0
			70	322	100	very dense			21.0			7.0
B20	HSA	SPT					N/A	N/A				
			40	392	34	dense						

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
MW-3	BH	D&M					N/A	N/A				
			10	321	26	med dense			10.2			
			20	311	26	med dense			13.3			
			30	301	27	med dense			9.4	1.1	93.7	5.1
			40	291	50	very dense			5.8			
			50	281	11	med dense			4.7			
			60	271	39	dense			3.9	6.3	89.0	4.7
			70	261	48	dense			19.9			
			80	251	N/A	N/A			11.2			
			90	241	N/A	N/A			21.3	8.3	86.7	5.1
			100	231	N/A	N/A			25.8			
			120	211	N/A	N/A			27.8	7.6	89.3	3.1
MW-4	BH	D&M					280.2	VWP 3/27/2002				
			10	377	40	dense			9.1			7.8
			20	367	45	dense			6.0	33.4	59.6	7.0
			30	357	34	dense			10.5			7.1
			40	347	47	dense			5.1	16.4	79.3	4.3
			50	337	72	very dense			5.3			12.4
			60	327	47	dense			13.3	4.3	81.2	14.6
			70	317	63	very dense			4.8			3.6
			80	307	21	med dense			5.2	2.6	91.1	6.3
			90	297	43	dense			4.9			5.3
			100	287	100	very dense			6.6	5.6	88.6	5.9
			110	277	100	very dense			16.8			5.2
			120	267	100	very dense			17.3	2.4	95.2	2.4
			130	257	N/A	N/A			35.4			7.2
			135	252	N/A	N/A			26.1			3.6
			155	232	N/A	N/A			23.6	0.6	95.9	3.6
			175	212	N/A	N/A			22.9	4.8	91.6	3.6
MW-5	BH	D&M					N/A	N/A				
			10	295	16	med dense			8.2			
			20	285	16	med dense			4.8			
			30	275	29	med dense			17.7	2.9	93.9	3.2
			40	265	27	med dense			21.4			
			41	264	27	med dense			22.7			
			50	255	11	med dense			21.1	0.0	97.6	2.4

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
MW-6	BH	D&M					N/A	N/A				
			30	284	6	loose			24.0			
			40	274	33	dense			25.5			
			50	264	10	med dense			31.2			
BW-4	MR	D&M					280.63	VWP 2/26/2002				
			10	358	100	very dense						
			20	348	100	very dense			5.7			
			30	338	100	very dense			11.9	16.4	76.7	6.8
			40	328	100	very dense			11.8			
			50	318	100	very dense			9.6	1.7	92.0	6.2
			60	308	100	very dense			18.1			
			70	298	100	very dense						
			80	288	100	very dense			15.1			
			90	278	100	very dense						
			100	268	100	very dense			18.0			
			110	258	100	very dense			23.0			
			120	248	100	very dense			23.1			
			130	238	100	very dense			20.7			
			140	228	100	very dense			23.7			13.7
			150	218	100	very dense			27.1			
			160	208	100	very dense			27.9			
			170	198	100	very dense			23.0			
			180	188	100	very dense			25.6			
BW-5	MR	D&M					364.2	VWP 2/26/2002				
			10	390	60	very dense			13.0			
			20	380	65	very dense			9.0			
			30	370	100	very dense			11.3	0.1	89.6	10.2
			40	360	100	very dense			13.6			
			50	350	100	very dense			18.9			
			60	340	100	very dense			21.4			
BW-6	MR	D&M					N/A	N/A				
			50	400	100	very dense			9.2	48.2	40.9	10.7
			70	380	100	very dense			20.0			
			80	370	100	very dense			21.5			

Boring ID	Drilling Method	Sampler	Depth (ft)	Elevation (ft)	N-Value	Density	Water Elevation (ft)	Method and Date of Water Measurements	Percent Moisture	Percent Gravel	Percent Sand	Percent Fines
E-108	Wireline	D&M			N/A	N/A	N/A	N/A				
					N/A	N/A						
E-109	Wireline	D&M			N/A	N/A	N/A	N/A				
					N/A	N/A						
E-110	Wireline	D&M			100	very dense	N/A	N/A				
					100	very dense						
E-211	Wireline	D&M			N/A	N/A	287.12	OW, unknown date				
					N/A	N/A						

Appendix D

Unified Soil Classification Guide (Adapted from Zhou, 2006)

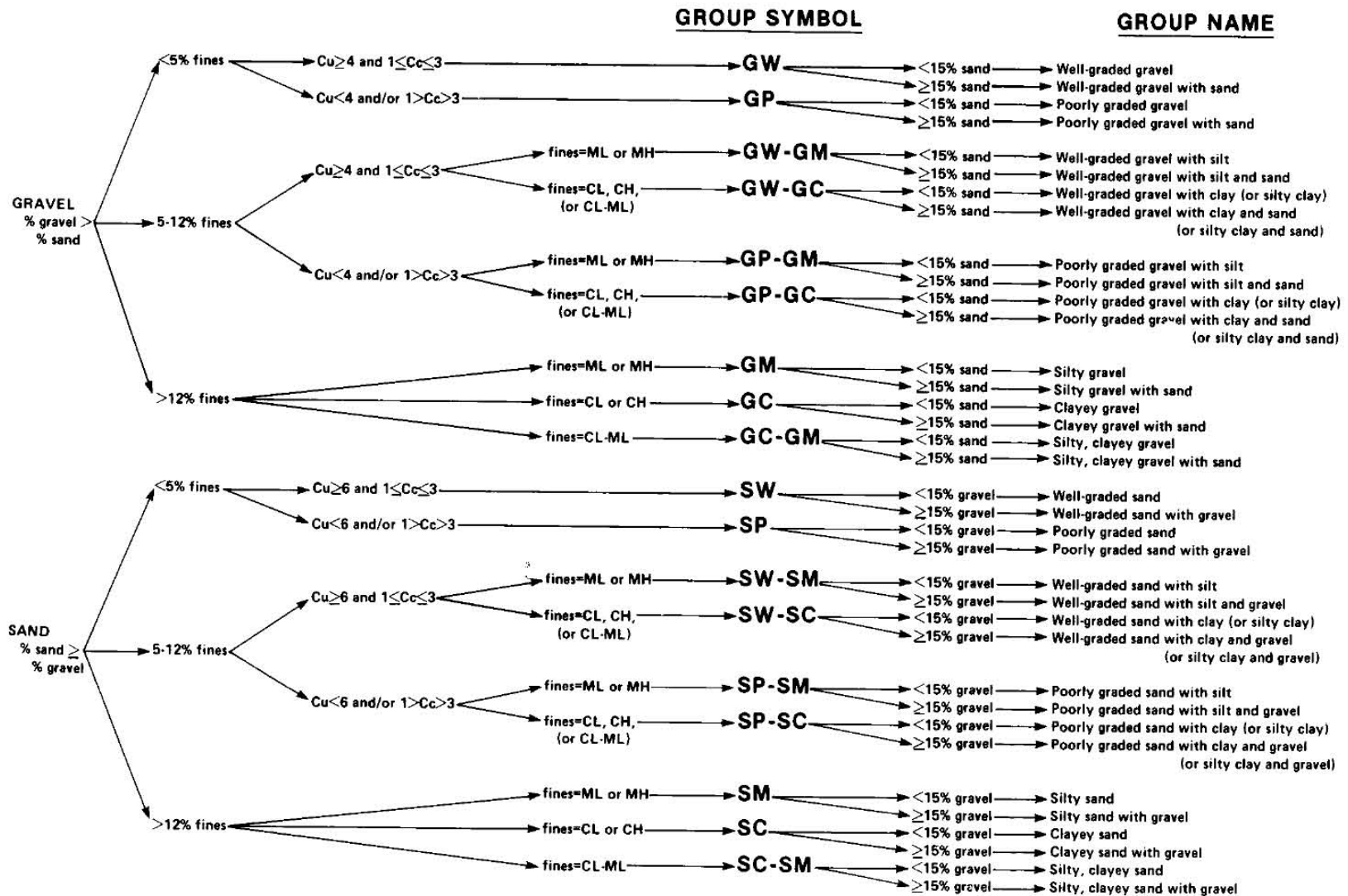


Figure 4-1: Flow chart to determine the group symbol and group name for coarse-grained soils (ASTM D 2487).

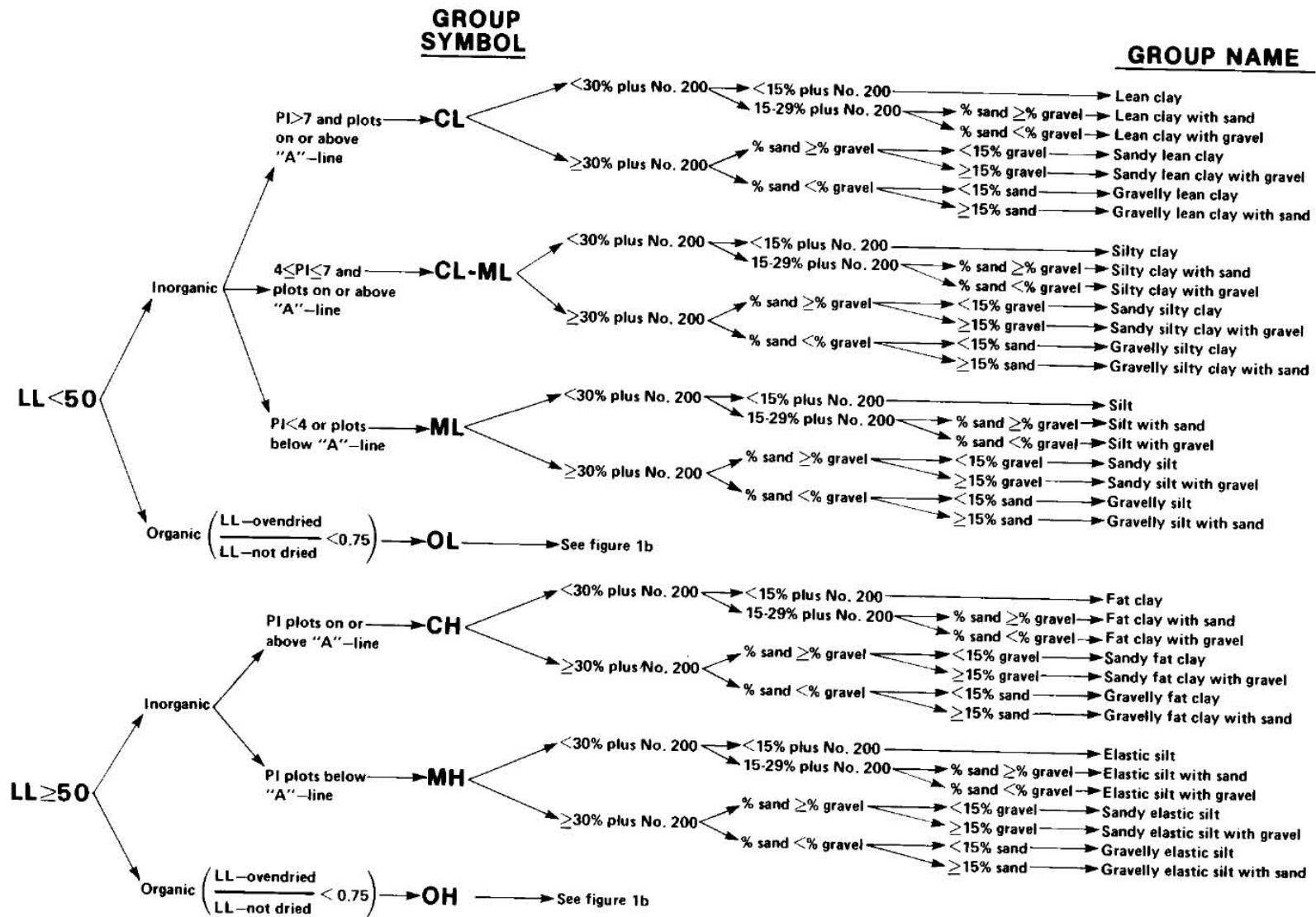


Figure 4-4a. Flow chart to determine the group symbol and group name for fine-grained soils (ASTM D 2487).

GROUP SYMBOL

GROUP NAME

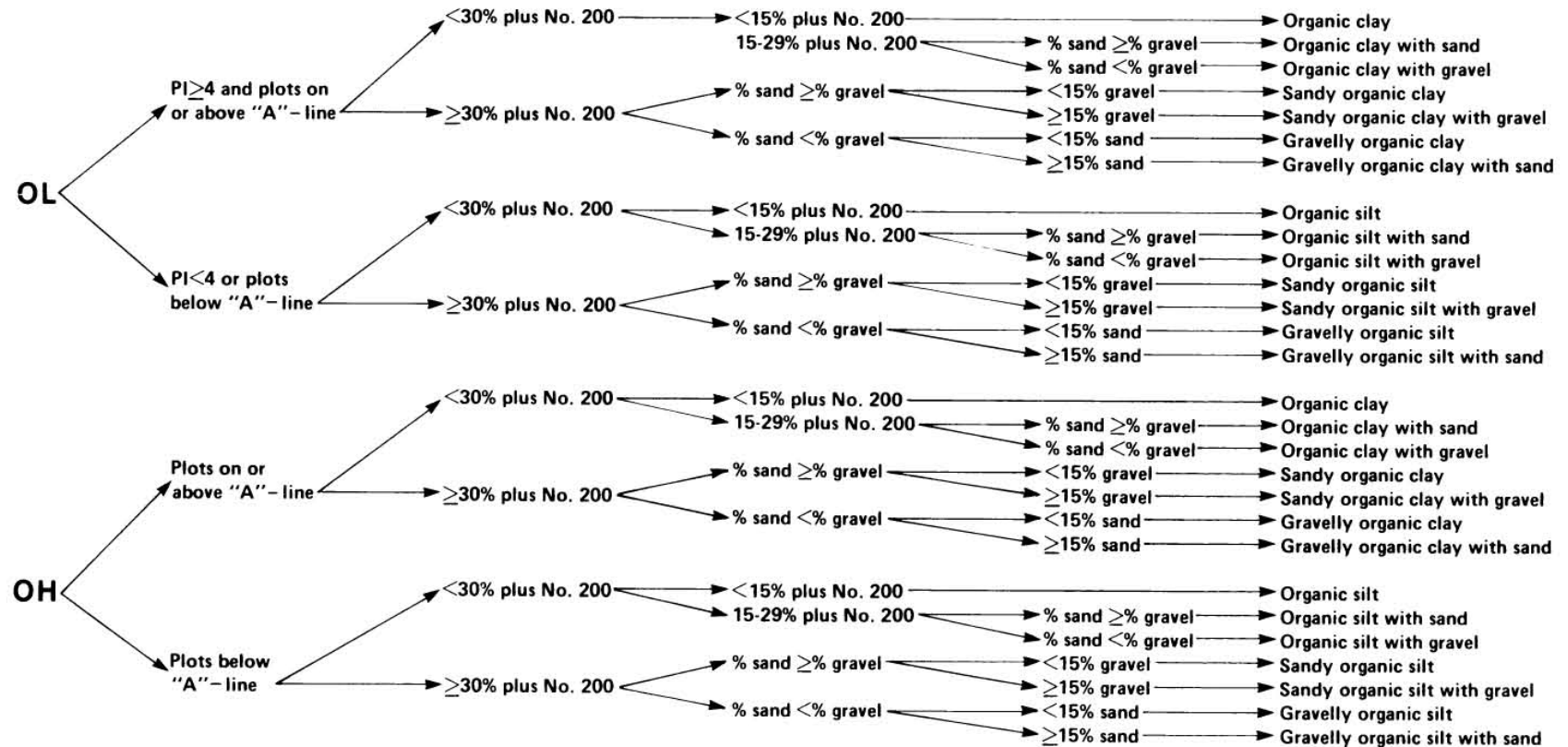


Figure 4-4b. Flow chart to determine the group symbol and group name for organic soils (ASTM D 2487).